

APPENDIX J
Traffic Data



MEMORANDUM

Date: December 20, 2007

To: Samar Hajeer, City of Sacramento
Aelita Milatzo, City of Sacramento

From: Bob Grandy, Fehr & Peers
Dan Block, Fehr & Peers

Subject: *Delta Shores Trip Generation*

SA07-0088A

A revised project description submitted for the Delta Shores project on November 1, 2007, indicates an increase in square footage for the Village Center from 1.23 million square feet to 1.3 million square feet of retail uses. This memorandum identifies the change in trip generation that would occur as a result of the change in retail uses and whether that change is likely to result in the identification of new significant traffic impacts.

Revised Project Description

The original project description for the Delta Shores project is provided below. The change in project description referenced above involves an increase of 70,000 square feet in retail uses in the Village Center.

Original Delta Shores Project Description:

- 5,222 Residential dwelling units
 - 675 single-family residential units
 - 4,547 multi-family residential units
- 1.39 million square-feet of Commercial/Retail uses
 - 1.23 million square-feet retail in the Village Center
 - 161,000 square-feet of mixed use retail in the Town Center
- 31.3 acres of Community uses
 - two elementary schools –700 students each
 - a 22,000 square-foot community center
 - two sports fields

Revised Trip Generation

The AM and PM peak hour trip generation of the project was estimated using trip generation data contained in *Trip Generation* (7th Edition, Institute of Transportation Engineers, 2003) for all uses except the proposed elementary and high school, which was estimated using the *San Diego Trip Generators* (San Diego Association of Governments, 1993). The data from *San Diego Trip Generators* was used because it is based on California data and also has generation data for the PM peak hour; PM data is not available for schools in *Trip Generation*.

Table 1 provides the gross number of vehicle trips that would be generated by the Delta Shores and Stone Boswell projects based on the **original** project description submitted by the applicant and the trip data from the two sources described above.

TABLE 1										
DELTA SHORES										
<u>ORIGINAL</u> TRIP GENERATION – GROSS TRIPS FOR PROPOSED PROJECT										
Land Use	Amount	Source	Trips							
			Weekday	AM Peak Hour			PM Peak Hour			
				In	Out	Total	In	Out	Total	
Single-Family Residential	675 DUs ²	ITE ¹ 210	5,520	119	357	476	338	198	536	
Multi-Family Residential	Single-Family Attached	1,337 DUs ²	ITE ¹ 210	10,934	236	706	942	669	393	1,062
	Medium-Density	1,337 DUs ²	ITE ¹ 230	5,818	70	341	411	338	166	504
	High-Density	1,415 DUs ²	ITE ¹ 220	8,618	139	558	697	515	277	792
Town Center Mixed-Use	Residential	458 DUs ²	ITE ¹ 220	2,790	45	180	225	166	90	256
	Retail	161 ksf ³	ITE ¹ 820	9,255	127	81	208	411	446	857
Village Center Commercial Retail	1,230 ksf ³	ITE ¹ 820	34,704	430	275	705	1574	1706	3280	
Community Center	22 ksf ³	ITE ¹ 495	180	22	14	36	10	26	36	
Park (2 soccer fields)	2 Fields	ITE ¹ 488	144	--	--	--	29	13	42	
Elementary School	1,400 students	SANDAG ⁴	1,960	307	203	510	29	69	98	
Gross Trips			79,923	1,495	2,715	4,210	4,079	3,384	7,463	
Notes:										
1. Based on trip generation data from Trip Generation (ITE, 2003).										
2. DU = dwelling unit										
3. ksf = thousand square feet										
4. Based on trip generation data from San Diego Traffic Generators (San Diego Association of Governments, 1993)										
Source: Fehr & Peers, 2007.										

Table 2 provides the gross number of vehicle trips that would be generated by the Delta Shores and Stone Boswell projects based on the **revised** project description submitted by the applicant and the trip data from the two sources described above.

TABLE 2										
DELTA SHORES										
REVISED TRIP GENERATION – GROSS TRIPS FOR PROPOSED PROJECT										
Land Use	Amount	Source	Weekday	Trips						
				AM Peak Hour			PM Peak Hour			
				In	Out	Total	In	Out	Total	
Single-Family Residential	675 DUs ²	ITE ¹ 210	5,520	119	357	476	338	198	536	
Multi-Family Residential	Single-Family Attached	1,337 DUs ²	ITE ¹ 210	10,934	236	706	942	669	393	1,062
	Medium-Density	1,337 DUs ²	ITE ¹ 230	5,818	70	341	411	338	166	504
	High-Density	1,415 DUs ²	ITE ¹ 220	8,618	139	558	697	515	277	792
Town Center Mixed-Use	Residential	458 DUs ²	ITE ¹ 220	2,790	45	180	225	166	90	256
	Retail	161 ksf ³	ITE ¹ 820	9,255	127	81	208	411	446	857
Village Center Commercial Retail	1,300 ksf ³	ITE ¹ 820	35,975	445	284	729	1633	1770	3403	
Community Center	22 ksf ³	ITE ¹ 495	180	22	14	36	10	26	36	
Park (2 soccer fields)	2 Fields	ITE ¹ 488	144	--	--	--	29	13	42	
Elementary School	1,400 students	SANDAG ⁴	1,960	307	203	510	29	69	98	
Gross Trips			81,194	1,510	2,724	4,234	4,138	3,448	7,586	
Notes:										
1. Based on trip generation data from Trip Generation (ITE, 2003).										
2. DU = dwelling unit										
3. ksf = thousand square feet										
4. Based on trip generation data from San Diego Traffic Generators (San Diego Association of Governments, 1993)										
Source: Fehr & Peers, 2007.										

The Village Center, as defined in the original project description, was projected to generate approximately 705 gross trips during the AM peak hour and 3,280 gross trips during the PM peak hour. The addition of 70,000 gross square feet of retail uses would generate approximately 20 additional trips during the AM peak hour and 125 additional trips during the PM peak hour.

Most of the trips generated by the Village Center would travel via Delta Shores Circle to Cosumnes River Boulevard, with the majority then destined to Interstate 5. The following is a brief summary of projected operating conditions at the study intersections between the Village Center and the I-5/Cosumnes River Boulevard interchange.

- Cosumnes River Boulevard/I-5 SB Ramps – this intersection is projected to operate at LOS B conditions in the AM peak hour and LOS C conditions in the PM peak hour under the Cumulative Plus Project scenario. Given the LOS E threshold for this intersection (i.e., Caltrans threshold), the added traffic generated by the 70,000 s.f. of additional retail uses would not likely generate significant additional impacts at this location.

- Cosumnes River Boulevard/I-5 NB Ramps – this intersection is projected to operate at LOS B conditions in the AM peak hour and LOS C conditions in the PM peak hour under the Cumulative Plus Project scenario. Given the LOS E threshold for this intersection (i.e., Caltrans threshold), the added traffic generated by the 70,000 s.f. of additional retail uses would not likely generate significant additional impacts at this location.
- Cosumnes River Boulevard/Delta Shores Circle (West) – this intersection is projected to operate at LOS D conditions in the AM peak hour and LOS E conditions in the PM peak hour under the Cumulative Plus Project scenario. These future conditions exceed the City's LOS C threshold. Additional mitigations at this intersection were deemed infeasible by the City as the improvements would be inconsistent with the City of Sacramento goals and objectives to create pedestrian-friendly streets and the Smart Growth policies. Therefore, the project's impact would be considered significant and unavoidable. The added traffic generated by the 70,000 s.f. of additional retail uses would not change this impact or mitigation determination.
- Delta Shores Circle South/Street D (North) - this intersection is projected to operate at LOS C conditions in the AM peak hour and LOS C conditions in the PM peak hour under the Cumulative Plus Project scenario. These future conditions do not exceed the City's LOS C threshold. The added traffic generated by the 70,000 s.f. of additional retail uses would not likely generate significant additional impacts at this location.

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	65	62	95	8	21.8	C	--
	R	52	54	104	7	12.6	B	--
	Subtotal	117	116	99	--	17.5	B	--
SB	L	73	71	97	7	24.4	C	--
	T	33	29	88	6	27.2	C	--
	Subtotal	106	100	94	--	25.2	C	--
WB	L	49	47	96	5	23.8	C	--
	R	71	75	106	6	5.9	A	--
	Subtotal	120	123	103	--	12.8	B	--
Total		342	339	99	--	18.1	B	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	57	61	107	7	15.2	B	--
	R	22	23	100	4	7.6	A	--
	Subtotal	79	84	106	--	13.2	B	--
EB	T	46	47	102	7	4.2	A	--
	R	79	78	99	9	3.3	A	--
	Subtotal	125	125	100	--	3.7	A	--
WB	T	98	101	103	11	3.2	A	--
	R	33	33	100	5	2.5	A	--
	Subtotal	130	134	103	--	3.0	A	--
Total		334	342	102	--	5.7	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	71	77	108	7	18.9	B	--
	R	35	38	109	6	2.9	A	--
	Subtotal	106	115	108	--	13.6	B	--
EB	T	71	73	103	7	3.5	A	--
	R	33	33	100	5	2.2	A	--
	Subtotal	103	106	103	--	3.1	A	--
WB	T	60	59	98	7	3.0	A	--
	R	188	187	99	11	6.0	A	--
	Subtotal	247	247	100	--	5.3	A	--
Total		457	467	102	--	6.9	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2194	186	--	--	115	--	--
	R	150	194	Yes	--	115	--	--
SB	L	150	242	Yes	--	152	Yes	--
	T	1133	242	--	--	152	--	--
WB	L	1068	149	--	--	95	--	--
	R	1068	88	--	--	49	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1433	88	--	--	58	--	--
	T	1433	70	--	--	40	--	--
	R	350	53	--	--	33	--	--
EB	T	1068	54	--	--	20	--	--
	R	250	49	--	--	27	--	--
WB	T	1047	46	--	--	17	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	87	--	--	62	--	--
	T	1532	79	--	--	60	--	--
	R	450	55	--	--	31	--	--
EB	T	1047	66	--	--	25	--	--
	R	1047	50	--	--	13	--	--
WB	T	2173	42	--	--	13	--	--
	R	2173	52	--	--	36	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	76	79	104	11	25.8	C	--
	R	41	40	98	5	11.1	B	--
	Subtotal	117	119	102	--	20.8	C	--
SB	L	147	146	99	8	42.1	D	--
	T	41	42	102	4	44.5	D	--
	Subtotal	187	188	101	--	42.6	D	--
WB	L	65	69	106	6	30.3	C	--
	R	5	5	100	3	1.8	A	--
	Subtotal	71	74	104	--	28.5	C	--
Total		375	380	101	--	33.1	C	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	179	177	98	13	15.8	B	--
	R	43	46	107	7	8.3	A	--
	Subtotal	223	223	100	--	14.2	B	--
EB	T	41	40	98	7	10.5	B	--
	R	147	150	102	9	9.2	A	--
	Subtotal	187	190	102	--	9.5	A	--
WB	T	27	28	104	4	5.0	A	--
	R	38	35	92	6	1.9	A	--
	Subtotal	65	63	97	--	3.2	A	--
Total		476	476	100	--	10.9	B	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	14	14	100	3	19.1	B	--
	R	38	38	100	9	4.9	A	--
	Subtotal	52	52	100	--	8.7	A	--
EB	T	198	201	102	12	3.1	A	--
	R	22	18	82	3	3.7	A	--
	Subtotal	220	220	100	--	3.2	A	--
WB	T	52	48	92	6	1.5	A	--
	R	111	109	98	11	2.5	A	--
	Subtotal	163	157	96	--	2.2	A	--
Total		435	428	98	--	3.5	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2194	238	--	--	144	--	--
	R	150	145	--	--	63	--	--
SB	L	150	485	Yes	--	296	Yes	--
	T	1133	485	--	--	296	--	--
WB	L	1068	201	--	--	131	--	--
	R	1068	19	--	--	7	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1433	179	--	--	113	--	--
	T	1433	151	--	--	96	--	--
	R	350	50	--	--	35	--	--
EB	T	1068	55	--	--	26	--	--
	R	250	90	--	--	64	--	--
WB	T	1047	41	--	--	17	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	39	--	--	17	--	--
	T	1532	29	--	--	15	--	--
	R	450	48	--	--	31	--	--
EB	T	1047	77	--	--	31	--	--
	R	1047	70	--	--	27	--	--
WB	T	2173	23	--	--	4	--	--
	R	2173	32	--	--	12	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	71	71	100	6	28.8	C	--
	R	49	47	96	8	13.3	B	--
	Subtotal	120	118	98	--	22.7	C	--
SB	L	87	87	100	8	19.5	B	--
	T	33	32	97	7	22.6	C	--
	Subtotal	120	119	99	--	20.3	C	--
WB	L	71	72	101	7	21.3	C	--
	R	103	99	96	9	10.8	B	--
	Subtotal	174	171	98	--	15.2	B	--
Total	413	408	99	--	18.9	B	--	

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	188	190	101	9	18.5	B	--
	R	27	29	107	5	9.9	A	--
	Subtotal	215	219	102	--	17.4	B	--
EB	T	90	87	97	9	11.3	B	--
	R	73	76	104	10	4.4	A	--
	Subtotal	163	163	100	--	8.1	A	--
WB	T	166	162	98	11	12.6	B	--
	R	84	82	98	8	3.0	A	--
	Subtotal	250	245	98	--	9.3	A	--
Total	628	627	100	--	11.8	B	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	54	52	96	4	34.2	C	--
	R	95	97	102	10	11.3	B	--
	Subtotal	149	149	100	--	19.4	B	--
EB	T	234	231	99	8	5.3	A	--
	R	43	48	112	6	3.5	A	--
	Subtotal	277	279	101	--	5.0	A	--
WB	T	245	241	98	7	9.8	A	--
	R	277	262	95	9	14.6	B	--
	Subtotal	522	503	96	--	12.3	B	--
Total		948	931	98	--	11.2	B	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	11	10	91	5	0.9	A	--
	Subtotal	11	10	91	--	0.9	A	--
EB	T	310	306	99	18	0.5	A	--
	R	19	19	100	6	0.2	A	--
	Subtotal	329	325	99	--	0.5	A	--
WB	T	511	493	96	14	4.1	A	--
	R	11	11	100	4	4.2	A	--
	Subtotal	522	504	97	--	4.1	A	--
Total		861	840	98	--	2.7	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	117	106	91	10	38.6	D	--
	T	19	19	95	4	32.6	C	--
	R	8	8	100	3	9.4	A	--
	Subtotal	144	132	92	--	36.0	D	--
SB	L	14	12	86	5	58.4	E	--
	T	33	28	85	5	49.7	D	--
	R	128	124	97	8	32.6	C	--
	Subtotal	174	164	94	--	37.4	D	--
EB	L	133	124	93	13	38.4	D	--
	T	122	122	100	10	9.8	A	--
	R	54	54	100	7	2.5	A	--
	Subtotal	310	300	97	--	20.3	C	--
WB	L	8	9	113	3	79.5	E	--
	T	277	269	97	12	26.6	C	--
	R	5	6	120	2	8.7	A	--
	Subtotal	291	284	98	--	27.8	C	--
Total	918	879	96	--	28.3	C	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	3	2	67	1	49.1	D	--
	T	54	51	94	9	15.7	B	--
	R	3	3	100	2	6.3	A	--
	Subtotal	60	57	95	--	16.5	B	--
SB	L	16	17	106	5	40.2	D	--
	T	46	43	93	5	7.5	A	--
	R	19	19	100	6	4.2	A	--
	U	11	10	91	4	47.5	D	--
Subtotal	92	89	97	--	17.4	B	--	
EB	L	22	17	77	3	46.3	D	--
	T	3	3	100	1	31.0	C	--
	R	3	3	100	2	2.5	A	--
	Subtotal	27	24	89	--	38.5	D	--
WB	L	3	3	100	2	55.5	E	--
	T	5	5	100	2	48.8	D	--
	R	52	54	104	5	14.5	B	--
	Subtotal	60	62	103	--	19.4	B	--
Total	239	231	97	--	19.9	B	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	33	34	103	7	5.5	A	--
	Subtotal	33	34	103	--	5.5	A	--
SB	R	3	2	67	1	4.6	A	--
	Subtotal	3	2	67	--	4.6	A	--
EB	L	3	2	67	1	7.3	A	--
	T	130	129	99	9	1.0	A	--
	R	3	3	100	2	0.9	A	--
	Subtotal	136	134	99	--	1.1	A	--
WB	L	16	16	100	3	6.2	A	--
	T	171	169	99	9	3.1	A	--
	R	5	6	120	3	1.9	A	--
	Subtotal	193	191	99	--	3.3	A	--
Total		364	361	99	--	2.7	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	212	--	--	142	--	--
	R	200	214	Yes	--	142	--	--
SB	L	165	155	--	--	110	--	--
	T	406	151	--	--	110	--	--
WB	L	370	194	--	--	118	--	--
	R	370	155	--	--	82	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	222	--	--	144	--	--
	T	1427	211	--	--	133	--	--
	R	350	61	--	--	33	--	--
EB	T	637	101	--	--	53	--	--
	R	250	52	--	--	32	--	--
WB	T	928	189	--	--	110	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	112	--	--	73	--	--
	T	1538	112	--	--	73	--	--
	R	450	138	--	--	77	--	--
EB	T	75	165	Yes	--	59	--	--
	R	75	131	Yes	--	41	--	--
WB	T	503	730	Yes	--	131	--	--
	R	503	744	Yes	--	168	--	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
WB	T	437	529	Yes	--	76	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	207	--	--	112	--	--
	T	562	110	--	--	36	--	--
	R	562	105	--	--	34	--	--
SB	L	150	150	--	--	51	--	--
	T	328	444	Yes	--	213	--	--
	R	200	326	Yes	--	229	Yes	--
EB	L	500	249	--	--	167	--	--
	T	437	89	--	--	51	--	--
WB	L	300	57	--	--	26	--	--
	T	1119	284	--	--	151	--	--
	R	300	30	--	--	8	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	32	--	--	12	--	--
	T	1101	114	--	--	65	--	--
	R	1101	55	--	--	28	--	--
SB	L	200	71	--	--	38	--	--
	T	498	61	--	--	23	--	--
	R	498	25	--	--	8	--	--
EB	U	200	66	--	--	35	--	--
	L	2396	85	--	--	42	--	--
	T	2396	27	--	--	7	--	--
WB	R	200	21	--	--	6	--	--
	L	200	31	--	--	11	--	--
	T	901	221	--	--	95	--	--
	R	901	221	--	--	95	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	79	--	--	48	--	--
SB	R	403	31	--	--	9	--	--
EB	L	100	31	--	--	6	--	--
WB	L	250	52	--	--	20	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	92	87	95	11	36.8	D	--
	R	60	57	95	9	18.7	B	--
	Subtotal	152	145	95	--	29.7	C	--
SB	L	152	146	96	9	30.2	C	--
	T	49	48	98	7	34.3	C	--
	Subtotal	201	194	97	--	31.2	C	--
WB	L	68	66	97	8	21.8	C	--
	R	46	46	100	7	7.9	A	--
	Subtotal	114	112	98	--	16.1	B	--
Total	467	450	96	--	27.0	C	--	

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	372	367	99	17	23.4	C	--
	R	38	39	103	4	13.0	B	--
	Subtotal	410	406	99	--	22.4	C	--
EB	T	111	108	97	6	17.9	B	--
	R	120	111	93	12	7.0	A	--
	Subtotal	231	218	94	--	12.4	B	--
WB	T	120	118	98	12	17.1	B	--
	R	133	125	93	12	3.4	A	--
	Subtotal	253	243	96	--	10.1	B	--
Total	894	867	97	--	16.4	B	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	14	12	86	4	20.7	C	--
	R	149	148	99	11	23.6	C	--
	Subtotal	163	160	98	--	23.4	C	--
EB	T	470	455	97	18	13.6	B	--
	R	14	15	100	5	10.1	B	--
	Subtotal	484	470	97	--	13.5	B	--
WB	T	285	280	98	22	16.1	B	--
	R	261	254	97	17	19.7	B	--
	Subtotal	546	534	98	--	17.8	B	--
Total		1193	1164	98	--	16.8	B	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	52	45	87	7	1.3	A	--
	Subtotal	52	45	87	--	1.3	A	--
EB	T	552	533	97	16	1.4	A	--
	R	68	67	99	7	1.1	A	--
	Subtotal	620	601	97	--	1.4	A	--
WB	T	495	489	99	18	4.8	A	--
	R	27	6	22	1	3.8	A	--
	Subtotal	522	495	95	--	4.8	A	--
Total		1193	1141	96	--	2.9	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	217	205	94	9	51.3	D	--
	T	63	61	98	6	32.2	C	--
	R	24	23	96	6	19.8	B	--
	Subtotal	304	289	95	--	44.8	D	--
SB	L	27	26	96	5	60.2	E	--
	T	79	76	96	10	48.2	D	--
	R	109	102	94	7	26.3	C	--
	Subtotal	215	205	95	--	38.8	D	--
EB	L	152	141	93	10	39.9	D	--
	T	245	250	102	11	24.8	C	--
	R	155	147	95	9	5.0	A	--
	Subtotal	552	538	97	--	23.4	C	--
WB	L	30	31	103	7	53.2	D	--
	T	196	187	95	19	33.7	C	--
	R	5	26	520	7	9.3	A	--
	Subtotal	231	244	106	--	33.6	C	--
Total	1302	1275	98	--	32.6	C	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	8	7	88	3	53.7	D	--
	T	114	107	94	8	34.0	C	--
	R	3	3	67	1	20.5	C	--
	Subtotal	125	117	94	--	35.0	C	--
SB	L	33	37	112	5	39.0	D	--
	T	128	128	100	11	17.9	B	--
	R	54	55	102	9	6.5	A	--
	Subtotal	215	219	102	--	18.6	B	--
EB	L	166	121	73	10	42.6	D	--
	T	24	24	100	4	27.9	C	--
	R	8	8	100	3	4.5	A	--
	Subtotal	198	153	77	--	38.3	D	--
WB	L	3	3	100	2	65.1	E	--
	T	8	8	100	3	49.7	D	--
	R	27	30	111	7	20.4	C	--
	Subtotal	38	41	105	--	29.3	C	--
Total	576	530	92	--	28.7	C	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	24	21	88	4	6.5	A	--
	Subtotal	24	21	88	--	6.5	A	--
SB	R	3	3	100	1	3.0	A	--
	Subtotal	3	3	100	--	3.0	A	--
EB	L	3	3	100	1	3.9	A	--
	T	207	199	96	16	1.3	A	--
	R	3	3	100	2	0.6	A	--
	Subtotal	212	204	96	--	1.3	A	--
WB	L	33	33	100	6	10.5	B	--
	T	111	109	98	9	3.2	A	--
	R	14	13	93	4	2.0	A	--
	Subtotal	158	154	97	--	4.7	A	--
Total		397	383	96	--	3.0	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	352	--	--	211	--	--
	R	200	352	Yes	--	211	Yes	--
SB	L	165	462	Yes	--	257	Yes	--
	T	406	462	Yes	--	257	--	--
WB	L	370	193	--	--	101	--	--
	R	370	72	--	--	38	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	413	--	--	260	--	--
	T	1427	372	--	--	247	--	--
	R	350	94	--	--	39	--	--
EB	T	637	126	--	--	76	--	--
	R	250	83	--	--	52	--	--
WB	T	928	142	--	--	89	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	42	--	--	20	--	--
	T	1538	42	--	--	19	--	--
	R	450	204	--	--	138	--	--
EB	T	75	408	Yes	--	230	Yes	--
	R	75	407	Yes	--	229	Yes	--
WB	T	503	580	Yes	--	215	--	--
	R	503	546	Yes	--	215	--	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
WB	T	437	104	--	--	33	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	324	--	--	214	--	--
	T	562	171	--	--	93	--	--
	R	562	163	--	--	91	--	--
SB	L	150	189	Yes	--	65	--	--
	T	328	403	Yes	--	168	--	--
	R	200	324	Yes	--	184	--	--
EB	L	500	268	--	--	187	--	--
	T	437	234	--	--	174	--	--
	R	300	34	--	--	6	--	--
WB	L	300	111	--	--	67	--	--
	T	1119	267	--	--	183	--	--
	R	300	70	--	--	40	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	95	--	--	35	--	--
	T	1101	219	--	--	154	--	--
	R	1101	187	--	--	119	--	--
SB	L	200	569	Yes	--	170	--	--
	T	498	572	Yes	--	143	--	--
	R	498	448	--	--	84	--	--
EB	U	200	174	--	--	114	--	--
	L	2396	256	--	--	177	--	--
	T	2396	102	--	--	53	--	--
WB	R	200	37	--	--	12	--	--
	L	200	44	--	--	12	--	--
	T	901	172	--	--	86	--	--
	R	901	172	--	--	86	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Baseline With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	67	--	--	39	--	--
SB	R	403	33	--	--	11	--	--
EB	L	100	31	--	--	6	--	--
	T	370	3	--	--	0	--	--
WB	L	250	94	--	--	48	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	111	105	95	4	94.8	F	--
	R	76	90	118	10	75.8	E	--
	Subtotal	188	195	104	--	86.0	F	--
SB	L	71	65	92	10	159.7	F	--
	T	46	40	87	6	163.4	F	--
	Subtotal	117	105	90	--	161.1	F	--
WB	L	98	126	129	10	41.0	D	--
	R	130	124	95	9	16.7	B	--
	Subtotal	228	250	110	--	28.9	C	--
Total		533	550	103	--	74.4	E	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	71	115	162	6	18.2	B	--
	R	27	31	115	5	11.1	B	--
	Subtotal	98	146	149	--	16.7	B	--
EB	T	63	64	103	13	6.8	A	--
	R	84	91	108	8	4.4	A	--
	Subtotal	147	155	105	--	5.4	A	--
WB	T	201	228	113	12	6.4	A	--
	R	54	54	100	5	3.8	A	--
	Subtotal	255	282	111	--	5.9	A	--
Total		500	583	117	--	8.5	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	160	180	113	12	22.0	C	--
	R	71	76	107	12	4.6	A	--
	Subtotal	231	257	111	--	16.8	B	--
EB	T	92	142	154	9	5.1	A	--
	R	41	38	93	7	3.3	A	--
	Subtotal	133	180	135	--	4.7	A	--
WB	T	95	102	107	9	10.8	B	--
	R	245	247	100	9	7.2	A	--
	Subtotal	340	349	102	--	8.3	A	--
Total		704	785	112	--	10.3	B	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2194	1169	--	--	783	--	--
	R	150	1169	Yes	--	783	Yes	--
SB	L	150	913	Yes	--	685	Yes	--
	T	1133	913	--	--	685	--	--
WB	L	1068	419	--	--	293	--	--
	R	1068	221	--	--	144	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1433	143	--	--	96	--	--
	T	1433	122	--	--	81	--	--
	R	350	74	--	--	40	--	--
EB	T	1068	83	--	--	34	--	--
	R	250	57	--	--	36	--	--
WB	T	1047	105	--	--	53	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	189	--	--	127	--	--
	T	1532	179	--	--	120	--	--
	R	450	64	--	--	38	--	--
EB	T	1047	88	--	--	39	--	--
	R	1047	70	--	--	27	--	--
WB	T	2173	167	--	--	94	--	--
	R	2173	169	--	--	94	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	98	89	91	8	69.6	E	--
	R	106	140	132	14	48.7	D	--
	Subtotal	204	229	112	--	56.9	E	--
SB	L	111	132	119	8	70.3	E	--
	T	63	56	90	6	74.3	E	--
	Subtotal	174	188	108	--	71.5	E	--
WB	L	76	99	130	8	47.9	D	--
	R	68	53	78	8	7.0	A	--
	Subtotal	144	152	106	--	33.6	C	--
Total		522	569	109	--	55.5	E	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	207	227	109	17	18.6	B	--
	R	43	54	126	8	10.1	B	--
	Subtotal	250	280	112	--	16.9	B	--
EB	T	71	90	127	7	12.2	B	--
	R	147	182	124	10	10.5	B	--
	Subtotal	217	272	125	--	11.0	B	--
WB	T	101	101	100	11	8.5	A	--
	R	65	75	115	6	3.4	A	--
	Subtotal	166	176	106	--	6.3	A	--
Total		633	728	115	--	12.2	B	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	84	72	86	10	23.2	C	--
	R	52	47	90	6	5.9	A	--
	Subtotal	136	119	88	--	16.4	B	--
EB	T	239	277	116	19	5.5	A	--
	R	38	37	97	6	6.0	A	--
	Subtotal	277	314	113	--	5.5	A	--
WB	T	82	105	128	6	7.3	A	--
	R	166	190	114	10	5.0	A	--
	Subtotal	247	295	119	--	5.8	A	--
Total		660	727	110	--	7.4	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2194	1084	--	--	612	--	--
	R	150	180	Yes	--	170	Yes	--
SB	L	150	946	Yes	--	552	Yes	--
	T	1133	946	--	--	552	--	--
WB	L	1068	377	--	--	247	--	--
	R	1068	78	--	--	44	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1433	217	--	--	155	--	--
	T	1433	201	--	--	143	--	--
	R	350	65	--	--	43	--	--
EB	T	1068	304	--	--	101	--	--
	R	250	122	--	--	73	--	--
WB	T	1047	99	--	--	51	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative No Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	94	--	--	61	--	--
	T	1532	88	--	--	59	--	--
	R	450	58	--	--	35	--	--
EB	T	1047	132	--	--	72	--	--
	R	1047	122	--	--	65	--	--
WB	T	2173	109	--	--	58	--	--
	R	2173	109	--	--	58	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	111	102	92	5	118.7	F	--
	R	101	93	92	12	88.9	F	--
	Subtotal	212	195	92	--	104.6	F	--
SB	L	92	93	101	7	24.0	C	--
	T	46	42	91	7	26.9	C	--
	Subtotal	139	136	98	--	24.9	C	--
WB	L	149	122	82	4	98.6	F	--
	R	136	122	90	13	39.6	D	--
	Subtotal	285	244	86	--	69.0	E	--
Total		636	574	90	--	70.6	E	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	209	211	101	16	21.9	C	--
	R	35	35	100	4	13.2	B	--
	Subtotal	245	246	100	--	20.7	C	--
EB	T	120	119	98	12	12.1	B	--
	R	101	93	92	10	5.3	A	--
	Subtotal	220	212	96	--	9.1	A	--
WB	T	272	242	89	20	14.5	B	--
	R	101	94	93	12	4.0	A	--
	Subtotal	372	336	90	--	11.6	B	--
Total		837	793	95	--	13.7	B	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	168	152	90	6	94.4	F	--
	R	130	133	102	13	13.6	B	--
	Subtotal	299	284	95	--	56.7	E	--
EB	T	280	279	99	21	5.5	A	--
	R	49	47	96	9	3.6	A	--
	Subtotal	329	326	99	--	5.2	A	--
WB	T	269	249	92	21	28.1	C	--
	R	364	306	84	3	58.0	E	--
	Subtotal	633	555	88	--	44.6	D	--
Total	1261	1165	92	--	36.5	D	--	

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	8	9	113	4	0.7	A	--
	Subtotal	8	9	113	--	0.7	A	--
EB	T	391	391	100	26	0.6	A	--
	R	19	19	100	3	0.1	A	--
	Subtotal	410	410	100	--	0.6	A	--
WB	T	625	557	89	21	14.2	B	--
	R	11	11	100	3	16.5	C	--
	Subtotal	636	567	89	--	14.3	B	--
Total	1054	986	94	--	8.5	A	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	117	111	95	12	41.0	D	--
	T	16	16	100	3	32.3	C	--
	R	11	10	91	2	10.8	B	--
	Subtotal	144	136	94	--	37.9	D	--
SB	L	14	11	79	3	87.8	F	--
	T	24	24	100	5	70.5	E	--
	R	114	95	83	11	100.6	F	--
	Subtotal	152	130	86	--	94.0	F	--
EB	L	120	114	95	12	60.6	E	--
	T	223	221	99	19	11.3	B	--
	R	49	50	102	7	2.5	A	--
	Subtotal	391	385	98	--	24.7	C	--
WB	L	5	7	120	2	85.6	F	--
	T	405	374	92	18	54.4	D	--
	R	5	4	80	3	36.3	D	--
	Subtotal	416	384	92	--	54.7	D	--
Total	1103	1036	94	--	46.3	D	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	8	9	113	2	56.1	E	--
	T	52	49	92	7	21.4	C	--
	R	3	4	133	2	9.4	A	--
	Subtotal	63	61	98	--	25.6	C	--
SB	L	16	17	106	3	39.5	D	--
	T	43	46	107	5	9.4	A	--
	R	11	12	109	3	4.2	A	--
	U	11	12	109	3	38.5	D	--
Subtotal	82	88	107	--	18.6	B	--	
EB	L	22	21	95	6	44.9	D	--
	T	3	3	100	2	32.4	C	--
	R	5	5	100	2	2.2	A	--
	Subtotal	30	29	97	--	35.9	D	--
WB	L	3	3	100	1	56.4	E	--
	T	8	8	100	3	49.0	D	--
	R	54	54	100	8	17.7	B	--
	Subtotal	65	65	100	--	23.4	C	--
Total	239	243	102	--	23.7	C	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	33	30	91	7	6.5	A	--
	Subtotal	33	30	91	--	6.5	A	--
SB	R	3	3	100	2	7.4	A	--
	Subtotal	3	3	100	--	7.4	A	--
EB	L	3	3	100	2	9.2	A	--
	T	188	180	96	16	1.2	A	--
	R	3	3	100	2	0.8	A	--
	Subtotal	193	186	96	--	1.3	A	--
WB	L	19	15	79	4	18.0	C	--
	T	283	242	86	16	25.4	D	--
	R	5	5	100	1	6.0	A	--
	Subtotal	307	262	85	--	24.6	C	--
Total		535	481	90	--	14.4	B	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	1370	--	--	950	--	--
	R	200	1370	Yes	--	950	Yes	--
SB	L	165	236	Yes	--	155	--	--
	T	406	236	--	--	155	--	--
WB	L	370	878	Yes	--	573	Yes	--
	R	370	853	Yes	--	391	Yes	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	249	--	--	171	--	--
	T	1427	236	--	--	160	--	--
	R	350	81	--	--	44	--	--
EB	T	637	129	--	--	67	--	--
	R	250	63	--	--	38	--	--
WB	T	928	278	--	--	141	--	--
	R	175	59	--	--	6	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	421	--	--	298	--	--
	T	1538	420	--	--	297	--	--
	R	450	196	--	--	111	--	--
EB	T	75	135	Yes	--	58	--	--
	R	75	121	Yes	--	56	--	--
WB	T	503	955	Yes	--	548	Yes	--
	R	503	1382	Yes	--	1008	Yes	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
EB	T	151	98	--	--	14	--	--
WB	T	437	568	Yes	--	292	--	--
	R	150	160	Yes	--	45	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	183	--	--	122	--	--
	T	562	75	--	--	37	--	--
	R	562	74	--	--	37	--	--
SB	L	150	358	Yes	--	173	Yes	--
	T	328	688	Yes	--	452	Yes	--
	R	200	528	Yes	--	366	Yes	--
EB	L	500	276	--	--	193	--	--
	T	437	192	--	--	102	--	--
WB	L	300	47	--	--	23	--	--
	T	1119	712	--	--	371	--	--
	R	300	143	--	--	25	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	73	--	--	37	--	--
	T	1101	128	--	--	74	--	--
	R	1101	89	--	--	42	--	--
SB	L	200	94	--	--	44	--	--
	T	498	70	--	--	34	--	--
	R	498	30	--	--	9	--	--
EB	U	200	89	--	--	39	--	--
	L	2396	78	--	--	49	--	--
	T	2396	22	--	--	6	--	--
WB	R	200	21	--	--	8	--	--
	L	200	37	--	--	12	--	--
	T	901	229	--	--	116	--	--
	R	901	229	--	--	116	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	73	--	--	46	--	--
SB	R	403	39	--	--	12	--	--
EB	L	100	31	--	--	8	--	--
WB	L	250	143	--	--	42	--	--
	T	637	475	--	--	213	--	--
	R	637	429	--	--	136	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	111	95	86	5	120.7	F	--
	R	147	130	88	15	96.6	F	--
	Subtotal	258	225	87	--	106.8	F	--
SB	L	152	138	91	10	75.4	E	--
	T	65	63	97	7	78.3	E	--
	Subtotal	217	201	93	--	76.3	E	--
WB	L	109	105	96	6	57.9	E	--
	R	52	49	94	5	14.0	B	--
	Subtotal	160	154	96	--	44.0	D	--
	Total	636	579	91	--	79.5	E	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	459	444	97	13	38.2	D	--
	R	52	51	98	6	22.7	C	--
	Subtotal	511	495	97	--	36.6	D	--
EB	T	144	132	92	11	29.3	C	--
	R	174	160	92	15	10.6	B	--
	Subtotal	318	292	92	--	19.1	B	--
WB	T	152	147	97	13	34.6	C	--
	R	171	157	91	11	4.8	A	--
	Subtotal	323	304	94	--	19.2	B	--
	Total	1152	1090	95	--	27.1	C	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	54	52	96	8	35.3	D	--
	R	171	170	99	13	40.4	D	--
	Subtotal	226	221	98	--	39.2	D	--
EB	T	571	540	95	17	15.5	B	--
	R	33	31	94	4	12.5	B	--
	Subtotal	603	571	95	--	15.3	B	--
WB	T	329	310	94	22	24.0	C	--
	R	345	309	90	4	44.8	D	--
	Subtotal	674	619	92	--	34.4	C	--
Total		1503	1411	94	--	27.4	C	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	52	51	98	4	1.6	A	--
	Subtotal	52	51	98	--	1.6	A	--
EB	T	674	639	95	22	1.6	A	--
	R	68	69	101	9	0.9	A	--
	Subtotal	742	708	95	--	1.5	A	--
WB	T	622	580	93	22	10.2	B	--
	R	8	9	113	3	11.2	B	--
	Subtotal	630	589	93	--	10.2	B	--
Total		1424	1347	95	--	5.3	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	209	192	92	13	54.0	D	--
	T	57	55	96	9	33.3	C	--
	R	27	26	96	6	25.2	C	--
	Subtotal	293	273	93	--	47.1	D	--
SB	L	27	27	100	3	63.7	E	--
	T	68	69	101	9	52.2	D	--
	R	95	92	97	10	49.0	D	--
	Subtotal	190	188	99	--	52.3	D	--
EB	L	147	135	92	6	52.8	D	--
	T	378	370	98	21	23.5	C	--
	R	149	140	94	8	5.2	A	--
	Subtotal	674	644	96	--	25.7	C	--
WB	L	33	32	97	6	66.9	E	--
	T	326	314	96	15	73.6	E	--
	R	27	24	89	5	37.7	D	--
	Subtotal	386	370	96	--	70.7	E	--
Total	1543	1475	96	--	44.3	D	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	14	13	93	4	65.5	E	--
	T	111	110	99	9	37.1	D	--
	R	3	2	67	1	15.6	B	--
	Subtotal	128	125	98	--	39.7	D	--
SB	L	46	46	100	9	39.3	D	--
	T	125	119	95	8	20.0	C	--
	R	54	53	98	5	6.5	A	--
	U	46	46	100	5	42.7	D	--
Subtotal	272	264	97	--	24.6	C	--	
EB	L	111	103	93	12	43.7	D	--
	T	16	17	106	5	23.9	C	--
	R	14	13	93	3	5.3	A	--
	Subtotal	141	133	94	--	37.2	D	--
WB	L	3	2	67	2	47.6	D	--
	T	16	16	100	5	46.3	D	--
	R	27	29	107	5	26.9	C	--
	Subtotal	46	47	102	--	34.5	C	--
Total	587	569	97	--	31.7	C	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	24	26	108	7	11.7	B	--
	Subtotal	24	26	108	--	11.7	B	--
SB	R	3	4	133	2	3.5	A	--
	Subtotal	3	4	133	--	3.5	A	--
EB	L	3	3	100	2	5.8	A	--
	T	293	264	90	19	1.6	A	--
	R	3	2	67	1	1.2	A	--
	Subtotal	299	269	90	--	1.6	A	--
WB	L	33	27	82	5	21.3	C	--
	T	158	155	97	9	6.6	A	--
	R	14	14	100	4	2.9	A	--
	Subtotal	204	195	96	--	8.4	A	--
Total		530	494	93	--	4.8	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	1774	--	--	1118	--	--
	R	200	1774	Yes	--	1118	Yes	--
SB	L	165	896	Yes	--	675	Yes	--
	T	406	896	Yes	--	675	Yes	--
WB	L	370	436	Yes	--	303	--	--
	R	370	197	--	--	73	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	715	--	--	505	--	--
	T	1427	705	--	--	498	--	--
	R	350	349	--	--	112	--	--
EB	T	637	184	--	--	121	--	--
	R	250	141	--	--	82	--	--
WB	T	928	329	--	--	207	--	--
	R	175	79	--	--	14	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	127	--	--	73	--	--
	T	1538	127	--	--	73	--	--
	R	450	276	--	--	196	--	--
EB	T	75	444	Yes	--	266	Yes	--
	R	75	443	Yes	--	265	Yes	--
WB	T	503	1037	Yes	--	490	--	--
	R	503	1283	Yes	--	762	Yes	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	R	758	46	--	--	7	--	--
WB	T	437	472	Yes	--	185	--	--
	R	150	124	--	--	20	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	313	--	--	218	--	--
	T	562	173	--	--	98	--	--
	R	562	170	--	--	98	--	--
SB	L	150	233	Yes	--	91	--	--
	T	328	467	Yes	--	251	--	--
	R	200	352	Yes	--	236	Yes	--
EB	L	500	286	--	--	210	--	--
	T	437	315	--	--	237	--	--
	R	300	49	--	--	9	--	--
WB	L	300	192	--	--	85	--	--
	T	1119	689	--	--	502	--	--
	R	300	302	Yes	--	124	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	130	--	--	56	--	--
	T	1101	242	--	--	166	--	--
	R	1101	207	--	--	127	--	--
SB	L	200	581	Yes	--	203	Yes	--
	T	498	601	Yes	--	175	--	--
	R	498	432	--	--	106	--	--
EB	U	200	192	--	--	128	--	--
	L	2396	226	--	--	155	--	--
	T	2396	79	--	--	33	--	--
WB	R	200	37	--	--	16	--	--
	L	200	28	--	--	9	--	--
	T	901	193	--	--	109	--	--
	R	901	193	--	--	109	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	92	--	--	48	--	--
SB	R	403	33	--	--	14	--	--
EB	L	100	32	--	--	7	--	--
WB	L	250	125	--	--	57	--	--
	T	637	67	--	--	16	--	--
	R	637	9	--	--	0	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	111	98	88	8	55.8	E	--
	R	101	102	101	8	25.1	C	--
	Subtotal	212	200	94	--	40.1	D	--
SB	L	92	93	101	9	39.4	D	--
	T	46	46	100	8	40.9	D	--
	Subtotal	139	139	100	--	39.9	D	--
WB	L	149	145	97	10	46.0	D	--
	R	136	137	101	13	22.8	C	--
	Subtotal	285	282	99	--	34.7	C	--
Total		636	620	97	--	37.6	D	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	209	209	100	19	28.0	C	--
	R	35	35	100	7	16.0	B	--
	Subtotal	245	245	100	--	26.3	C	--
EB	T	120	116	97	9	17.2	B	--
	R	101	104	103	9	6.4	A	--
	Subtotal	220	220	100	--	12.1	B	--
WB	T	272	263	96	20	28.6	C	--
	R	101	90	89	12	13.1	B	--
	Subtotal	372	352	95	--	24.6	C	--
Total		837	817	98	--	21.8	C	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	168	167	99	13	36.1	D	--
	R	130	125	96	15	13.0	B	--
	Subtotal	299	292	98	--	26.2	C	--
EB	T	280	277	99	16	13.6	B	--
	R	49	48	98	5	12.7	B	--
	Subtotal	329	325	99	--	13.4	B	--
WB	T	204	186	91	11	15.5	B	--
	R	429	365	85	11	12.6	B	--
	Subtotal	633	551	87	--	13.6	B	--
Total	1261	1168	93	--	16.7	B	--	

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	8	8	100	4	1.0	A	--
	Subtotal	8	8	100	--	1.0	A	--
EB	T	391	380	97	20	2.4	A	--
	R	19	18	95	4	1.7	A	--
	Subtotal	410	399	97	--	2.4	A	--
WB	T	625	548	88	15	6.1	A	--
	R	11	10	91	2	5.3	A	--
	Subtotal	636	559	88	--	6.1	A	--
Total	1054	966	92	--	4.5	A	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	117	104	89	6	38.7	D	--
	T	16	20	125	4	31.4	C	--
	R	11	11	100	2	12.4	B	--
	Subtotal	144	135	94	--	35.5	D	--
SB	L	14	12	86	4	83.5	F	--
	T	24	24	100	4	80.3	F	--
	R	114	102	89	4	110.0	F	--
	Subtotal	152	138	91	--	102.5	F	--
EB	L	120	110	92	6	48.5	D	--
	T	223	220	99	17	11.7	B	--
	R	49	50	102	9	2.7	A	--
	Subtotal	391	380	97	--	21.2	C	--
WB	L	5	5	100	2	77.4	E	--
	T	405	355	88	13	105.9	F	--
	R	5	5	80	3	75.7	E	--
	Subtotal	416	365	88	--	105.1	F	--
Total	1103	1018	92	--	64.2	E	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	8	8	100	2	55.7	E	--
	T	52	51	98	4	18.3	B	--
	R	3	3	100	1	3.3	A	--
	Subtotal	63	62	100	--	22.5	C	--
SB	L	16	15	94	4	47.3	D	--
	T	43	48	112	7	8.2	A	--
	R	11	12	109	3	4.3	A	--
	U	11	9	82	3	49.5	D	--
Subtotal	82	84	102	--	19.0	B	--	
EB	L	22	20	91	3	48.7	D	--
	T	3	3	100	2	38.6	D	--
	R	5	5	100	2	2.5	A	--
	Subtotal	30	28	93	--	39.4	D	--
WB	L	3	4	133	2	60.6	E	--
	T	8	9	113	3	36.7	D	--
	R	54	51	94	6	14.8	B	--
	Subtotal	65	63	97	--	20.5	C	--
Total	239	238	100	--	22.7	C	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	33	32	97	5	8.6	A	--
	Subtotal	33	32	97	--	8.6	A	--
SB	R	3	3	100	1	12.0	B	--
	Subtotal	3	3	100	--	12.0	B	--
EB	L	3	3	100	1	12.5	B	--
	T	188	190	101	10	1.3	A	--
	R	3	3	100	2	1.1	A	--
	Subtotal	193	195	101	--	1.5	A	--
WB	L	19	18	95	4	13.9	B	--
	T	283	276	98	13	8.3	A	--
	R	5	6	120	2	4.0	A	--
	Subtotal	307	299	97	--	8.6	A	--
Total		535	529	99	--	6.0	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	644	--	--	357	--	--
	R	200	229	Yes	--	151	--	--
SB	L	165	334	Yes	--	218	Yes	--
	T	406	334	--	--	218	--	--
WB	L	370	571	Yes	--	398	Yes	--
	R	370	547	Yes	--	275	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	315	--	--	227	--	--
	T	1427	285	--	--	204	--	--
	R	350	96	--	--	57	--	--
EB	T	637	187	--	--	91	--	--
	R	250	85	--	--	45	--	--
WB	T	1048	508	--	--	309	--	--
	R	175	182	Yes	--	43	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	279	--	--	192	--	--
	T	1535	259	--	--	175	--	--
	R	450	148	--	--	96	--	--
EB	T	1048	364	--	--	181	--	--
	R	1048	360	--	--	179	--	--
WB	T	710	330	--	--	119	--	--
	R	710	200	--	--	74	--	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
WB	T	437	134	--	--	30	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	185	--	--	114	--	--
	T	562	82	--	--	43	--	--
	R	562	82	--	--	43	--	--
SB	L	150	425	Yes	--	237	Yes	--
	T	328	758	Yes	--	547	Yes	--
	R	200	599	Yes	--	434	Yes	--
EB	L	500	240	--	--	173	--	--
	T	437	178	--	--	104	--	--
WB	L	300	44	--	--	18	--	--
	T	1119	1045	--	--	793	--	--
	R	300	181	--	--	40	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	76	--	--	36	--	--
	T	1101	111	--	--	63	--	--
	R	1101	73	--	--	31	--	--
SB	L	200	85	--	--	45	--	--
	T	498	56	--	--	25	--	--
	R	498	33	--	--	8	--	--
EB	U	200	72	--	--	41	--	--
	L	2396	85	--	--	48	--	--
	T	2396	22	--	--	6	--	--
WB	R	200	21	--	--	8	--	--
	L	200	37	--	--	17	--	--
	T	901	216	--	--	98	--	--
	R	901	216	--	--	98	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: AM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	78	--	--	49	--	--
SB	R	403	32	--	--	10	--	--
EB	L	100	32	--	--	8	--	--
	T	370	8	--	--	1	--	--
WB	R	370	8	--	--	1	--	--
	L	250	69	--	--	28	--	--
	T	637	170	--	--	72	--	--
	R	637	57	--	--	9	--	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	T	111	106	95	12	62.1	E	--
	R	147	138	94	14	39.3	D	--
	Subtotal	258	244	95	--	49.2	D	--
SB	L	152	138	91	10	85.0	F	--
	T	65	65	100	6	89.0	F	--
	Subtotal	217	203	94	--	86.3	F	--
WB	L	109	96	88	8	55.2	E	--
	R	52	45	87	5	13.2	B	--
	Subtotal	160	142	89	--	41.8	D	--
	Total	636	589	93	--	60.2	E	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	L	459	453	99	19	27.6	C	--
	R	52	45	87	4	17.0	B	--
	Subtotal	511	498	97	--	26.6	C	--
EB	T	144	131	91	20	36.5	D	--
	R	174	160	92	14	11.9	B	--
	Subtotal	318	291	92	--	23.0	C	--
WB	T	152	139	91	15	49.9	D	--
	R	171	154	90	10	7.7	A	--
	Subtotal	323	293	91	--	27.7	C	--
	Total	1152	1082	94	--	25.9	C	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	54	51	93	9	35.3	D	--
	R	171	169	99	10	53.5	D	--
	Subtotal	226	220	97	--	49.3	D	--
EB	T	571	555	97	19	20.7	C	--
	R	33	29	88	7	23.6	C	--
	Subtotal	603	584	97	--	20.8	C	--
WB	T	269	246	91	11	14.6	B	--
	R	405	366	90	9	15.0	B	--
	Subtotal	674	612	91	--	14.8	B	--
Total		1503	1416	94	--	22.6	C	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
SB	R	52	50	96	7	1.8	A	--
	Subtotal	52	50	96	--	1.8	A	--
EB	T	674	656	97	21	4.8	A	--
	R	68	64	94	9	4.6	A	--
	Subtotal	742	720	97	--	4.8	A	--
WB	T	622	566	91	13	7.6	A	--
	R	8	7	88	3	5.5	A	--
	Subtotal	630	573	91	--	7.5	A	--
Total		1424	1344	94	--	5.9	A	--

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	209	193	92	15	63.3	E	--
	T	57	55	95	7	34.6	C	--
	R	27	24	89	5	23.5	C	--
	Subtotal	293	271	92	--	54.1	D	--
SB	L	27	27	100	4	67.0	E	--
	T	68	64	94	8	56.7	E	--
	R	95	91	95	11	50.9	D	--
	Subtotal	190	182	96	--	55.4	E	--
EB	L	147	134	91	12	53.1	D	--
	T	378	374	99	19	23.1	C	--
	R	149	150	101	11	5.4	A	--
	Subtotal	674	658	98	--	25.2	C	--
WB	L	33	29	88	5	70.2	E	--
	T	326	292	90	9	111.5	F	--
	R	27	25	93	4	70.0	E	--
	Subtotal	386	346	90	--	105.1	F	--
Total	1543	1457	94	--	53.3	D	--	

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	L	14	14	100	4	61.3	E	--
	T	111	104	94	9	36.0	D	--
	R	3	3	100	2	21.0	C	--
	Subtotal	128	120	94	--	38.6	D	--
SB	L	46	42	91	7	42.1	D	--
	T	125	123	98	10	20.0	B	--
	R	54	52	96	7	6.9	A	--
	U	46	47	102	5	45.3	D	--
Subtotal	272	264	97	--	25.5	C	--	
EB	L	111	106	95	9	44.5	D	--
	T	16	20	125	5	27.6	C	--
	R	14	13	93	2	4.1	A	--
	Subtotal	141	139	99	--	38.3	D	--
WB	L	3	1	33	1	55.9	E	--
	T	16	17	100	4	53.6	D	--
	R	27	29	107	4	26.3	C	--
	Subtotal	46	47	102	--	36.8	D	--
Total	587	570	97	--	32.3	C	--	

SIMTRAFFIC LEVEL OF SERVICE REPORT Including Upstream Delays

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Demand Volume	Volume Served			Delay/Veh (sec)		
			Avg	%	Std Dev	Avg	LOS	Std Dev
NB	R	24	23	96	6	12.4	B	--
	Subtotal	24	23	96	--	12.4	B	--
SB	R	3	2	67	1	3.7	A	--
	Subtotal	3	2	67	--	3.7	A	--
EB	L	3	3	100	2	6.1	A	--
	T	293	269	92	21	1.6	A	--
	R	3	3	100	2	1.1	A	--
	Subtotal	299	275	92	--	1.6	A	--
WB	L	33	27	82	7	23.4	C	--
	T	158	144	91	9	5.6	A	--
	R	14	13	93	5	3.0	A	--
	Subtotal	204	184	90	--	8.0	A	--
Total		530	484	91	--	4.6	A	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 9: Consumnes River Blvd & Freeport Blvd **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	T	2154	940	--	--	562	--	--
	R	200	240	Yes	--	199	--	--
SB	L	165	1027	Yes	--	770	Yes	--
	T	406	1027	Yes	--	770	Yes	--
WB	L	370	379	Yes	--	259	--	--
	R	370	131	--	--	58	--	--

Intersection: 10: Consumnes River Blvd & I-5 SB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
SB	L	1427	586	--	--	384	--	--
	T	1427	577	--	--	365	--	--
	R	350	142	--	--	57	--	--
EB	T	637	279	--	--	158	--	--
	R	250	192	--	--	96	--	--
WB	T	1048	333	--	--	238	--	--
	R	175	158	--	--	40	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 11: Consumnes River Blvd & I-5 NB Ramps **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	105	--	--	63	--	--
	T	1539	98	--	--	52	--	--
	R	450	337	--	--	245	--	--
EB	T	1048	674	--	--	423	--	--
	R	1048	672	--	--	423	--	--
WB	T	711	486	--	--	192	--	--
	R	711	388	--	--	135	--	--

Intersection: 12: Consumnes River Blvd & Driveway **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
WB	T	437	175	--	--	70	--	--
	R	150	18	--	--	3	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 13: Consumnes River Blvd & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	450	385	--	--	251	--	--
	T	562	168	--	--	97	--	--
	R	562	167	--	--	95	--	--
SB	L	150	253	Yes	--	106	--	--
	T	328	498	Yes	--	273	--	--
	R	200	390	Yes	--	255	Yes	--
EB	L	500	277	--	--	210	--	--
	T	437	334	--	--	237	--	--
	R	300	55	--	--	8	--	--
WB	L	300	195	--	--	68	--	--
	T	1119	975	--	--	658	--	--
	R	300	300	Yes	--	121	--	--

Intersection: 19: Driveway & Delta Shores Circle **Type:** Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	L	200	99	--	--	54	--	--
	T	1101	233	--	--	157	--	--
	R	1101	193	--	--	120	--	--
SB	L	200	666	Yes	--	182	--	--
	T	498	669	Yes	--	158	--	--
	R	498	512	Yes	--	82	--	--
	U	200	201	Yes	--	131	--	--
EB	L	2396	227	--	--	167	--	--
	T	2396	94	--	--	41	--	--
	R	200	37	--	--	15	--	--
WB	L	200	26	--	--	4	--	--
	T	901	229	--	--	107	--	--
	R	901	229	--	--	107	--	--

SIMTRAFFIC QUEUING REPORT Including Upstream Queues

Project: Delta Shores **HCM:** 2000
Scenario: Cumulative With Project - Mitigated 2 **PHF:** 0.92
TOD: PM **Analysis Period:** 15 Minutes **# of Runs:** 10

Intersection: 35: Consumnes River Blvd & West Project Access **Type:** Un-Signalized

Approach	Movement	Storage Length	Maximum Queue (ft)			Average Queue (ft)		
			Avg	> Storage	Std Dev	Avg	> Storage	Std Dev
NB	R	392	78	--	--	43	--	--
SB	R	403	35	--	--	10	--	--
EB	L	100	24	--	--	5	--	--
	T	370	23	--	--	3	--	--
	R	370	23	--	--	3	--	--
WB	L	250	111	--	--	56	--	--
	T	637	25	--	--	5	--	--
	R	637	2	--	--	0	--	--

MANORSIDE DRIVE SOUTH OF MEADOWVIEW ROAD

Direction 1

Page : 1

Time	NR		SB		Combined		Wednesday					
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.						
12:00 01/31	3	5	2	19	5	24						
12:15	1	9	0	18	1	27						
12:30	2	11	2	18	4	29						
12:45	5	11	36	7	16	71	8	18	27	107		
01:00	0	15		3	17		3		32			
01:15	0	12		5	15		5		27			
01:30	3	21		0	8		3		29			
01:45	2	5	9	57	4	12	21	61	6	17	30	118
02:00	1	23		2	28		3		51			
02:15	0	17		1	21		1		38			
02:30	1	16		3	22		4		38			
02:45	9	2	15	71	0	6	31	102	0	8	46	173
03:00	2	18		2	18		4		36			
03:15	0	12		0	31		0		43			
03:30	7	13		2	34		9		47			
03:45	2	11	29	72	0	4	39	122	2	15	68	194
04:00	2	12		3	31		5		43			
04:15	2	13		1	24		3		37			
04:30	2	7		2	20		4		27			
04:45	3	9	16	48	0	6	24	99	3	15	40	147
05:00	5	18		2	30		7		48			
05:15	11	19		5	33		16		52			
05:30	9	19		1	28		10		47			
05:45	10	35	14	70	3	11	33	124	13	46	47	194
06:00	15	15		5	32		20		47			
06:15	14	18		0	42		14		60			
06:30	21	15		6	38		27		53			
06:45	11	61	20	68	5	16	27	139	16	77	47	207
07:00	27	19		6	19		33		38			
07:15	30	10		8	31		38		41			
07:30	30	11		16	18		46		29			
07:45	24	111	9	49	19	49	21	89	43	160	30	138
08:00	22	6		22	19		44		25			
08:15	17	15		6	18		23		33			
08:30	19	8		11	24		30		32			
08:45	14	72	8	37	11	50	10	71	26	122	18	108
09:00	17	7		16	13		33		20			
09:15	12	7		14	15		26		22			
09:30	12	4		20	13		32		17			
09:45	6	47	7	25	7	57	11	52	13	104	18	77
10:00	9	8		8	12		17		20			
10:15	9	8		9	25		18		33			
10:30	4	9		13	12		17		21			
10:45	20	42	4	29	18	48	8	47	38	90	12	86
11:00	11	1		14	15		25		16			
11:15	17	3		19	10		36		13			
11:30	24	2		29	5		53		7			
11:45	13	65	2	8	11	73	5	35	24	138	7	43
Totals	471	570	339	1072	810	1592						
Day Totals		1041		1361		2402						
Split %	58.1%	35.8%	41.8%	64.2%								
Peak Hour	07:00	03:00	10:45	05:45	07:15	05:45						
Volume	111	72	80	145	171	207						
P.H.F.	.92	.62	.68	.86	.92	.86						

FREEFORD BLVD. BETWEEN MEADOWVIEW ROAD
AND STONECREST AVENUE

Start Date: 01/31/2007

File I.D. : FREE-V

Page : 1

Time	Direction 1		Direction 2		Combined		Wednesday					
	NB	SB	NB	SB	NB	SB						
12:00 01/31	24	38	4	41	28	79						
12:15	2	41	0	47	2	88						
12:30	5	51	2	28	7	79						
12:45	0	31	174	5	11	45	161	5	42	89	335	
01:00	1	38	3	52	4	90						
01:15	0	46	4	41	4	87						
01:30	3	41	3	35	6	76						
01:45	7	11	34	159	2	12	44	172	9	23	78	331
02:00	1	49	1	45	2	94						
02:15	1	63	0	36	1	95						
02:30	0	52	1	54	1	106						
02:45	2	4	59	223	4	6	44	179	6	10	103	402
03:00	1	60	1	45	2	105						
03:15	0	60	2	47	2	107						
03:30	2	75	1	37	3	112						
03:45	3	6	66	261	1	5	59	188	4	11	125	449
04:00	0	59	2	58	2	117						
04:15	1	99	1	67	2	126						
04:30	2	58	1	61	3	119						
04:45	2	5	55	231	4	8	50	236	6	13	105	467
05:00	5	84	6	45	11	129						
05:15	7	81	9	50	16	131						
05:30	19	85	7	50	16	135						
05:45	14	45	64	314	16	38	52	197	30	83	116	511
06:00	16	57	19	37	15	94						
06:15	18	46	14	33	12	79						
06:30	25	31	28	23	15	54						
06:45	31	90	15	149	46	127	39	132	77	217	54	281
07:00	35	18	49	29	11	47						
07:15	65	22	41	32	106	54						
07:30	59	21	65	20	124	41						
07:45	67	226	17	78	58	213	27	108	125	439	44	186
08:00	54	25	61	23	115	48						
08:15	47	14	56	33	103	37						
08:30	50	12	42	16	92	28						
08:45	41	192	13	64	34	193	17	79	75	385	30	143
09:00	45	14	21	18	66	32						
09:15	29	14	44	16	73	30						
09:30	39	14	28	16	67	30						
09:45	30	143	7	49	36	129	12	62	66	272	19	111
10:00	34	6	36	13	70	19						
10:15	29	8	30	8	59	16						
10:30	31	6	43	5	74	11						
10:45	44	138	3	23	38	147	4	30	82	285	7	53
11:00	37	4	36	7	73	11						
11:15	43	6	28	5	71	11						
11:30	41	4	48	9	89	13						
11:45	44	165	2	16	55	167	4	25	99	332	6	41
Totals	1056	1741	1056	1569	2112	3310						
Day Totals		2797		2625		5422						
Split %	50.0%	53.6%	50.0%	47.4%								
Peak Hour	07:15	05:00	07:30	03:45	07:15	05:00						
Volume	245	314	240	245	470	511						
P.H.F.	.91	.92	.92	.91	.94	.94						

24TH STREET SOUTH OF MEADOWVIEW ROAD

Direction 1

Page : 1

Origin	NB		SB		Combined		Wednesday					
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.						
12:00 01/31	12	56	7	50	19	106						
12:15	2	61	13	55	15	116						
12:30	3	64	7	40	10	104						
12:45	7	24	55	236	9	35	44	189	16	60	99	425
01:00	6	56	9	59	15	116						
01:15	3	52	8	47	11	99						
01:30	4	38	3	52	7	90						
01:45	1	14	57	203	7	27	63	241	8	41	140	444
02:00	5	105	7	80	12	185						
02:15	2	58	3	64	5	122						
02:30	6	81	4	62	10	143						
02:45	6	19	72	316	2	16	53	265	6	35	135	585
03:00	2	76	5	56	7	132						
03:15	2	59	4	79	6	138						
03:30	3	68	2	92	5	160						
03:45	3	10	65	268	1	12	83	310	4	22	148	578
04:00	10	71	4	68	14	139						
04:15	9	60	3	75	12	135						
04:30	13	85	1	81	14	166						
04:45	7	39	77	293	2	10	75	299	9	49	152	592
05:00	16	68	5	96	21	164						
05:15	17	82	5	74	22	156						
05:30	34	74	9	98	43	162						
05:45	31	38	56	280	6	25	90	348	37	123	146	628
06:00	34	92	9	110	43	202						
06:15	27	67	6	81	33	148						
06:30	49	61	16	77	65	138						
06:45	55	165	48	268	21	52	42	310	76	217	90	578
07:00	68	62	32	55	100	117						
07:15	77	49	39	60	116	109						
07:30	120	50	91	69	211	119						
07:45	155	420	52	213	108	270	47	231	263	690	99	444
08:00	91	40	50	54	141	94						
08:15	52	42	40	42	92	84						
08:30	48	37	23	39	71	76						
08:45	60	251	32	151	51	164	44	179	111	415	76	330
09:00	46	29	34	47	80	76						
09:15	43	36	40	32	83	68						
09:30	44	21	39	37	83	58						
09:45	52	185	27	113	43	156	35	151	95	341	52	264
10:00	36	20	29	26	65	48						
10:15	36	18	34	24	70	42						
10:30	37	30	44	25	81	55						
10:45	37	146	18	86	36	143	21	98	73	289	39	184
11:00	48	10	32	21	80	31						
11:15	59	16	60	19	119	35						
11:30	48	8	40	13	98	21						
11:45	44	199	11	45	46	178	10	63	90	377	21	108
Totals	1570	2472	1089	2688	2659	5160						
Day Totals		4042		3777		7819						
Split %	59.0%	47.9%	40.9%	52.0%								

Peak Hour	07:15	02:00	07:30	05:30	07:15	05:15
Volume	443	316	289	369	731	666
P.H.F.	.71	.75	.66	.83	.69	.82

Time	6B		NB		Combined		Wednesday					
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.						
12:00 01/31	7	27	3	27	10	54						
12:15	8	32	2	23	10	55						
12:30	5	33	1	28	6	61						
12:45	5	25	38	130	6	12	30	108	11	37	68	238
01:00	7	32	1	33	8	65						
01:15	6	48	3	35	9	83						
01:30	7	42	4	44	11	86						
01:45	6	26	53	175	1	9	45	157	7	36	98	332
02:00	3	52	4	51	7	103						
02:15	3	43	5	40	8	83						
02:30	6	49	2	49	8	98						
02:45	5	17	45	189	0	11	53	193	5	28	98	382
03:00	3	39	4	53	7	92						
03:15	5	65	5	40	10	105						
03:30	5	75	4	35	9	110						
03:45	5	18	56	235	5	18	41	169	10	36	97	404
04:00	6	63	5	49	11	112						
04:15	5	60	19	55	24	115						
04:30	1	69	8	49	9	118						
04:45	5	17	63	255	6	38	45	198	11	55	108	453
05:00	5	68	15	41	20	109						
05:15	5	51	19	50	24	101						
05:30	1	67	32	51	33	118						
05:45	11	22	61	247	27	93	43	185	38	115	104	432
06:00	12	56	28	53	40	109						
06:15	7	58	31	41	38	99						
06:30	8	50	26	30	34	80						
06:45	13	40	34	198	42	127	27	151	55	167	61	349
07:00	21	50	32	34	53	94						
07:15	22	41	51	30	73	71						
07:30	48	40	95	25	143	65						
07:45	55	146	37	168	77	255	17	126	132	401	74	294
08:00	50	42	53	20	103	62						
08:15	20	38	53	32	73	70						
08:30	22	30	46	15	68	45						
08:45	11	123	26	136	41	193	19	86	72	316	45	222
09:00	17	31	43	23	60	54						
09:15	32	26	53	23	85	49						
09:30	31	23	36	19	67	42						
09:45	23	103	21	191	26	158	13	78	49	261	34	179
10:00	25	20	18	8	43	28						
10:15	17	24	26	26	43	50						
10:30	15	22	28	16	43	38						
10:45	26	83	12	78	24	96	14	64	50	179	26	142
11:00	15	18	25	8	40	26						
11:15	29	13	17	3	46	16						
11:30	38	12	37	12	75	24						
11:45	28	110	12	55	32	111	8	31	60	221	20	85
Totals	730	1967	1121	1546	1851	3513						
Day Totals		2697		2667		5364						
Split %	39.4%	55.9%	60.5%	44.0%								
Peak Hour	07:15	04:15	07:30	04:00	07:15	04:00						
Volume	175	280	278	198	451	453						
P.H.F.	.79	.94	.73	.9	.78	.95						

All Traffic Data, Inc.

(916) 771-8700
Fax 786-2879

SACRAMENTO

File Name : COSUMNES-FRANKLIN-F
Site Code : 00000000
Start Date : 2/7/2007
Page No : 1

Groups Printed- Unshifted

Start Time	FRANKLIN BLVD. Southbound				COSUMNES RIVER BLVD. Westbound				FRANKLIN BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	70	34	104	80	0	41	121	84	296	0	380	0	0	0	0	605
07:15	0	140	59	199	105	0	42	147	59	353	0	412	0	0	0	0	758
07:30	0	224	103	327	104	0	60	164	72	353	0	425	0	0	0	0	916
07:45	0	224	117	341	89	0	57	146	70	406	0	476	0	0	0	0	963
Total	0	658	313	971	378	0	200	578	285	1408	0	1693	0	0	0	0	3242
08:00	0	103	64	167	94	0	55	149	51	313	0	364	0	0	0	0	680
08:15	0	106	41	147	96	0	58	154	121	383	0	504	0	0	0	0	805
08:30	0	79	48	127	67	0	49	116	86	223	0	309	0	0	0	0	552
08:45	0	79	50	129	71	0	49	120	74	183	0	257	0	0	0	0	506
Total	0	367	203	570	328	0	211	539	332	1102	0	1434	0	0	0	0	2543

*** BREAK ***

16:00	0	244	73	317	108	0	95	203	63	172	0	235	0	0	0	0	755
16:15	0	228	84	312	75	0	82	157	65	147	0	212	0	0	0	0	681
16:30	0	237	104	341	73	0	100	173	73	149	0	222	0	0	0	0	736
16:45	0	297	73	370	82	0	80	162	48	191	0	239	0	0	0	0	771
Total	0	1006	334	1340	338	0	357	695	249	659	0	908	0	0	0	0	2943
17:00	0	278	101	379	90	0	106	196	56	179	0	235	0	0	0	0	810
17:15	0	323	97	420	69	0	93	162	67	164	0	231	0	0	0	0	813
17:30	0	270	90	360	66	0	148	214	70	186	0	256	0	0	0	0	830
17:45	0	317	95	412	83	0	120	203	88	167	0	255	0	0	0	0	870
Total	0	1188	383	1571	308	0	467	775	281	696	0	977	0	0	0	0	3323
Grand Total	0	3219	1233	4452	1352	0	1235	2587	1147	3865	0	5012	0	0	0	0	12051
Apprch %	0	72.3	27.7		52.3	0	47.7		22.9	77.1	0		0	0	0		
Total %	0	26.7	10.2	36.9	11.2	0	10.2	21.5	9.5	32.1	0	41.6	0	0	0	0	

Start Time	FRANKLIN BLVD. Southbound				COSUMNES RIVER BLVD. Westbound				FRANKLIN BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:30	0	224	103	327	104	0	60	164	72	353	0	425	0	0	0	0	916
07:45	0	224	117	341	89	0	57	146	70	406	0	476	0	0	0	0	963
08:00	0	103	64	167	94	0	55	149	51	313	0	364	0	0	0	0	680
08:15	0	106	41	147	96	0	58	154	121	383	0	504	0	0	0	0	805
Total Volume	0	657	325	982	383	0	230	613	314	1455	0	1769	0	0	0	0	3364
% App. Total	0	66.9	33.1		62.5	0	37.5		17.8	82.2	0		0	0	0		
PHF	.000	.733	.694	.720	.921	.000	.958	.934	.649	.896	.000	.877	.000	.000	.000	.000	.873

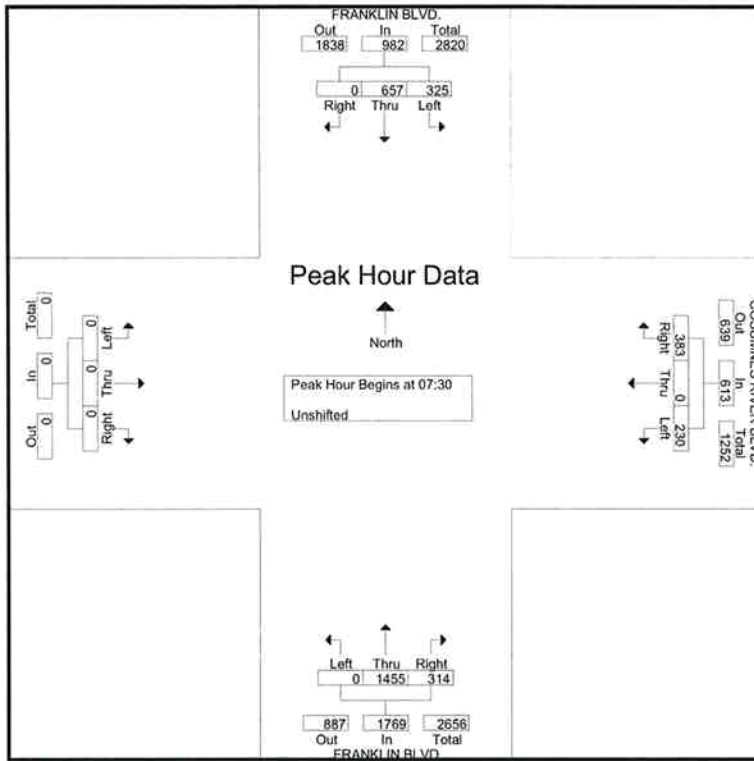
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 07:30

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

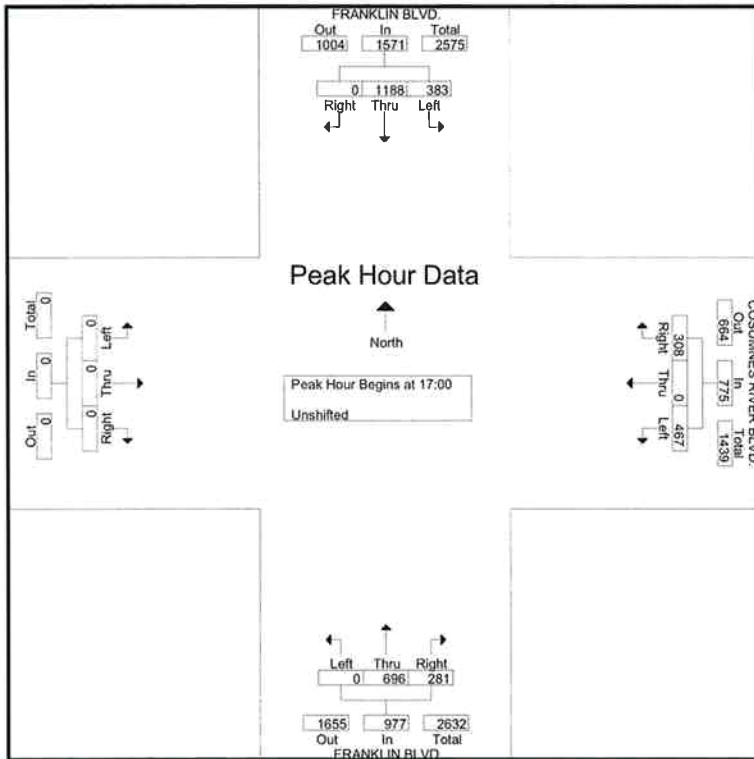
17:00	0	278	101	379	90	0	106	196	56	179	0	235	0	0	0	0	810
17:15	0	323	97	420	69	0	93	162	67	164	0	231	0	0	0	0	813
17:30	0	270	90	360	66	0	148	214	70	186	0	256	0	0	0	0	830
17:45	0	317	95	412	83	0	120	203	88	167	0	255	0	0	0	0	870
Total Volume	0	1188	383	1571	308	0	467	775	281	696	0	977	0	0	0	0	3323
% App. Total	0	75.6	24.4		39.7	0	60.3		28.8	71.2	0		0	0	0		
PHF	.000	.920	.948	.935	.856	.000	.789	.905	.798	.935	.000	.954	.000	.000	.000	.000	.955

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Groups Printed- Unshifted

Start Time	FRANKLIN BLVD. Southbound				MACK RD. Westbound				FRANKLIN BLVD. Northbound				MACK RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00 AM	10	75	21	106	40	132	20	192	38	251	135	424	9	60	20	89	811
07:15 AM	12	81	27	120	48	185	25	258	41	255	144	440	12	65	27	104	922
07:30 AM	15	90	26	131	50	211	28	289	44	249	154	447	19	91	31	141	1008
07:45 AM	20	76	33	129	50	197	40	287	49	244	159	452	28	71	36	135	1003
Total	57	322	107	486	188	725	113	1026	172	999	592	1763	68	287	114	469	3744
08:00 AM	13	48	35	96	48	158	19	225	46	256	155	457	11	59	42	112	890
08:15 AM	8	58	23	89	40	133	19	192	59	227	144	430	10	61	30	101	812
08:30 AM	4	76	28	108	40	124	32	196	33	204	130	367	8	73	27	108	779
08:45 AM	10	76	35	121	44	104	28	176	22	170	114	306	15	78	41	134	737
Total	35	258	121	414	172	519	98	789	160	857	543	1560	44	271	140	455	3218
*** BREAK ***																	
04:00 PM	24	178	91	293	45	199	54	298	35	136	99	270	116	174	28	318	1179
04:15 PM	13	167	76	256	65	179	64	308	41	124	112	277	122	221	32	375	1216
04:30 PM	26	194	88	308	57	180	50	287	27	118	109	254	166	191	26	383	1232
04:45 PM	31	166	75	272	66	195	72	333	32	109	110	251	168	246	31	445	1301
Total	94	705	330	1129	233	753	240	1226	135	487	430	1052	572	832	117	1521	4928
05:00 PM	25	211	93	329	49	205	65	319	39	135	113	287	184	205	25	414	1349
05:15 PM	32	231	73	336	40	208	74	322	42	116	106	264	170	173	23	366	1288
05:30 PM	26	215	85	326	54	167	72	293	39	121	109	269	155	162	21	338	1226
05:45 PM	39	166	64	269	63	194	64	321	41	108	102	251	158	224	29	411	1252
Total	122	823	315	1260	206	774	275	1255	161	480	430	1071	667	764	98	1529	5115
Grand Total	308	2108	873	3289	799	2771	726	4296	628	2823	1995	5446	1351	2154	469	3974	17005
Apprch %	9.4	64.1	26.5		18.6	64.5	16.9		11.5	51.8	36.6		34	54.2	11.8		
Total %	1.8	12.4	5.1	19.3	4.7	16.3	4.3	25.3	3.7	16.6	11.7	32	7.9	12.7	2.8	23.4	

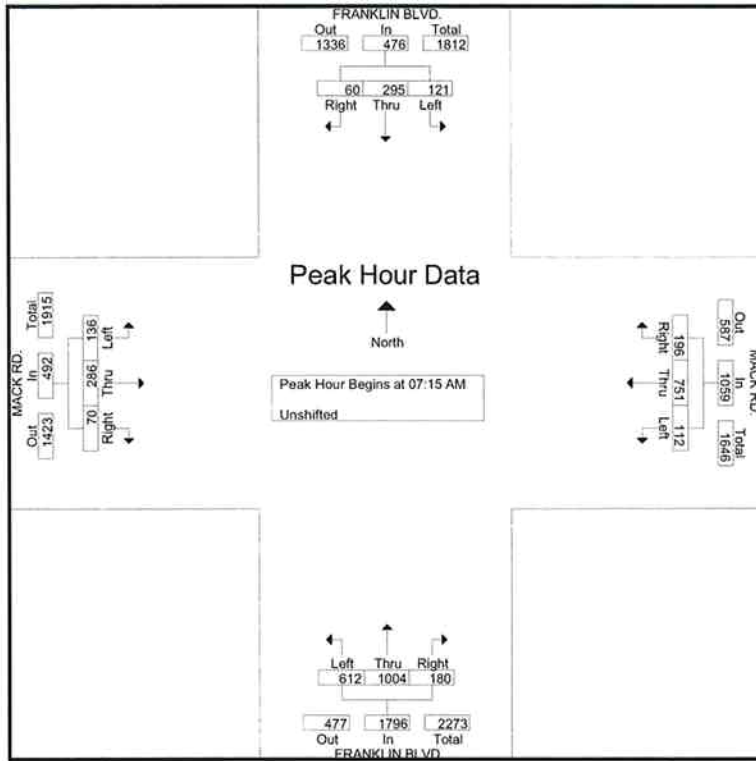
Start Time	FRANKLIN BLVD. Southbound				MACK RD. Westbound				FRANKLIN BLVD. Northbound				MACK RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	12	81	27	120	48	185	25	258	41	255	144	440	12	65	27	104	922
07:30 AM	15	90	26	131	50	211	28	289	44	249	154	447	19	91	31	141	1008
07:45 AM	20	76	33	129	50	197	40	287	49	244	159	452	28	71	36	135	1003
08:00 AM	13	48	35	96	48	158	19	225	46	256	155	457	11	59	42	112	890
Total Volume	60	295	121	476	196	751	112	1059	180	1004	612	1796	70	286	136	492	3823
% App. Total	12.6	62	25.4		18.5	70.9	10.6		10	55.9	34.1		14.2	58.1	27.6		
PHF	.750	.819	.864	.908	.980	.890	.700	.916	.918	.980	.962	.982	.625	.786	.810	.872	.948

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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:30 PM

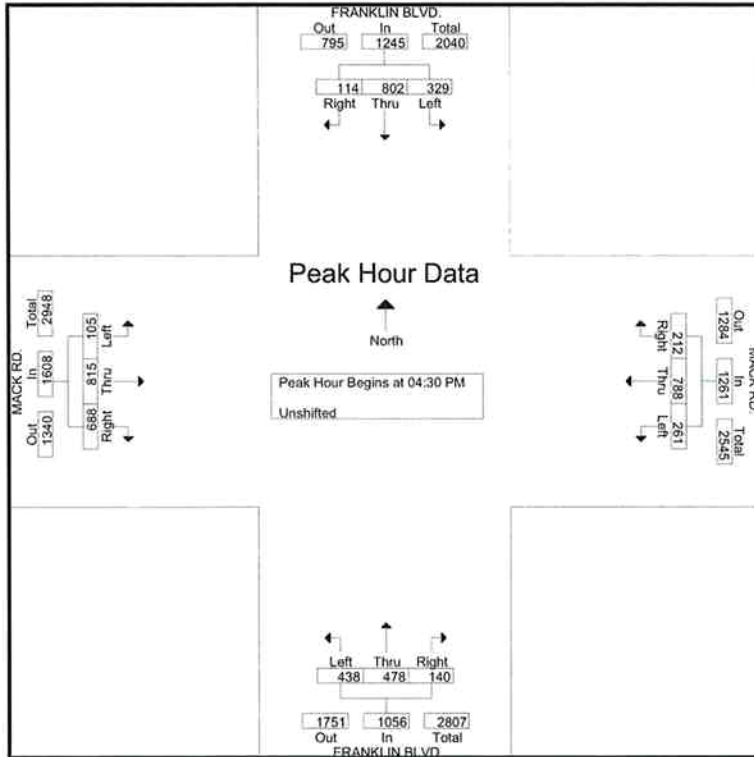
04:30 PM	26	194	88	308	57	180	50	287	27	118	109	254	166	191	26	383	1232
04:45 PM	31	166	75	272	66	195	72	333	32	109	110	251	168	246	31	445	1301
05:00 PM	25	211	93	329	49	205	65	319	39	135	113	287	184	205	25	414	1349
05:15 PM	32	231	73	336	40	208	74	322	42	116	106	264	170	173	23	366	1288
Total Volume	114	802	329	1245	212	788	261	1261	140	478	438	1056	688	815	105	1608	5170
% App. Total	9.2	64.4	26.4		16.8	62.5	20.7		13.3	45.3	41.5		42.8	50.7	6.5		
PHF	.891	.868	.884	.926	.803	.947	.882	.947	.833	.885	.969	.920	.935	.828	.847	.903	.958

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Groups Printed- Unshifted

Start Time	24TH ST. Southbound				MEADOWVIEW RD. Westbound				24TH ST. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	17	20	32	69	52	188	17	257	15	32	13	60	9	145	41	195	581
07:15	19	21	35	75	60	206	27	293	15	40	21	76	11	138	48	197	641
07:30	16	39	38	93	103	245	36	384	35	59	13	107	7	202	42	251	835
07:45	24	40	50	114	116	242	55	413	36	85	20	141	18	164	61	243	911
Total	76	120	155	351	331	881	135	1347	101	216	67	384	45	649	192	886	2968
08:00	31	25	44	100	116	202	21	339	23	42	13	78	17	156	46	219	736
08:15	29	14	47	90	81	186	19	286	17	40	7	64	17	150	32	199	639
08:30	24	20	38	82	75	185	16	276	15	30	8	53	5	128	29	162	573
08:45	18	11	34	63	51	158	11	220	19	35	7	61	10	128	43	181	525
Total	102	70	163	335	323	731	67	1121	74	147	35	256	49	562	150	761	2473

*** BREAK ***

16:00	51	42	101	194	71	167	17	255	34	32	6	72	9	231	40	280	801
16:15	44	54	106	204	59	191	31	281	17	28	9	54	10	235	38	283	822
16:30	45	44	127	216	52	187	32	271	21	34	18	73	14	209	48	271	831
16:45	39	43	107	189	55	194	31	280	25	39	14	78	16	263	45	324	871
Total	179	183	441	803	237	739	111	1087	97	133	47	277	49	938	171	1158	3325
17:00	61	53	133	247	58	166	37	261	17	28	16	61	18	220	34	272	841
17:15	48	74	120	242	55	227	23	305	28	44	5	77	13	256	31	300	924
17:30	66	47	150	263	54	174	34	262	21	30	8	59	10	224	43	277	861
17:45	56	50	160	266	56	194	27	277	24	31	14	69	18	206	52	276	888
Total	231	224	563	1018	223	761	121	1105	90	133	43	266	59	906	160	1125	3514
Grand Total	588	597	1322	2507	1114	3112	434	4660	362	629	192	1183	202	3055	673	3930	12280
Apprch %	23.5	23.8	52.7	20.4	23.9	66.8	9.3	37.9	30.6	53.2	16.2	9.6	5.1	77.7	17.1	32	
Total %	4.8	4.9	10.8	20.4	9.1	25.3	3.5	37.9	2.9	5.1	1.6	9.6	1.6	24.9	5.5	32	

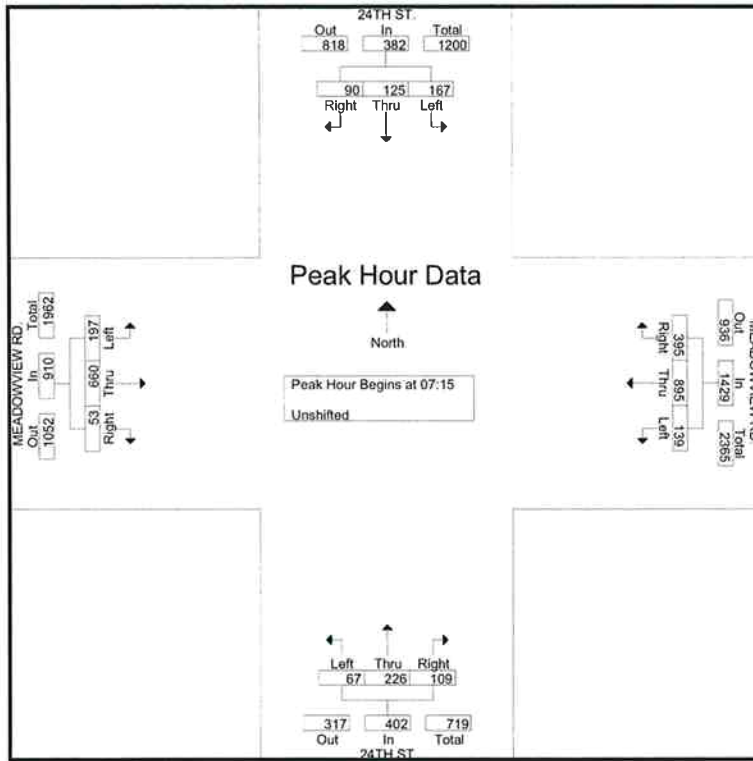
Start Time	24TH ST. Southbound				MEADOWVIEW RD. Westbound				24TH ST. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	19	21	35	75	60	206	27	293	15	40	21	76	11	138	48	197	641
07:30	16	39	38	93	103	245	36	384	35	59	13	107	7	202	42	251	835
07:45	24	40	50	114	116	242	55	413	36	85	20	141	18	164	61	243	911
08:00	31	25	44	100	116	202	21	339	23	42	13	78	17	156	46	219	736
Total Volume	90	125	167	382	395	895	139	1429	109	226	67	402	53	660	197	910	3123
% App. Total	23.6	32.7	43.7	20.4	27.6	62.6	9.7	37.9	27.1	56.2	16.7	9.6	5.8	72.5	21.6	32	
PHF	.726	.781	.835	.838	.851	.913	.632	.865	.757	.665	.798	.713	.736	.817	.807	.906	.857

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

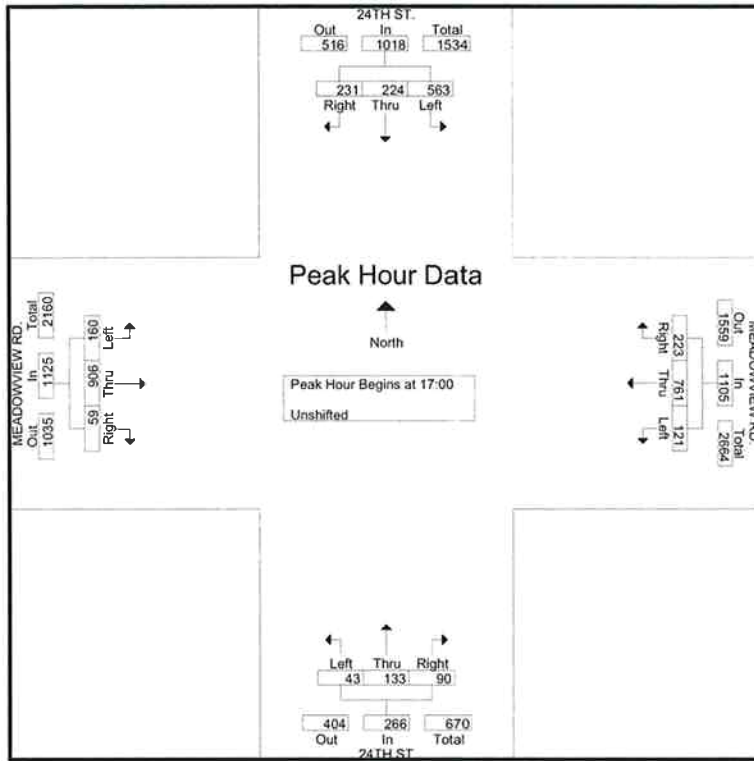
17:00	61	53	133	247	58	166	37	261	17	28	16	61	18	220	34	272	841
17:15	48	74	120	242	55	227	23	305	28	44	5	77	13	256	31	300	924
17:30	66	47	150	263	54	174	34	262	21	30	8	59	10	224	43	277	861
17:45	56	50	160	266	56	194	27	277	24	31	14	69	18	206	52	276	888
Total Volume	231	224	563	1018	223	761	121	1105	90	133	43	266	59	906	160	1125	3514
% App. Total	22.7	22	55.3		20.2	68.9	11		33.8	50	16.2		5.2	80.5	14.2		
PHF	.875	.757	.880	.957	.961	.838	.818	.906	.804	.756	.672	.864	.819	.885	.769	.938	.951

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Groups Printed- Unshifted

Start Time	DETROIT BLVD. Southbound				MEADOWVIEW RD. Westbound				DETROIT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	6	0	8	14	35	292	11	338	26	1	15	42	9	195	10	214	608
07:15	10	2	12	24	76	339	14	429	35	1	18	54	11	180	16	207	714
07:30	4	1	16	21	65	369	33	467	49	1	45	95	25	268	11	304	887
07:45	12	0	10	22	42	380	27	449	50	1	43	94	34	223	6	263	828
Total	32	3	46	81	218	1380	85	1683	160	4	121	285	79	866	43	988	3037
08:00	6	0	13	19	46	347	29	422	31	0	25	56	18	228	13	259	756
08:15	6	0	9	15	21	255	18	294	31	2	21	54	9	198	3	210	573
08:30	5	1	9	15	35	282	13	330	30	1	11	42	5	207	3	215	602
08:45	3	0	7	10	19	234	12	265	24	1	11	36	7	198	6	211	522
Total	20	1	38	59	121	1118	72	1311	116	4	68	188	39	831	25	895	2453
*** BREAK ***																	
16:00	18	29	45	92	19	252	33	304	36	10	17	63	21	346	15	382	841
16:15	14	1	56	71	16	253	42	311	34	0	16	50	26	361	4	391	823
16:30	13	1	52	66	11	258	36	305	38	0	19	57	23	332	7	362	790
16:45	12	0	57	69	17	266	52	335	36	1	25	62	26	304	6	336	802
Total	57	31	210	298	63	1029	163	1255	144	11	77	232	96	1343	32	1471	3256
17:00	11	3	80	94	18	287	44	349	30	1	17	48	15	360	6	381	872
17:15	14	1	61	76	27	308	36	371	39	0	17	56	20	427	4	451	954
17:30	16	1	70	87	13	260	38	311	34	0	16	50	16	361	4	381	829
17:45	14	0	78	92	15	284	39	338	32	1	14	47	20	338	7	365	842
Total	55	5	289	349	73	1139	157	1369	135	2	64	201	71	1486	21	1578	3497
Grand Total	164	40	583	787	475	4666	477	5618	555	21	330	906	285	4526	121	4932	12243
Apprch %	20.8	5.1	74.1		8.5	83.1	8.5		61.3	2.3	36.4		5.8	91.8	2.5		
Total %	1.3	0.3	4.8	6.4	3.9	38.1	3.9	45.9	4.5	0.2	2.7	7.4	2.3	37	1	40.3	

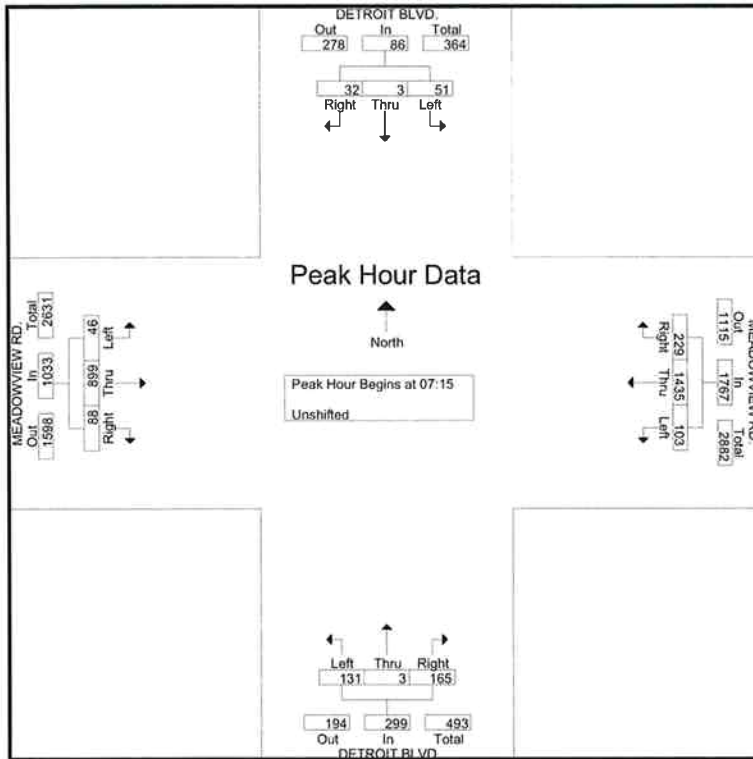
Start Time	DETROIT BLVD. Southbound				MEADOWVIEW RD. Westbound				DETROIT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	10	2	12	24	76	339	14	429	35	1	18	54	11	180	16	207	714
07:30	4	1	16	21	65	369	33	467	49	1	45	95	25	268	11	304	887
07:45	12	0	10	22	42	380	27	449	50	1	43	94	34	223	6	263	828
08:00	6	0	13	19	46	347	29	422	31	0	25	56	18	228	13	259	756
Total Volume	32	3	51	86	229	1435	103	1767	165	3	131	299	88	899	46	1033	3185
% App. Total	37.2	3.5	59.3		13	81.2	5.8		55.2	1	43.8		8.5	87	4.5		
PHF	.667	.375	.797	.896	.753	.944	.780	.946	.825	.750	.728	.787	.647	.839	.719	.850	.898

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

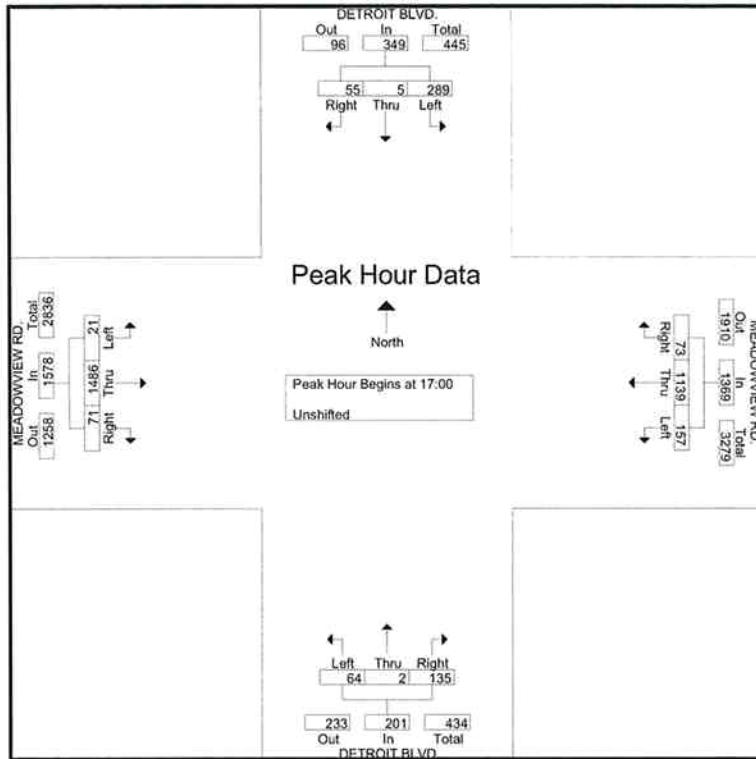
17:00	11	3	80	94	18	287	44	349	30	1	17	48	15	360	6	381	872
17:15	14	1	61	76	27	308	36	371	39	0	17	56	20	427	4	451	954
17:30	16	1	70	87	13	260	38	311	34	0	16	50	16	361	4	381	829
17:45	14	0	78	92	15	284	39	338	32	1	14	47	20	338	7	365	842
Total Volume	55	5	289	349	73	1139	157	1369	135	2	64	201	71	1486	21	1578	3497
% App. Total	15.8	1.4	82.8		5.3	83.2	11.5		67.2	1	31.8		4.5	94.2	1.3		
PHF	.859	.417	.903	.928	.676	.925	.892	.923	.865	.500	.941	.897	.888	.870	.750	.875	.916

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Groups Printed- Un-shifted

Start Time	FREEPORT BLVD. Southbound				MEADOWVIEW RD. Westbound				FREEPORT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	7	22	14	43	50	215	8	273	9	15	18	42	20	150	83	253	611
07:15	5	29	19	53	78	184	10	272	15	41	24	80	29	174	100	303	708
07:30	12	44	16	72	81	187	14	282	12	37	14	63	34	198	119	351	768
07:45	14	26	18	58	59	204	7	270	16	26	20	62	33	208	114	355	745
Total	38	121	67	226	268	790	39	1097	52	119	76	247	116	730	416	1262	2832
08:00	16	18	25	59	61	197	7	265	14	22	30	66	29	211	82	322	712
08:15	8	28	24	60	58	150	10	218	6	23	23	52	21	175	72	268	598
08:30	4	18	19	41	66	137	10	213	9	26	22	57	21	118	67	206	517
08:45	5	18	14	37	53	139	10	202	8	19	20	47	23	101	53	177	463
Total	33	82	82	197	238	623	37	898	37	90	95	222	94	605	274	973	2290

*** BREAK ***

16:00	45	21	101	167	54	193	9	256	14	15	34	63	21	205	32	258	744
16:15	55	24	100	179	47	229	21	297	18	13	29	60	33	247	32	312	848
16:30	45	19	89	153	38	228	11	277	15	15	34	64	25	262	40	327	821
16:45	59	31	93	183	53	181	14	248	21	21	38	80	33	228	42	303	814
Total	204	95	383	682	192	831	55	1078	68	64	135	267	112	942	146	1200	3227
17:00	85	21	82	188	54	193	8	255	13	17	31	61	32	232	45	309	813
17:15	111	15	112	238	51	181	9	241	22	21	44	87	43	250	31	324	890
17:30	66	14	89	169	39	168	9	216	16	15	52	83	20	241	31	292	760
17:45	80	17	86	183	55	200	13	268	16	8	46	70	27	252	28	307	828
Total	342	67	369	778	199	742	39	980	67	61	173	301	122	975	135	1232	3291
Grand Total	617	365	901	1883	897	2986	170	4053	224	334	479	1037	444	3252	971	4667	11640
Approch %	32.8	19.4	47.8		22.1	73.7	4.2		21.6	32.2	46.2		9.5	69.7	20.8		
Total %	5.3	3.1	7.7	16.2	7.7	25.7	1.5	34.8	1.9	2.9	4.1	8.9	3.8	27.9	8.3	40.1	

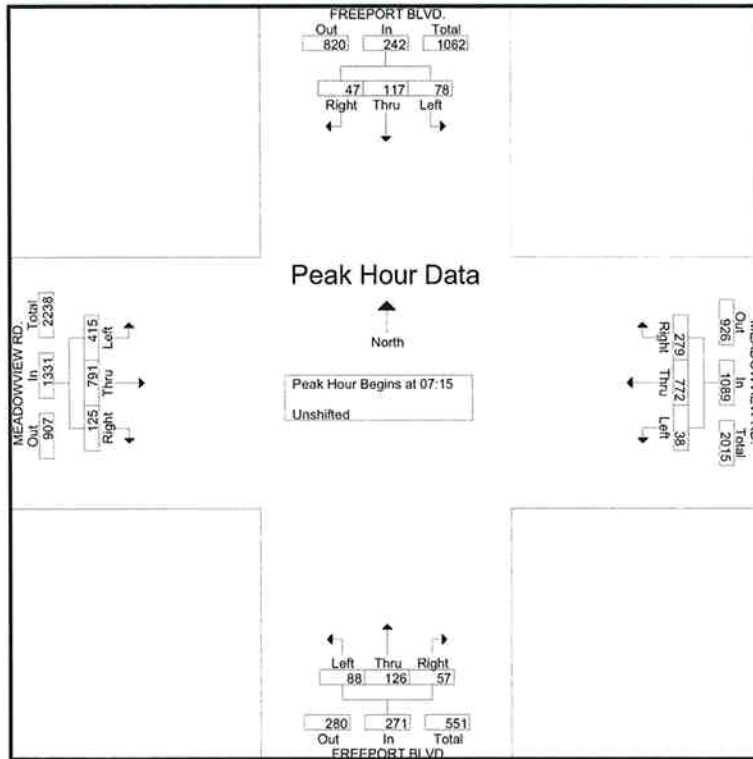
Start Time	FREEPORT BLVD. Southbound				MEADOWVIEW RD. Westbound				FREEPORT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	5	29	19	53	78	184	10	272	15	41	24	80	29	174	100	303	708
07:30	12	44	16	72	81	187	14	282	12	37	14	63	34	198	119	351	768
07:45	14	26	18	58	59	204	7	270	16	26	20	62	33	208	114	355	745
08:00	16	18	25	59	61	197	7	265	14	22	30	66	29	211	82	322	712
Total Volume	47	117	78	242	279	772	38	1089	57	126	88	271	125	791	415	1331	2933
% App. Total	19.4	48.3	32.2		25.6	70.9	3.5		21	46.5	32.5		9.4	59.4	31.2		
PHF	.734	.665	.780	.840	.861	.946	.679	.965	.891	.768	.733	.847	.919	.937	.872	.937	.955

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 16:30

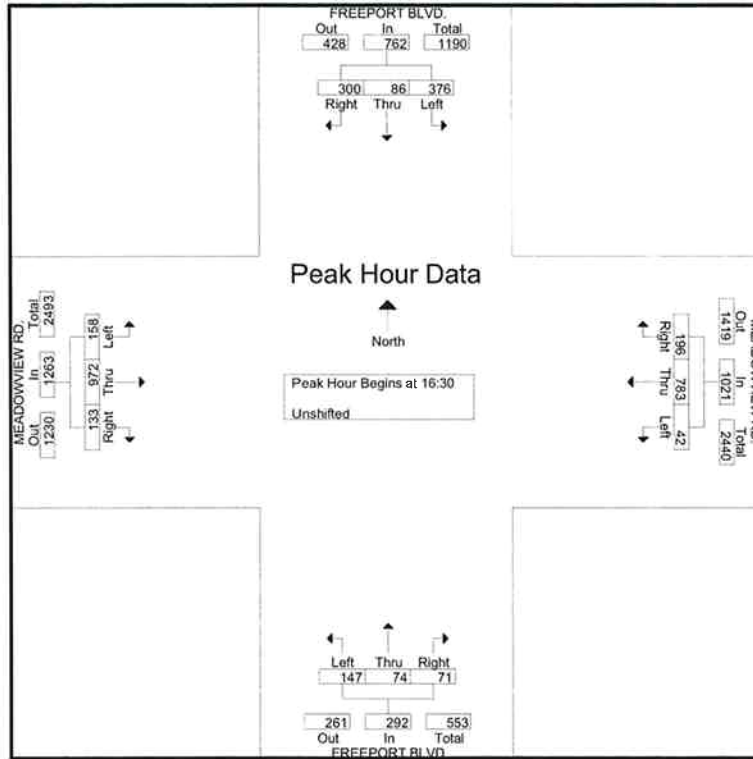
16:30	45	19	89	153	38	228	11	277	15	15	34	64	25	262	40	327	821
16:45	59	31	93	183	53	181	14	248	21	21	38	80	33	228	42	303	814
17:00	85	21	82	188	54	193	8	255	13	17	31	61	32	232	45	309	813
17:15	111	15	112	238	51	181	9	241	22	21	44	87	43	250	31	324	890
Total Volume	300	86	376	762	196	783	42	1021	71	74	147	292	133	972	158	1263	3338
% App. Total	39.4	11.3	49.3		19.2	76.7	4.1		24.3	25.3	50.3		10.5	77	12.5		
PHF	.676	.694	.839	.800	.907	.859	.750	.921	.807	.881	.835	.839	.773	.927	.878	.966	.938

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File Name : MEADOWVIEW-MANORSIDE-F
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Groups Printed- Unshifted

Start Time	Southbound				MEADOWVIEW RD. Westbound				MANORSIDE DR. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	0	0	0	0	237	5	242	12	0	11	23	3	152	0	155	420
07:15	0	0	0	0	0	250	5	255	20	0	15	35	6	196	0	202	492
07:30	0	0	0	0	0	232	15	247	18	0	16	34	12	231	0	243	524
07:45	0	0	0	0	0	267	6	273	13	0	21	34	14	255	0	269	576
Total	0	0	0	0	0	986	31	1017	63	0	63	126	35	834	0	869	2012
08:00	0	0	0	0	0	258	6	264	13	0	16	29	19	235	0	254	547
08:15	0	0	0	0	0	224	10	234	5	0	6	11	12	187	0	199	444
08:30	0	0	0	0	0	174	9	183	7	0	5	12	10	139	0	149	344
08:45	0	0	0	0	0	195	3	198	10	0	10	20	4	127	0	131	349
Total	0	0	0	0	0	851	28	879	35	0	37	72	45	688	0	733	1684
*** BREAK ***																	
16:00	0	0	0	0	0	238	14	252	9	0	11	20	13	295	0	308	580
16:15	0	0	0	0	0	294	12	306	11	0	6	17	15	284	0	299	622
16:30	0	0	0	0	0	235	21	256	10	0	7	17	23	291	0	314	587
16:45	0	0	0	0	0	219	17	236	10	0	5	15	12	261	0	273	524
Total	0	0	0	0	0	986	64	1050	40	0	29	69	63	1131	0	1194	2313
17:00	0	0	0	0	0	237	22	259	15	0	5	20	16	288	0	304	583
17:15	0	0	0	0	0	230	14	244	7	0	6	13	13	297	0	310	567
17:30	0	0	0	0	0	212	17	229	10	0	11	21	11	299	0	310	560
17:45	0	0	0	0	0	260	14	274	8	0	3	11	14	304	0	318	603
Total	0	0	0	0	0	939	67	1006	40	0	25	65	54	1188	0	1242	2313
Grand Total	0	0	0	0	0	3762	190	3952	178	0	154	332	197	3841	0	4038	8322
Apprch %	0	0	0	0	0	95.2	4.8		53.6	0	46.4		4.9	95.1	0		
Total %	0	0	0	0	0	45.2	2.3	47.5	2.1	0	1.9	4	2.4	46.2	0	48.5	

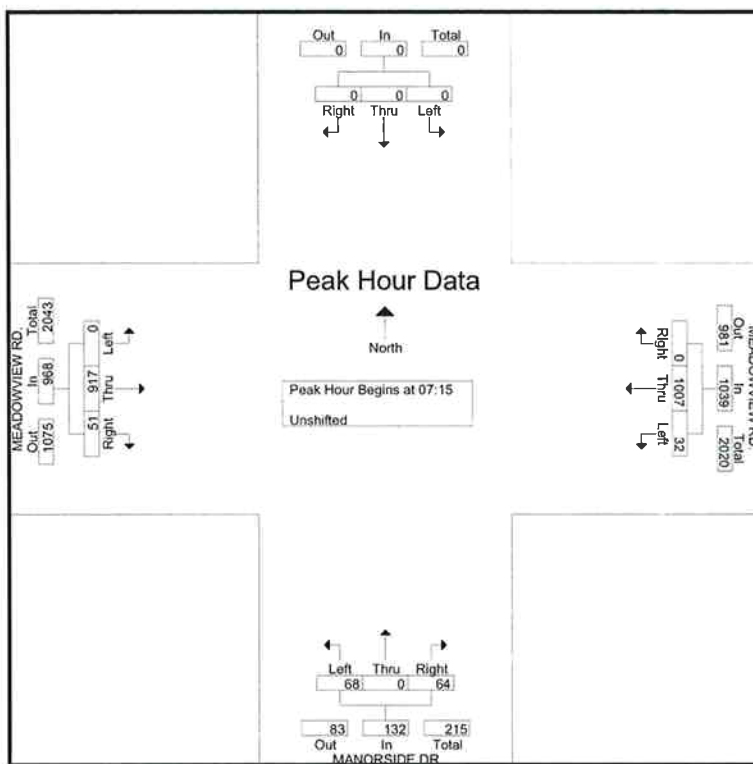
Start Time	Southbound				MEADOWVIEW RD. Westbound				MANORSIDE DR. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	0	0	0	0	0	250	5	255	20	0	15	35	6	196	0	202	492
07:30	0	0	0	0	0	232	15	247	18	0	16	34	12	231	0	243	524
07:45	0	0	0	0	0	267	6	273	13	0	21	34	14	255	0	269	576
08:00	0	0	0	0	0	258	6	264	13	0	16	29	19	235	0	254	547
Total Volume	0	0	0	0	0	1007	32	1039	64	0	68	132	51	917	0	968	2139
% App. Total	0	0	0	0	0	96.9	3.1		48.5	0	51.5		5.3	94.7	0		
PHF	.000	.000	.000	.000	.000	.943	.533	.951	.800	.000	.810	.943	.671	.899	.000	.900	.928

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 16:15

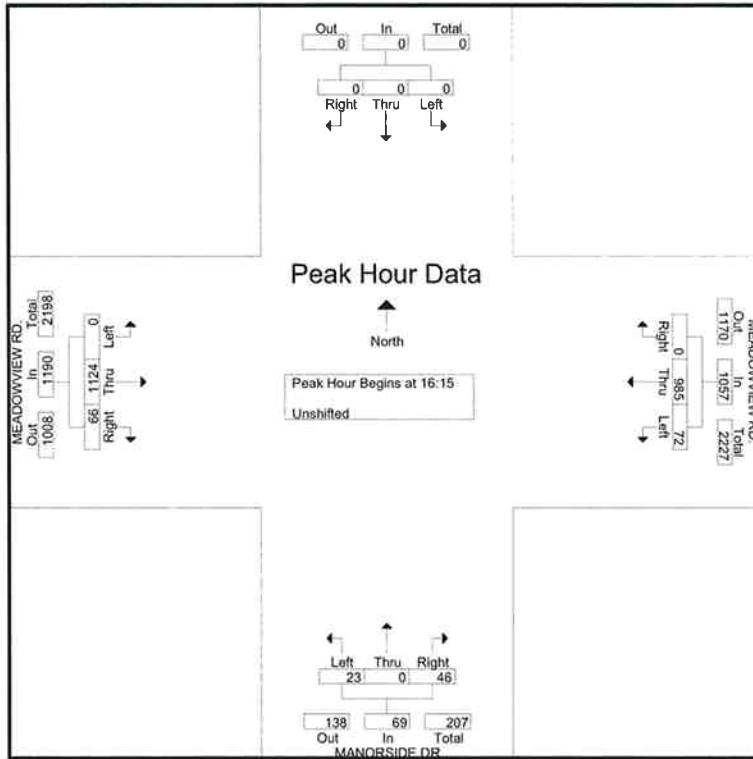
16:15	0	0	0	0	0	294	12	306	11	0	6	17	15	284	0	299	622
16:30	0	0	0	0	0	235	21	256	10	0	7	17	23	291	0	314	587
16:45	0	0	0	0	0	219	17	236	10	0	5	15	12	261	0	273	524
17:00	0	0	0	0	0	237	22	259	15	0	5	20	16	288	0	304	583
Total Volume	0	0	0	0	0	985	72	1057	46	0	23	69	66	1124	0	1190	2316
% App. Total	0	0	0	0	0	93.2	6.8		66.7	0	33.3		5.5	94.5	0		
PHF	.000	.000	.000	.000	.000	.838	.818	.864	.767	.000	.821	.862	.717	.966	.000	.947	.931

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Groups Printed- Unshifted

Start Time	I-5 NORTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 NORTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	0	0	0	182	55	0	237	63	0	25	88	121	178	0	299	624
07:15	0	0	0	0	171	58	0	229	94	0	50	144	126	213	0	339	712
07:30	0	0	0	0	152	97	0	249	79	0	39	118	90	284	0	374	741
07:45	0	0	0	0	133	104	0	237	101	0	62	163	91	263	0	354	754
Total	0	0	0	0	638	314	0	952	337	0	176	513	428	938	0	1366	2831
08:00	0	0	0	0	165	111	0	276	0	0	58	58	109	234	0	343	677
08:15	0	0	0	0	227	83	0	310	100	0	63	163	118	192	0	310	783
08:30	0	0	0	0	182	73	0	255	79	0	48	127	131	130	0	261	643
08:45	0	0	0	0	212	100	0	312	92	0	43	135	109	129	0	238	685
Total	0	0	0	0	786	367	0	1153	271	0	212	483	467	685	0	1152	2788
*** BREAK ***																	
16:00	0	0	0	0	120	150	0	270	44	0	55	99	31	236	0	267	636
16:15	0	0	0	0	120	182	0	302	53	0	44	97	41	286	0	327	726
16:30	0	0	0	0	122	174	0	296	49	0	50	99	50	263	0	313	708
16:45	0	0	0	0	112	190	0	302	67	0	50	117	30	235	0	265	684
Total	0	0	0	0	474	696	0	1170	213	0	199	412	152	1020	0	1172	2754
17:00	0	0	0	0	107	215	0	322	46	0	53	99	59	288	0	347	768
17:15	0	0	0	0	94	228	0	322	38	0	73	111	47	280	0	327	760
17:30	0	0	0	0	93	191	0	284	47	0	69	116	52	253	0	305	705
17:45	0	0	0	0	98	199	0	297	50	0	72	122	41	236	0	277	696
Total	0	0	0	0	392	833	0	1225	181	0	267	448	199	1057	0	1256	2929
Grand Total	0	0	0	0	2290	2210	0	4500	1002	0	854	1856	1246	3700	0	4946	11302
Approch %	0	0	0	0	50.9	49.1	0	39.8	5.4	0	4.6	16.4	25.2	74.8	0	43.8	
Total %	0	0	0	0	20.3	19.6	0	39.8	8.9	0	7.6	16.4	11	32.7	0	43.8	

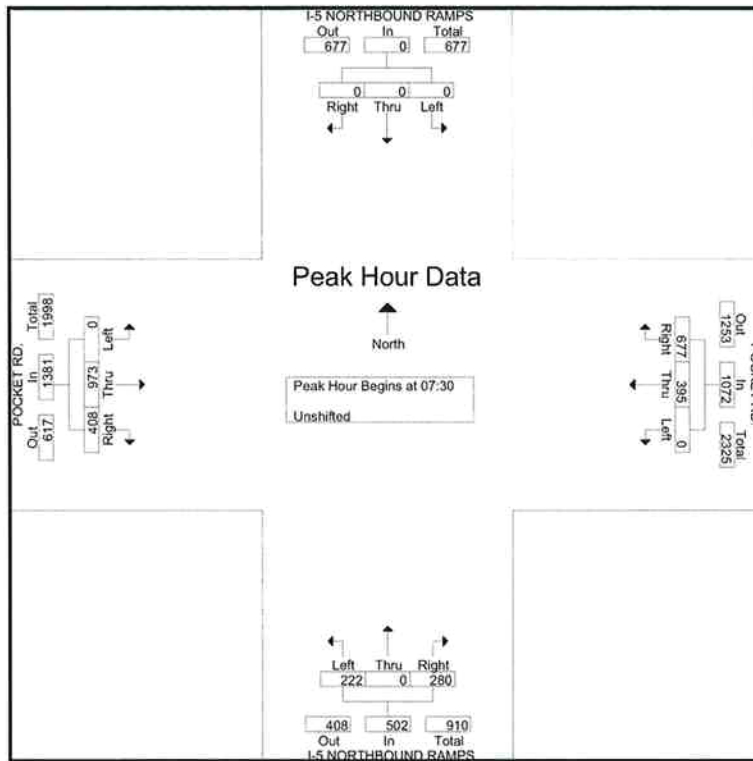
Start Time	I-5 NORTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 NORTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30																	
07:30	0	0	0	0	152	97	0	249	79	0	39	118	90	284	0	374	741
07:45	0	0	0	0	133	104	0	237	101	0	62	163	91	263	0	354	754
08:00	0	0	0	0	165	111	0	276	0	0	58	58	109	234	0	343	677
08:15	0	0	0	0	227	83	0	310	100	0	63	163	118	192	0	310	783
Total Volume	0	0	0	0	677	395	0	1072	280	0	222	502	408	973	0	1381	2955
% App. Total	0	0	0	0	63.2	36.8	0	39.8	55.8	0	44.2	16.4	29.5	70.5	0	43.8	
PHF	.000	.000	.000	.000	.746	.890	.000	.865	.693	.000	.881	.770	.864	.857	.000	.923	.943

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

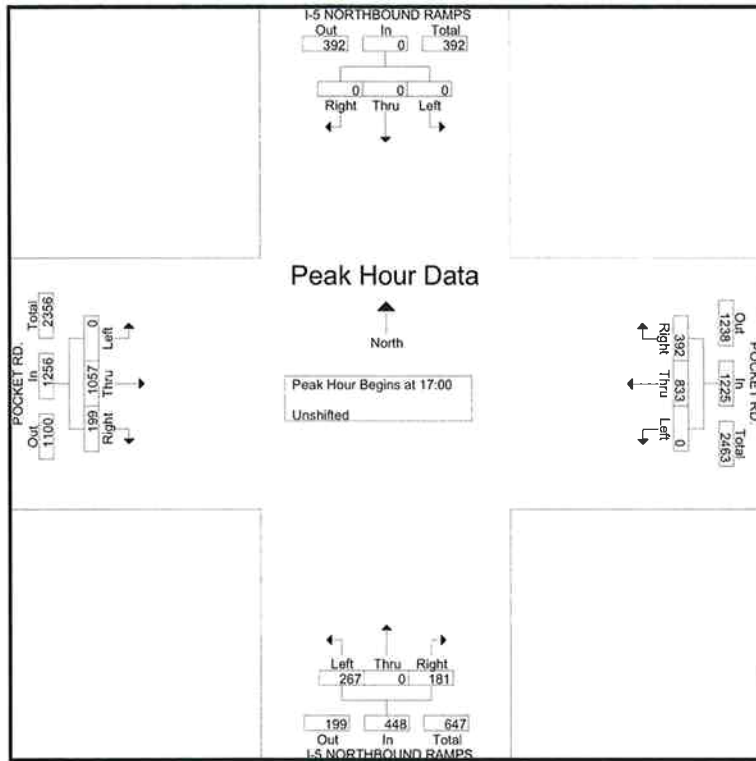
17:00	0	0	0	0	107	215	0	322	46	0	53	99	59	288	0	347	768
17:15	0	0	0	0	94	228	0	322	38	0	73	111	47	280	0	327	760
17:30	0	0	0	0	93	191	0	284	47	0	69	116	52	253	0	305	705
17:45	0	0	0	0	98	199	0	297	50	0	72	122	41	236	0	277	696
Total Volume	0	0	0	0	392	833	0	1225	181	0	267	448	199	1057	0	1256	2929
% App. Total	0	0	0	0	32	68	0	40.4	40.4	0	59.6	15.8	84.2	0			
PHF	.000	.000	.000	.000	.916	.913	.000	.951	.905	.000	.914	.918	.843	.918	.000	.905	.953

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Groups Printed- Unshifted

Start Time	I-5 SOUTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 SOUTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	18	0	79	97	29	51	0	80	0	0	0	0	53	220	0	273	450
07:15	22	0	96	118	28	80	0	108	0	0	0	0	66	243	0	309	535
07:30	35	0	112	147	30	106	0	136	0	0	0	0	49	262	0	311	594
07:45	25	0	115	140	24	142	0	166	0	0	0	0	64	239	0	303	609
Total	100	0	402	502	111	379	0	490	0	0	0	0	232	964	0	1196	2188
08:00	43	0	105	148	21	148	0	169	0	0	0	0	57	238	0	295	612
08:15	44	0	95	139	22	124	0	146	0	0	0	0	38	215	0	253	538
08:30	41	0	57	98	23	98	0	121	0	0	0	0	51	204	0	255	474
08:45	26	0	72	98	34	109	0	143	0	0	0	0	29	166	0	195	436
Total	154	0	329	483	100	479	0	579	0	0	0	0	175	823	0	998	2060

*** BREAK ***

16:00	94	0	169	263	48	157	0	205	0	0	0	0	41	98	0	139	607
16:15	107	0	210	317	51	175	0	226	0	0	0	0	45	117	0	162	705
16:30	93	0	193	286	53	171	0	224	0	0	0	0	54	120	0	174	684
16:45	116	0	152	268	69	171	0	240	0	0	0	0	51	113	0	164	672
Total	410	0	724	1134	221	674	0	895	0	0	0	0	191	448	0	639	2668
17:00	100	0	182	282	73	195	0	268	0	0	0	0	61	165	0	226	776
17:15	117	0	193	310	89	212	0	301	0	0	0	0	59	134	0	193	804
17:30	132	0	164	296	56	204	0	260	0	0	0	0	60	141	0	201	757
17:45	117	0	148	265	68	213	0	281	0	0	0	0	51	129	0	180	726
Total	466	0	687	1153	286	824	0	1110	0	0	0	0	231	569	0	800	3063
Grand Total	1130	0	2142	3272	718	2356	0	3074	0	0	0	0	829	2804	0	3633	9979
Apprch %	34.5	0	65.5		23.4	76.6	0		0	0	0	0	22.8	77.2	0		
Total %	11.3	0	21.5	32.8	7.2	23.6	0	30.8	0	0	0	0	8.3	28.1	0	36.4	

Start Time	I-5 SOUTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 SOUTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:30	35	0	112	147	30	106	0	136	0	0	0	0	49	262	0	311	594
07:45	25	0	115	140	24	142	0	166	0	0	0	0	64	239	0	303	609
08:00	43	0	105	148	21	148	0	169	0	0	0	0	57	238	0	295	612
08:15	44	0	95	139	22	124	0	146	0	0	0	0	38	215	0	253	538
Total Volume	147	0	427	574	97	520	0	617	0	0	0	0	208	954	0	1162	2353
% App. Total	25.6	0	74.4		15.7	84.3	0		0	0	0	0	17.9	82.1	0		
PHF	.835	.000	.928	.970	.808	.878	.000	.913	.000	.000	.000	.000	.812	.910	.000	.934	.961

Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1

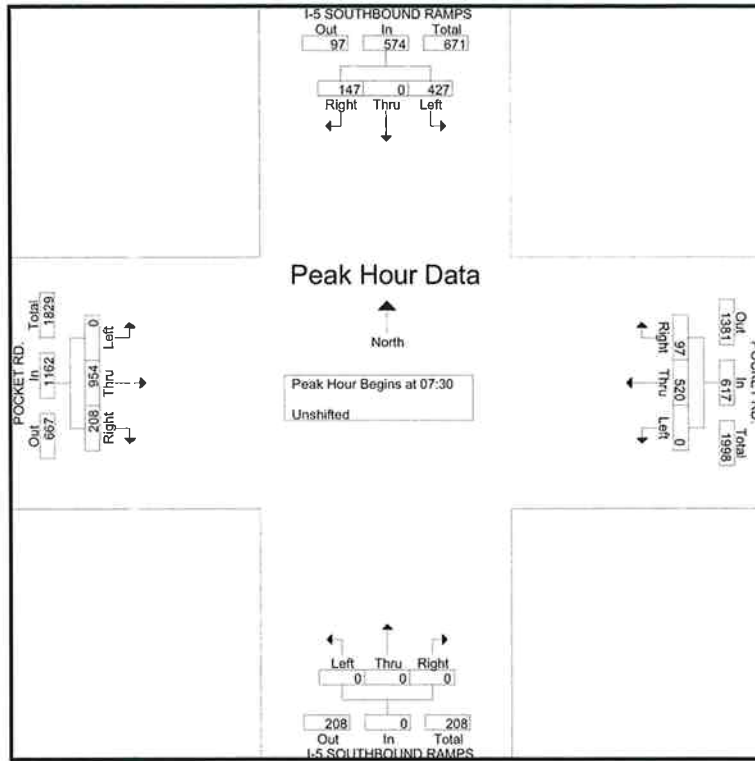
Peak Hour for Entire Intersection Begins at 07:30

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

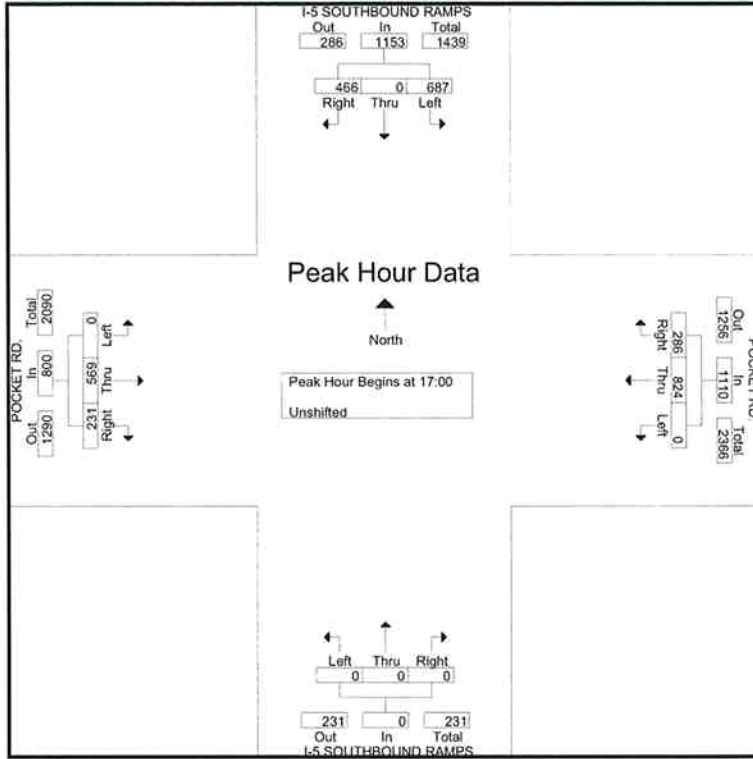
17:00	100	0	182	282	73	195	0	268	0	0	0	0	61	165	0	226	776
17:15	117	0	193	310	89	212	0	301	0	0	0	0	59	134	0	193	804
17:30	132	0	164	296	56	204	0	260	0	0	0	0	60	141	0	201	757
17:45	117	0	148	265	68	213	0	281	0	0	0	0	51	129	0	180	726
Total Volume	466	0	687	1153	286	824	0	1110	0	0	0	0	231	569	0	800	3063
% App. Total	40.4	0	59.6		25.8	74.2	0		0	0	0	0	28.9	71.1	0		
PHF	.883	.000	.890	.930	.803	.967	.000	.922	.000	.000	.000	.000	.947	.862	.000	.885	.952

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File Name : POCKET-I-5 SB RAMPS
 Site Code : 00000000
 Start Date : 2/6/2007
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File Name : STONECREST-FREEPORT-F
Site Code : 00000000
Start Date : 2/7/2007
Page No : 1

Groups Printed- Unshifted

Start Time	FREEPORT BLVD. Southbound				STONECREST AVE. Westbound				FREEPORT BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	36	0	36	1	0	0	1	0	70	0	70	0	0	0	0	107
07:15	0	41	1	42	0	0	0	0	1	85	0	86	0	0	0	0	128
07:30	0	51	1	52	0	0	0	0	0	84	0	84	0	0	0	0	136
07:45	0	40	0	40	0	0	0	0	1	73	0	74	0	0	0	0	114
Total	0	168	2	170	1	0	0	1	2	312	0	314	0	0	0	0	485
08:00	0	42	0	42	2	0	1	3	0	40	0	40	0	0	0	0	85
08:15	0	54	2	56	0	0	0	0	0	52	0	52	0	0	0	0	108
08:30	0	25	1	26	0	0	0	0	0	42	0	42	0	0	0	0	68
08:45	0	29	0	29	0	0	0	0	0	39	0	39	0	0	0	0	68
Total	0	150	3	153	2	0	1	3	0	173	0	173	0	0	0	0	329
*** BREAK ***																	
16:00	0	48	1	49	0	0	0	0	0	60	0	60	0	0	0	0	109
16:15	0	65	0	65	0	0	0	0	0	58	0	58	0	0	0	0	123
16:30	0	58	0	58	1	0	0	1	0	55	0	55	0	0	0	0	114
16:45	0	65	1	66	1	0	0	1	0	78	0	78	0	0	0	0	145
Total	0	236	2	238	2	0	0	2	0	251	0	251	0	0	0	0	491
17:00	0	61	1	62	2	0	0	2	0	59	0	59	0	0	0	0	123
17:15	0	52	0	52	1	0	0	1	1	81	0	82	0	0	0	0	135
17:30	0	45	0	45	1	0	0	1	0	86	0	86	0	0	0	0	132
17:45	0	47	1	48	2	0	0	2	0	60	0	60	0	0	0	0	110
Total	0	205	2	207	6	0	0	6	1	286	0	287	0	0	0	0	500
Grand Total	0	759	9	768	11	0	1	12	3	1022	0	1025	0	0	0	0	1805
Apprch %	0	98.8	1.2		91.7	0	8.3		0.3	99.4	0		0	0	0		
Total %	0	42	0.5	42.5	0.6	0	0.1	0.7	0.2	56.6	0	56.8	0	0	0	0	

Start Time	FREEPORT BLVD. Southbound				STONECREST AVE. Westbound				FREEPORT BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	36	0	36	1	0	0	1	0	70	0	70	0	0	0	0	107
07:15	0	41	1	42	0	0	0	0	1	85	0	86	0	0	0	0	128
07:30	0	51	1	52	0	0	0	0	0	84	0	84	0	0	0	0	136
07:45	0	40	0	40	0	0	0	0	1	73	0	74	0	0	0	0	114
Total Volume	0	168	2	170	1	0	0	1	2	312	0	314	0	0	0	0	485
% App. Total	0	98.8	1.2		100	0	0		0.6	99.4	0		0	0	0		
PHF	.000	.824	.500	.817	.250	.000	.000	.250	.500	.918	.000	.913	.000	.000	.000	.000	.892

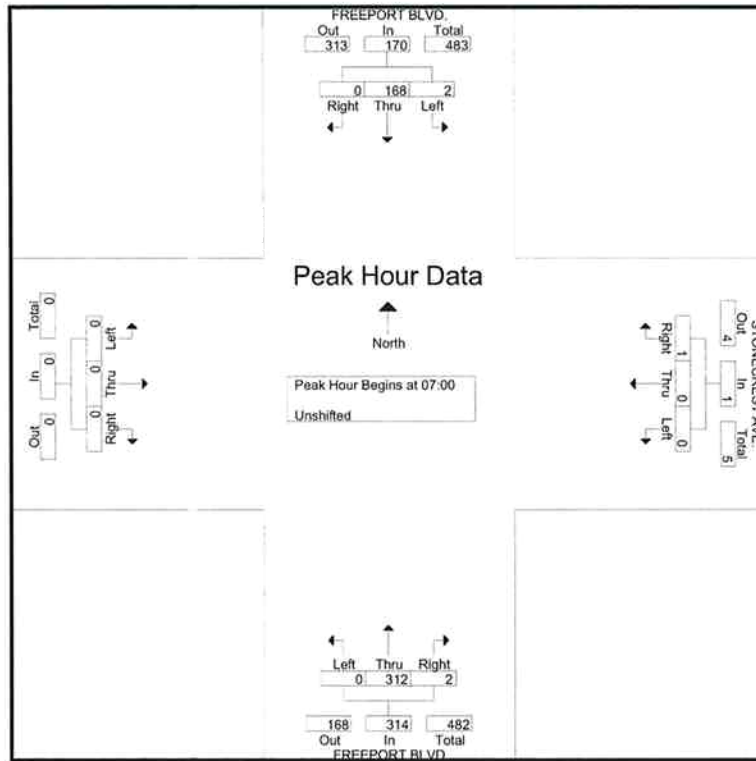
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 07:00

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File Name : STONECREST-FREEPORT-F
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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 16:45

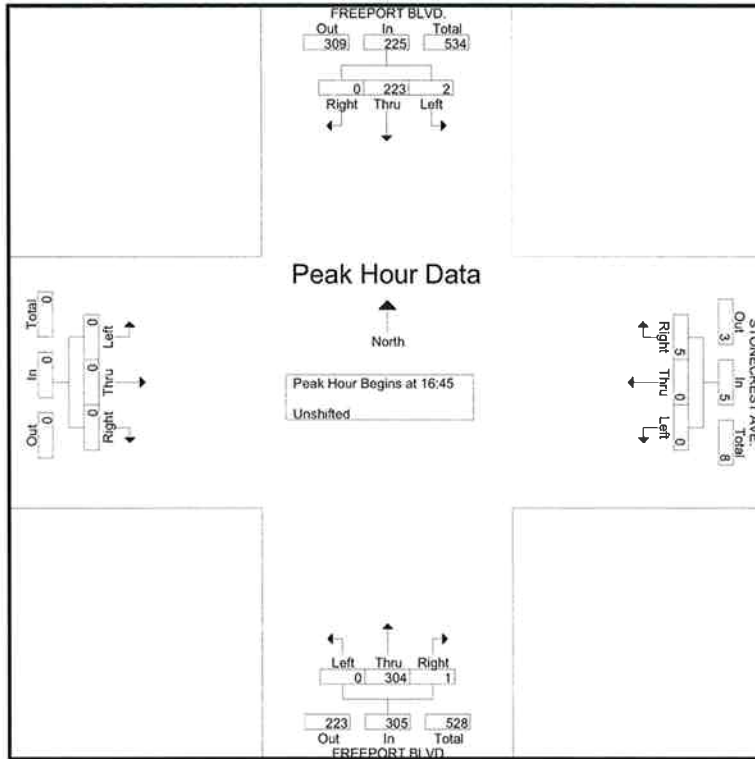
16:45	0	65	1	66	1	0	0	1	0	78	0	78	0	0	0	0	145
17:00	0	61	1	62	2	0	0	2	0	59	0	59	0	0	0	0	123
17:15	0	52	0	52	1	0	0	1	1	81	0	82	0	0	0	0	135
17:30	0	45	0	45	1	0	0	1	0	86	0	86	0	0	0	0	132
Total Volume	0	223	2	225	5	0	0	5	1	304	0	305	0	0	0	0	535
% App. Total	0	99.1	0.9	100	0	0	0	0.3	99.7	0	0	0	0	0	0	0	
PHF	.000	.858	.500	.852	.625	.000	.000	.625	.250	.884	.000	.887	.000	.000	.000	.000	.922

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File Name : STONECREST-FREEPORT-F
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File Name : MEADOWVIEW-FREEPORT-F

Site Code : 00000000

Start Date : 2/6/2007

Page No : 1

Groups Printed- Unshifted

Start Time	FREEPORT BLVD. Southbound				MEADOWVIEW RD. Westbound				FREEPORT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	7	22	14	43	50	215	8	273	9	15	18	42	20	150	83	253	611
07:15	5	29	19	53	78	184	10	272	15	41	24	80	29	174	100	303	708
07:30	12	44	16	72	81	187	14	282	12	37	14	63	34	198	119	351	768
07:45	14	26	18	58	59	204	7	270	16	26	20	62	33	208	114	355	745
Total	38	121	67	226	268	790	39	1097	52	119	76	247	116	730	416	1262	2832
08:00	16	18	25	59	61	197	7	265	14	22	30	66	29	211	82	322	712
08:15	8	28	24	60	58	150	10	218	6	23	23	52	21	175	72	268	598
08:30	4	18	19	41	66	137	10	213	9	26	22	57	21	118	67	206	517
08:45	5	18	14	37	53	139	10	202	8	19	20	47	23	101	53	177	463
Total	33	82	82	197	238	623	37	898	37	90	95	222	94	605	274	973	2290

*** BREAK ***

16:00	45	21	101	167	54	193	9	256	14	15	34	63	21	205	32	258	744
16:15	55	24	100	179	47	229	21	297	18	13	29	60	33	247	32	312	848
16:30	45	19	89	153	38	228	11	277	15	15	34	64	25	262	40	327	821
16:45	59	31	93	183	53	181	14	248	21	21	38	80	33	228	42	303	814
Total	204	95	383	682	192	831	55	1078	68	64	135	267	112	942	146	1200	3227
17:00	85	21	82	188	54	193	8	255	13	17	31	61	32	232	45	309	813
17:15	111	15	112	238	51	181	9	241	22	21	44	87	43	250	31	324	890
17:30	66	14	89	169	39	168	9	216	16	15	52	83	20	241	31	292	760
17:45	80	17	86	183	55	200	13	268	16	8	46	70	27	252	28	307	828
Total	342	67	369	778	199	742	39	980	67	61	173	301	122	975	135	1232	3291
Grand Total	617	365	901	1883	897	2986	170	4053	224	334	479	1037	444	3252	971	4667	11640
Apprch %	32.8	19.4	47.8		22.1	73.7	4.2		21.6	32.2	46.2		9.5	69.7	20.8		
Total %	5.3	3.1	7.7	16.2	7.7	25.7	1.5	34.8	1.9	2.9	4.1	8.9	3.8	27.9	8.3	40.1	

Start Time	FREEPORT BLVD. Southbound				MEADOWVIEW RD. Westbound				FREEPORT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:15	5	29	19	53	78	184	10	272	15	41	24	80	29	174	100	303	708
07:30	12	44	16	72	81	187	14	282	12	37	14	63	34	198	119	351	768
07:45	14	26	18	58	59	204	7	270	16	26	20	62	33	208	114	355	745
08:00	16	18	25	59	61	197	7	265	14	22	30	66	29	211	82	322	712
Total Volume	47	117	78	242	279	772	38	1089	57	126	88	271	125	791	415	1331	2933
% App. Total	19.4	48.3	32.2		25.6	70.9	3.5		21	46.5	32.5		9.4	59.4	31.2		
PHF	.734	.665	.780	.840	.861	.946	.679	.965	.891	.768	.733	.847	.919	.937	.872	.937	.955

Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1

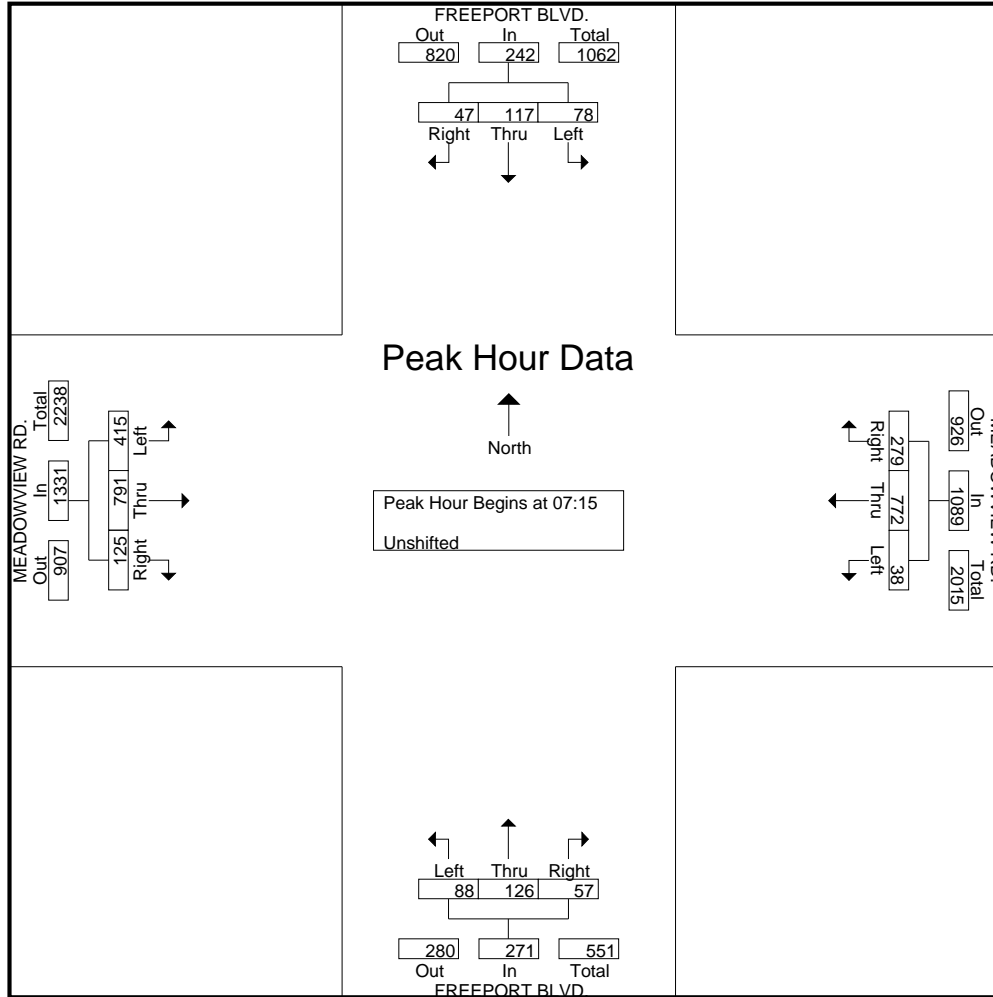
Peak Hour for Entire Intersection Begins at 07:15

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 16:30

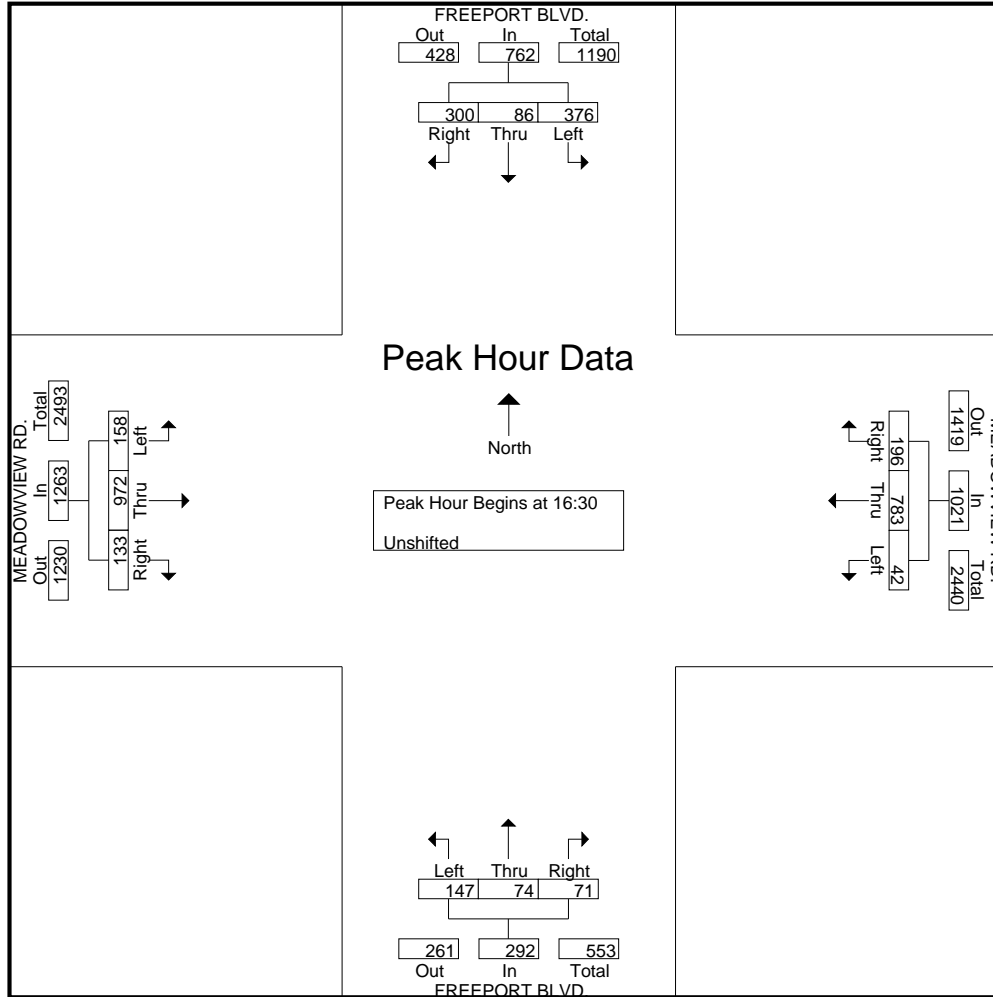
16:30	45	19	89	153	38	228	11	277	15	15	34	64	25	262	40	327	821
16:45	59	31	93	183	53	181	14	248	21	21	38	80	33	228	42	303	814
17:00	85	21	82	188	54	193	8	255	13	17	31	61	32	232	45	309	813
17:15	111	15	112	238	51	181	9	241	22	21	44	87	43	250	31	324	890
Total Volume	300	86	376	762	196	783	42	1021	71	74	147	292	133	972	158	1263	3338
% App. Total	39.4	11.3	49.3		19.2	76.7	4.1		24.3	25.3	50.3		10.5	77	12.5		
PHF	.676	.694	.839	.800	.907	.859	.750	.921	.807	.881	.835	.839	.773	.927	.878	.966	.938

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File Name : MEADOWVIEW-FREEPORT-F
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File Name : MEADOWVIEW-MANORSIDE-F

Site Code : 00000000

Start Date : 2/6/2007

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Groups Printed- Unshifted

Start Time	Southbound				MEADOWVIEW RD. Westbound				MANORSIDE DR. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	0	0	0	0	237	5	242	12	0	11	23	3	152	0	155	420
07:15	0	0	0	0	0	250	5	255	20	0	15	35	6	196	0	202	492
07:30	0	0	0	0	0	232	15	247	18	0	16	34	12	231	0	243	524
07:45	0	0	0	0	0	267	6	273	13	0	21	34	14	255	0	269	576
Total	0	0	0	0	0	986	31	1017	63	0	63	126	35	834	0	869	2012
08:00	0	0	0	0	0	258	6	264	13	0	16	29	19	235	0	254	547
08:15	0	0	0	0	0	224	10	234	5	0	6	11	12	187	0	199	444
08:30	0	0	0	0	0	174	9	183	7	0	5	12	10	139	0	149	344
08:45	0	0	0	0	0	195	3	198	10	0	10	20	4	127	0	131	349
Total	0	0	0	0	0	851	28	879	35	0	37	72	45	688	0	733	1684

*** BREAK ***

16:00	0	0	0	0	0	238	14	252	9	0	11	20	13	295	0	308	580
16:15	0	0	0	0	0	294	12	306	11	0	6	17	15	284	0	299	622
16:30	0	0	0	0	0	235	21	256	10	0	7	17	23	291	0	314	587
16:45	0	0	0	0	0	219	17	236	10	0	5	15	12	261	0	273	524
Total	0	0	0	0	0	986	64	1050	40	0	29	69	63	1131	0	1194	2313
17:00	0	0	0	0	0	237	22	259	15	0	5	20	16	288	0	304	583
17:15	0	0	0	0	0	230	14	244	7	0	6	13	13	297	0	310	567
17:30	0	0	0	0	0	212	17	229	10	0	11	21	11	299	0	310	560
17:45	0	0	0	0	0	260	14	274	8	0	3	11	14	304	0	318	603
Total	0	0	0	0	0	939	67	1006	40	0	25	65	54	1188	0	1242	2313
Grand Total	0	0	0	0	0	3762	190	3952	178	0	154	332	197	3841	0	4038	8322
Apprch %	0	0	0	0	0	95.2	4.8		53.6	0	46.4		4.9	95.1	0		
Total %	0	0	0	0	0	45.2	2.3	47.5	2.1	0	1.9	4	2.4	46.2	0	48.5	

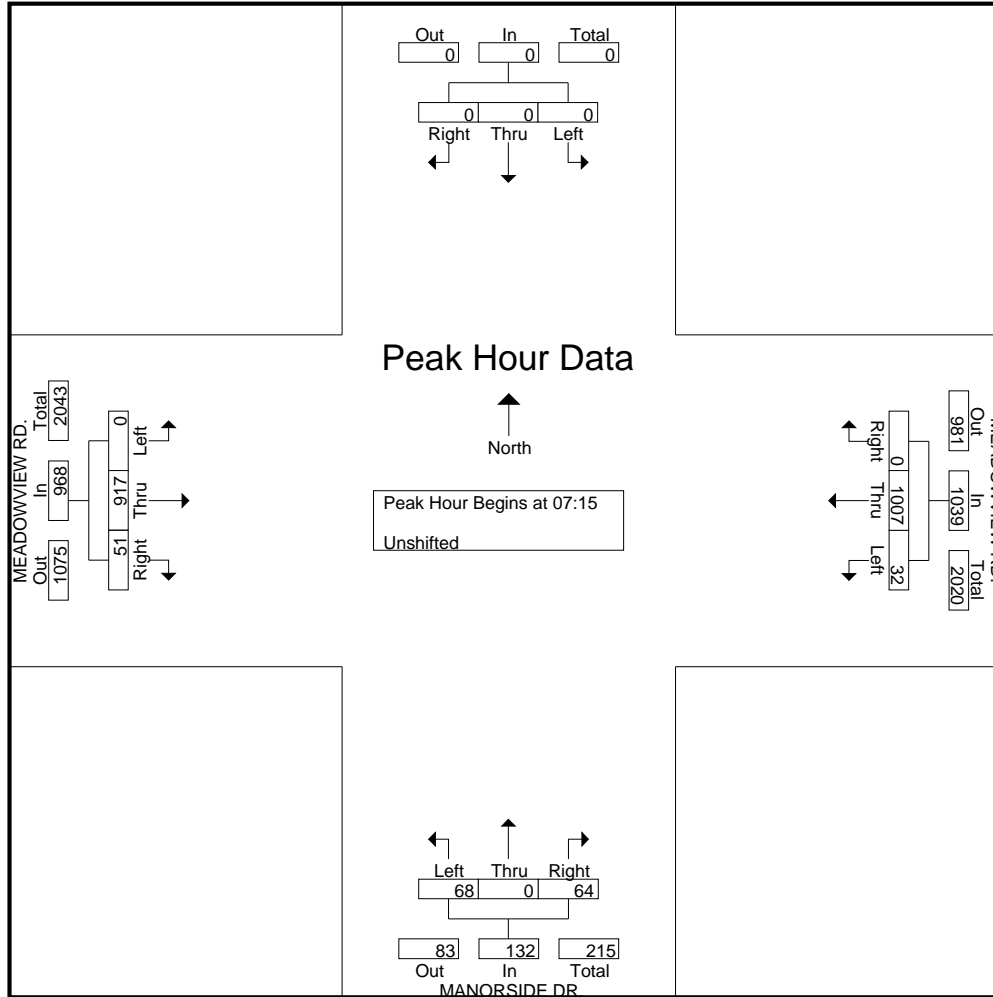
Start Time	Southbound				MEADOWVIEW RD. Westbound				MANORSIDE DR. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	0	0	0	0	0	250	5	255	20	0	15	35	6	196	0	202	492
07:30	0	0	0	0	0	232	15	247	18	0	16	34	12	231	0	243	524
07:45	0	0	0	0	0	267	6	273	13	0	21	34	14	255	0	269	576
08:00	0	0	0	0	0	258	6	264	13	0	16	29	19	235	0	254	547
Total Volume	0	0	0	0	0	1007	32	1039	64	0	68	132	51	917	0	968	2139
% App. Total	0	0	0	0	0	96.9	3.1		48.5	0	51.5		5.3	94.7	0		
PHF	.000	.000	.000	.000	.000	.943	.533	.951	.800	.000	.810	.943	.671	.899	.000	.900	.928

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File Name : MEADOWVIEW-MANORSIDE-F
Site Code : 00000000
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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 16:15

16:15	0	0	0	0	0	294	12	306	11	0	6	17	15	284	0	299	622
16:30	0	0	0	0	0	235	21	256	10	0	7	17	23	291	0	314	587
16:45	0	0	0	0	0	219	17	236	10	0	5	15	12	261	0	273	524
17:00	0	0	0	0	0	237	22	259	15	0	5	20	16	288	0	304	583
Total Volume	0	0	0	0	0	985	72	1057	46	0	23	69	66	1124	0	1190	2316
% App. Total	0	0	0	0	0	93.2	6.8		66.7	0	33.3		5.5	94.5	0		
PHF	.000	.000	.000	.000	.000	.838	.818	.864	.767	.000	.821	.862	.717	.966	.000	.947	.931

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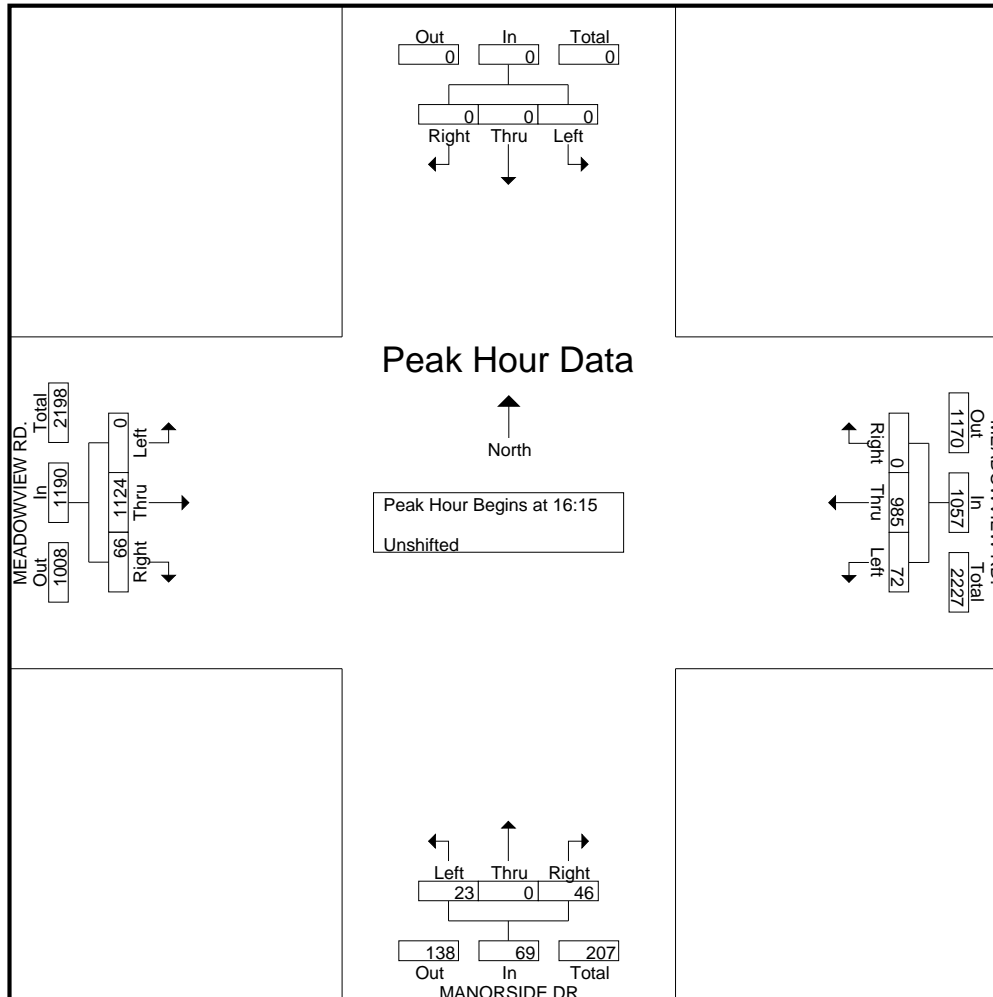
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File Name : MEADOWVIEW-MANORSIDE-F

Site Code : 00000000

Start Date : 2/6/2007

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File Name : POCKET-I-5 NB RAMPS

Site Code : 00000000

Start Date : 2/6/2007

Page No : 1

Groups Printed- Unshifted

Start Time	I-5 NORTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 NORTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	0	0	0	182	55	0	237	63	0	25	88	121	178	0	299	624
07:15	0	0	0	0	171	58	0	229	94	0	50	144	126	213	0	339	712
07:30	0	0	0	0	152	97	0	249	79	0	39	118	90	284	0	374	741
07:45	0	0	0	0	133	104	0	237	101	0	62	163	91	263	0	354	754
Total	0	0	0	0	638	314	0	952	337	0	176	513	428	938	0	1366	2831
08:00	0	0	0	0	165	111	0	276	0	0	58	58	109	234	0	343	677
08:15	0	0	0	0	227	83	0	310	100	0	63	163	118	192	0	310	783
08:30	0	0	0	0	182	73	0	255	79	0	48	127	131	130	0	261	643
08:45	0	0	0	0	212	100	0	312	92	0	43	135	109	129	0	238	685
Total	0	0	0	0	786	367	0	1153	271	0	212	483	467	685	0	1152	2788

*** BREAK ***

16:00	0	0	0	0	120	150	0	270	44	0	55	99	31	236	0	267	636
16:15	0	0	0	0	120	182	0	302	53	0	44	97	41	286	0	327	726
16:30	0	0	0	0	122	174	0	296	49	0	50	99	50	263	0	313	708
16:45	0	0	0	0	112	190	0	302	67	0	50	117	30	235	0	265	684
Total	0	0	0	0	474	696	0	1170	213	0	199	412	152	1020	0	1172	2754
17:00	0	0	0	0	107	215	0	322	46	0	53	99	59	288	0	347	768
17:15	0	0	0	0	94	228	0	322	38	0	73	111	47	280	0	327	760
17:30	0	0	0	0	93	191	0	284	47	0	69	116	52	253	0	305	705
17:45	0	0	0	0	98	199	0	297	50	0	72	122	41	236	0	277	696
Total	0	0	0	0	392	833	0	1225	181	0	267	448	199	1057	0	1256	2929
Grand Total	0	0	0	0	2290	2210	0	4500	1002	0	854	1856	1246	3700	0	4946	11302
Apprch %	0	0	0	0	50.9	49.1	0		54	0	46		25.2	74.8	0		
Total %	0	0	0	0	20.3	19.6	0	39.8	8.9	0	7.6	16.4	11	32.7	0	43.8	

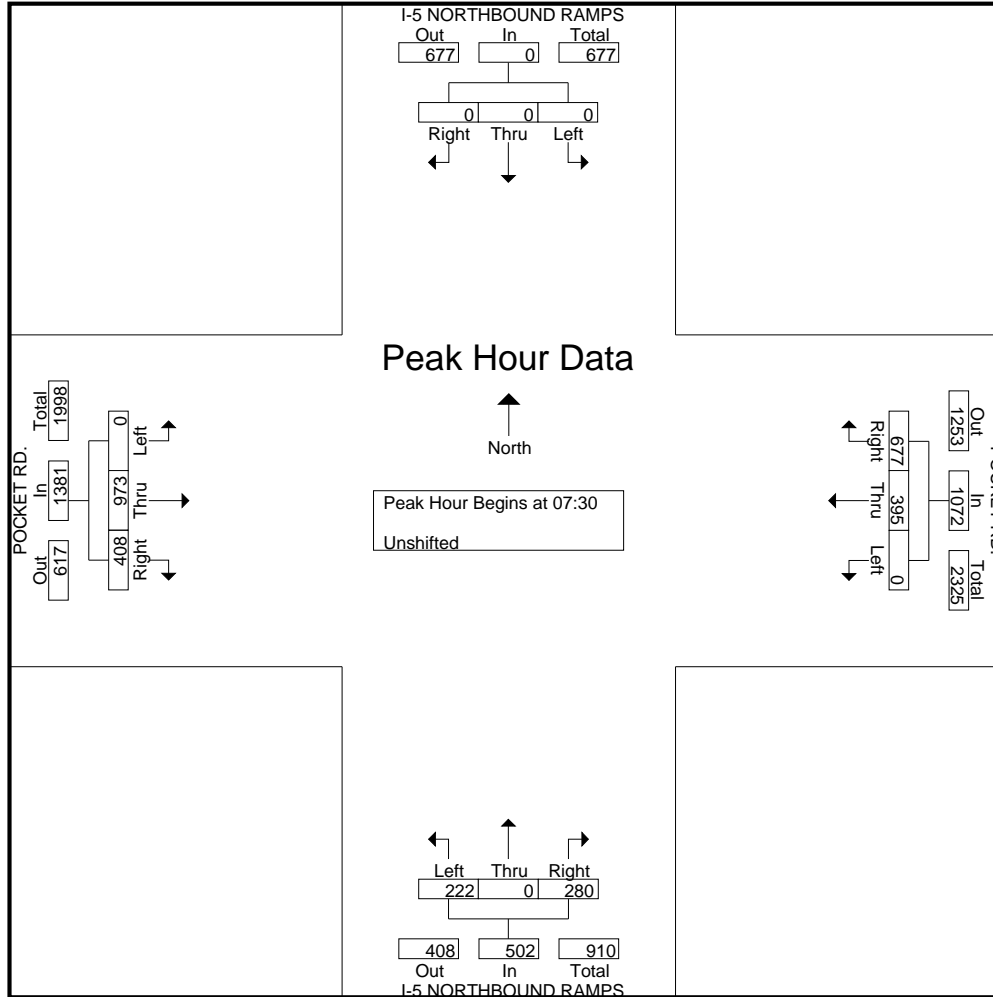
Start Time	I-5 NORTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 NORTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30																	
07:30	0	0	0	0	152	97	0	249	79	0	39	118	90	284	0	374	741
07:45	0	0	0	0	133	104	0	237	101	0	62	163	91	263	0	354	754
08:00	0	0	0	0	165	111	0	276	0	0	58	58	109	234	0	343	677
08:15	0	0	0	0	227	83	0	310	100	0	63	163	118	192	0	310	783
Total Volume	0	0	0	0	677	395	0	1072	280	0	222	502	408	973	0	1381	2955
% App. Total	0	0	0	0	63.2	36.8	0		55.8	0	44.2		29.5	70.5	0		
PHF	.000	.000	.000	.000	.746	.890	.000	.865	.693	.000	.881	.770	.864	.857	.000	.923	.943

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File Name : POCKET-I-5 NB RAMPS
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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

17:00	0	0	0	0	107	215	0	322	46	0	53	99	59	288	0	347	768
17:15	0	0	0	0	94	228	0	322	38	0	73	111	47	280	0	327	760
17:30	0	0	0	0	93	191	0	284	47	0	69	116	52	253	0	305	705
17:45	0	0	0	0	98	199	0	297	50	0	72	122	41	236	0	277	696
Total Volume	0	0	0	0	392	833	0	1225	181	0	267	448	199	1057	0	1256	2929
% App. Total	0	0	0	0	32	68	0		40.4	0	59.6		15.8	84.2	0		
PHF	.000	.000	.000	.000	.916	.913	.000	.951	.905	.000	.914	.918	.843	.918	.000	.905	.953

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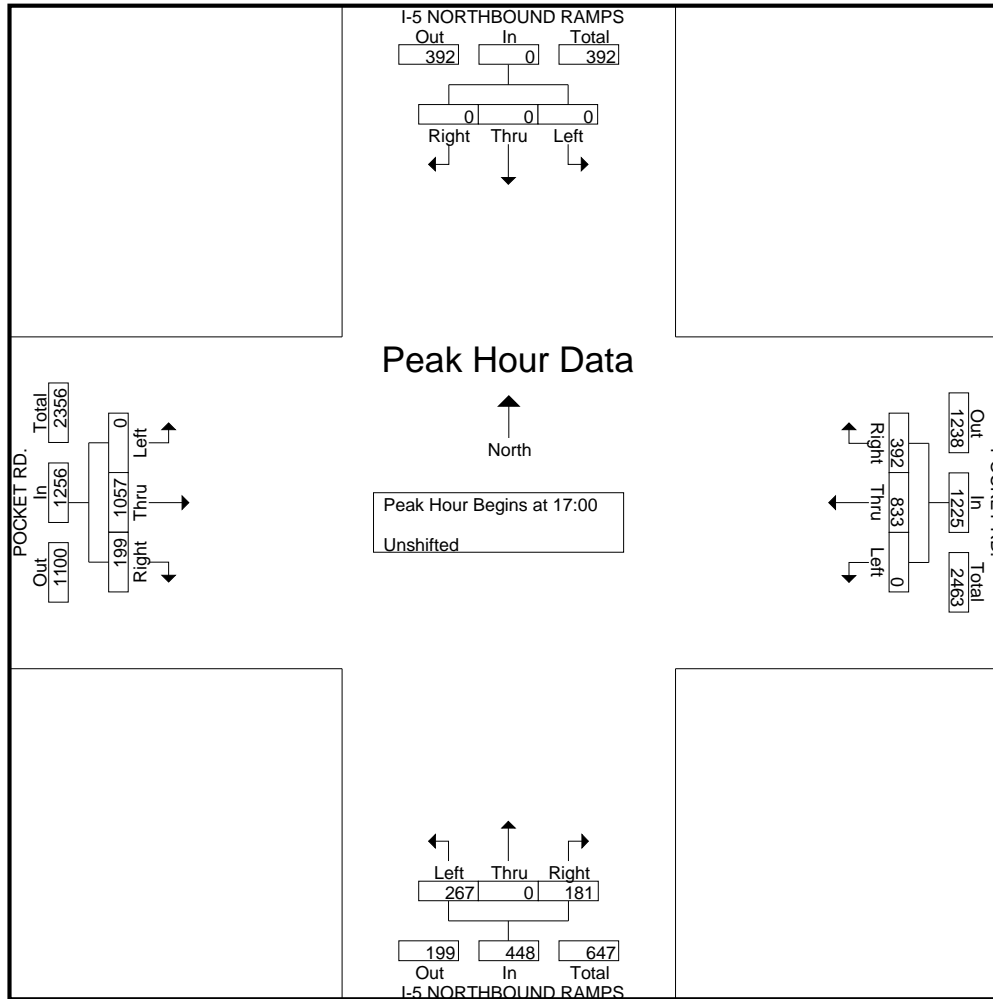
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File Name : POCKET-I-5 NB RAMPS

Site Code : 00000000

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File Name : POCKET-I-5 SB RAMPS

Site Code : 00000000

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Groups Printed- Unshifted

Start Time	I-5 SOUTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 SOUTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	18	0	79	97	29	51	0	80	0	0	0	0	53	220	0	273	450
07:15	22	0	96	118	28	80	0	108	0	0	0	0	66	243	0	309	535
07:30	35	0	112	147	30	106	0	136	0	0	0	0	49	262	0	311	594
07:45	25	0	115	140	24	142	0	166	0	0	0	0	64	239	0	303	609
Total	100	0	402	502	111	379	0	490	0	0	0	0	232	964	0	1196	2188
08:00	43	0	105	148	21	148	0	169	0	0	0	0	57	238	0	295	612
08:15	44	0	95	139	22	124	0	146	0	0	0	0	38	215	0	253	538
08:30	41	0	57	98	23	98	0	121	0	0	0	0	51	204	0	255	474
08:45	26	0	72	98	34	109	0	143	0	0	0	0	29	166	0	195	436
Total	154	0	329	483	100	479	0	579	0	0	0	0	175	823	0	998	2060

*** BREAK ***

16:00	94	0	169	263	48	157	0	205	0	0	0	0	41	98	0	139	607
16:15	107	0	210	317	51	175	0	226	0	0	0	0	45	117	0	162	705
16:30	93	0	193	286	53	171	0	224	0	0	0	0	54	120	0	174	684
16:45	116	0	152	268	69	171	0	240	0	0	0	0	51	113	0	164	672
Total	410	0	724	1134	221	674	0	895	0	0	0	0	191	448	0	639	2668
17:00	100	0	182	282	73	195	0	268	0	0	0	0	61	165	0	226	776
17:15	117	0	193	310	89	212	0	301	0	0	0	0	59	134	0	193	804
17:30	132	0	164	296	56	204	0	260	0	0	0	0	60	141	0	201	757
17:45	117	0	148	265	68	213	0	281	0	0	0	0	51	129	0	180	726
Total	466	0	687	1153	286	824	0	1110	0	0	0	0	231	569	0	800	3063
Grand Total	1130	0	2142	3272	718	2356	0	3074	0	0	0	0	829	2804	0	3633	9979
Apprch %	34.5	0	65.5		23.4	76.6	0		0	0	0	0	22.8	77.2	0		
Total %	11.3	0	21.5	32.8	7.2	23.6	0	30.8	0	0	0	0	8.3	28.1	0	36.4	

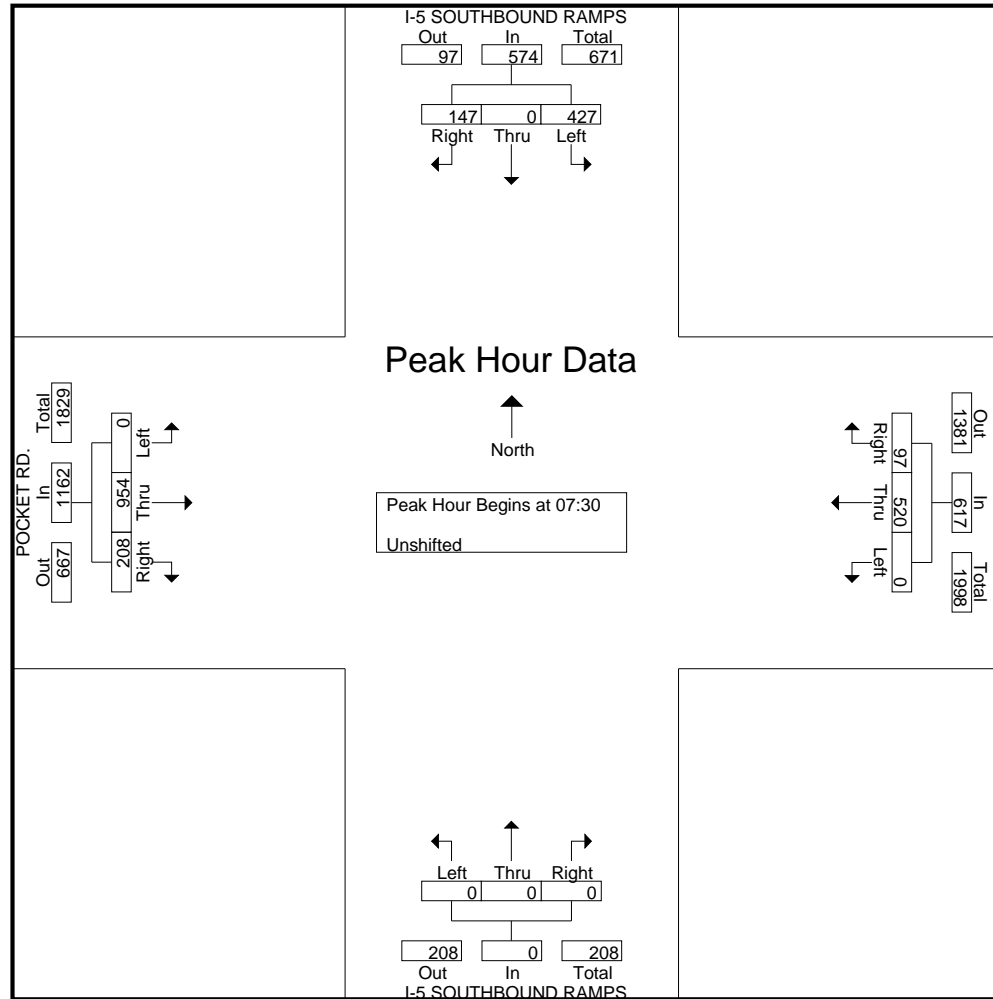
Start Time	I-5 SOUTHBOUND RAMPS Southbound				POCKET RD. Westbound				I-5 SOUTHBOUND RAMPS Northbound				POCKET RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30																	
07:30	35	0	112	147	30	106	0	136	0	0	0	0	49	262	0	311	594
07:45	25	0	115	140	24	142	0	166	0	0	0	0	64	239	0	303	609
08:00	43	0	105	148	21	148	0	169	0	0	0	0	57	238	0	295	612
08:15	44	0	95	139	22	124	0	146	0	0	0	0	38	215	0	253	538
Total Volume	147	0	427	574	97	520	0	617	0	0	0	0	208	954	0	1162	2353
% App. Total	25.6	0	74.4		15.7	84.3	0		0	0	0	0	17.9	82.1	0		
PHF	.835	.000	.928	.970	.808	.878	.000	.913	.000	.000	.000	.000	.812	.910	.000	.934	.961

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

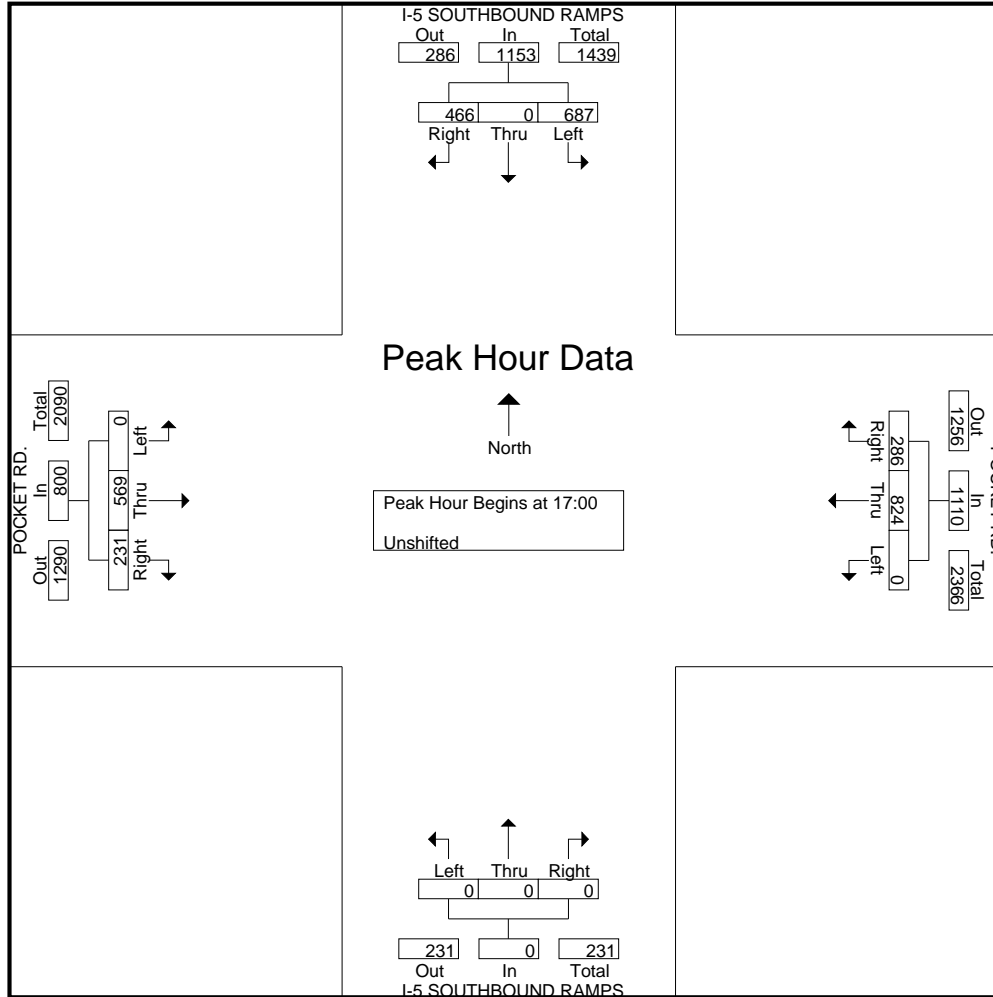
17:00	100	0	182	282	73	195	0	268	0	0	0	0	61	165	0	226	776
17:15	117	0	193	310	89	212	0	301	0	0	0	0	59	134	0	193	804
17:30	132	0	164	296	56	204	0	260	0	0	0	0	60	141	0	201	757
17:45	117	0	148	265	68	213	0	281	0	0	0	0	51	129	0	180	726
Total Volume	466	0	687	1153	286	824	0	1110	0	0	0	0	231	569	0	800	3063
% App. Total	40.4	0	59.6		25.8	74.2	0		0	0	0	0	28.9	71.1	0		
PHF	.883	.000	.890	.930	.803	.967	.000	.922	.000	.000	.000	.000	.947	.862	.000	.885	.952

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File Name : STONECREST-FREEPORT-F

Site Code : 00000000

Start Date : 2/7/2007

Page No : 1

Groups Printed- Unshifted

Start Time	FREEPORT BLVD. Southbound				STONECREST AVE. Westbound				FREEPORT BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	36	0	36	1	0	0	1	0	70	0	70	0	0	0	0	107
07:15	0	41	1	42	0	0	0	0	1	85	0	86	0	0	0	0	128
07:30	0	51	1	52	0	0	0	0	0	84	0	84	0	0	0	0	136
07:45	0	40	0	40	0	0	0	0	1	73	0	74	0	0	0	0	114
Total	0	168	2	170	1	0	0	1	2	312	0	314	0	0	0	0	485
08:00	0	42	0	42	2	0	1	3	0	40	0	40	0	0	0	0	85
08:15	0	54	2	56	0	0	0	0	0	52	0	52	0	0	0	0	108
08:30	0	25	1	26	0	0	0	0	0	42	0	42	0	0	0	0	68
08:45	0	29	0	29	0	0	0	0	0	39	0	39	0	0	0	0	68
Total	0	150	3	153	2	0	1	3	0	173	0	173	0	0	0	0	329

*** BREAK ***

16:00	0	48	1	49	0	0	0	0	0	60	0	60	0	0	0	0	109
16:15	0	65	0	65	0	0	0	0	0	58	0	58	0	0	0	0	123
16:30	0	58	0	58	1	0	0	1	0	55	0	55	0	0	0	0	114
16:45	0	65	1	66	1	0	0	1	0	78	0	78	0	0	0	0	145
Total	0	236	2	238	2	0	0	2	0	251	0	251	0	0	0	0	491
17:00	0	61	1	62	2	0	0	2	0	59	0	59	0	0	0	0	123
17:15	0	52	0	52	1	0	0	1	1	81	0	82	0	0	0	0	135
17:30	0	45	0	45	1	0	0	1	0	86	0	86	0	0	0	0	132
17:45	0	47	1	48	2	0	0	2	0	60	0	60	0	0	0	0	110
Total	0	205	2	207	6	0	0	6	1	286	0	287	0	0	0	0	500
Grand Total	0	759	9	768	11	0	1	12	3	1022	0	1025	0	0	0	0	1805
Aprch %	0	98.8	1.2		91.7	0	8.3		0.3	99.7	0		0	0	0		
Total %	0	42	0.5	42.5	0.6	0	0.1	0.7	0.2	56.6	0	56.8	0	0	0	0	

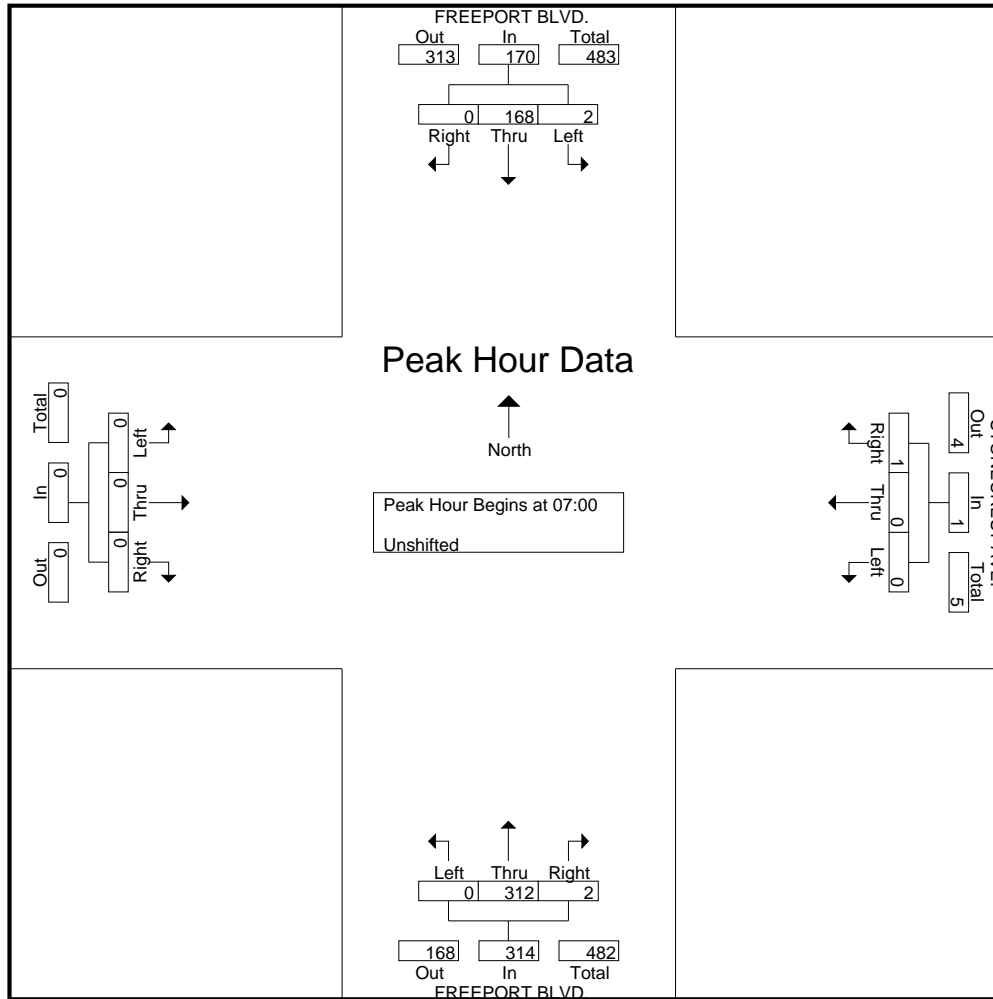
Start Time	FREEPORT BLVD. Southbound				STONECREST AVE. Westbound				FREEPORT BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00																	
07:00	0	36	0	36	1	0	0	1	0	70	0	70	0	0	0	0	107
07:15	0	41	1	42	0	0	0	0	1	85	0	86	0	0	0	0	128
07:30	0	51	1	52	0	0	0	0	0	84	0	84	0	0	0	0	136
07:45	0	40	0	40	0	0	0	0	1	73	0	74	0	0	0	0	114
Total Volume	0	168	2	170	1	0	0	1	2	312	0	314	0	0	0	0	485
% App. Total	0	98.8	1.2		100	0	0		0.6	99.4	0		0	0	0		
PHF	.000	.824	.500	.817	.250	.000	.000	.250	.500	.918	.000	.913	.000	.000	.000	.000	.892

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 16:45

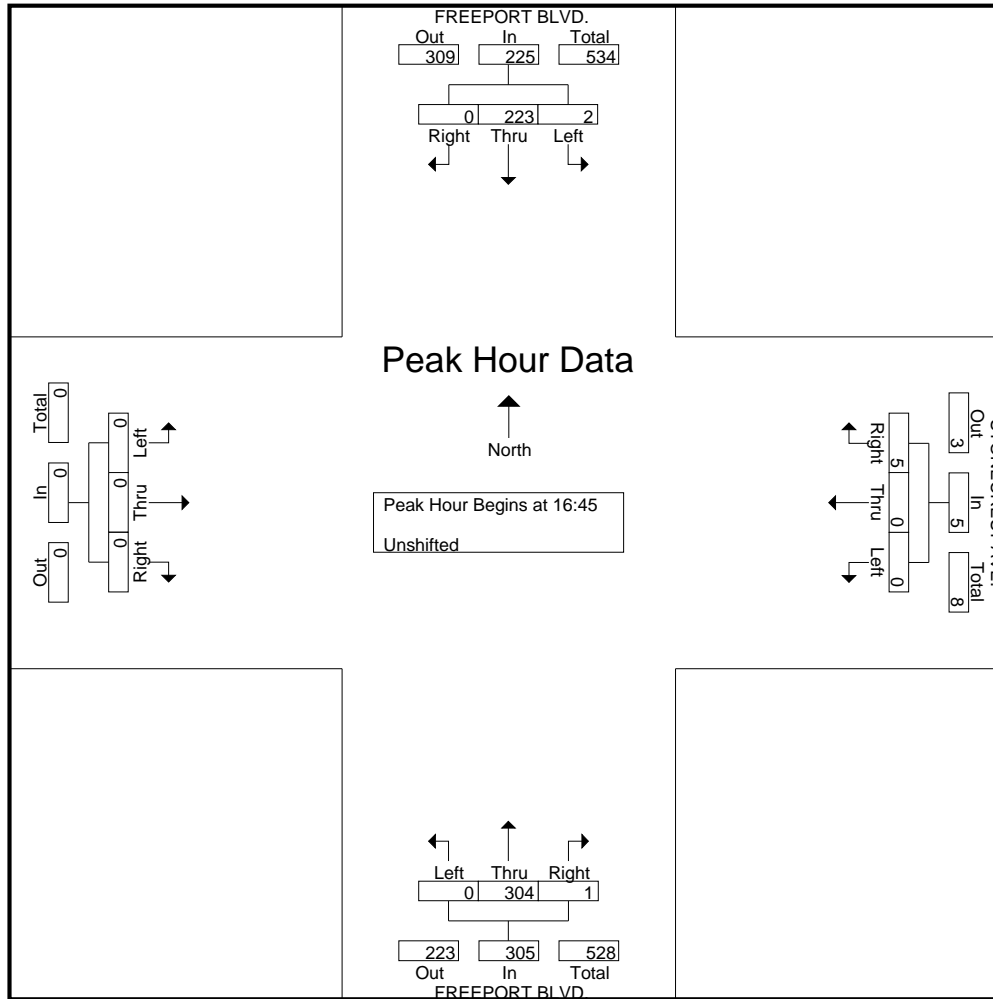
16:45	0	65	1	66	1	0	0	1	0	78	0	78	0	0	0	0	145
17:00	0	61	1	62	2	0	0	2	0	59	0	59	0	0	0	0	123
17:15	0	52	0	52	1	0	0	1	1	81	0	82	0	0	0	0	135
17:30	0	45	0	45	1	0	0	1	0	86	0	86	0	0	0	0	132
Total Volume	0	223	2	225	5	0	0	5	1	304	0	305	0	0	0	0	535
% App. Total	0	99.1	0.9	100	100	0	0	0	0.3	99.7	0	100	0	0	0	0	100
PHF	.000	.858	.500	.852	.625	.000	.000	.625	.250	.884	.000	.887	.000	.000	.000	.000	.922

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File Name : COSUMNES-FRANKLIN-F

Site Code : 00000000

Start Date : 2/7/2007

Page No : 1

Groups Printed- Unshifted

Start Time	FRANKLIN BLVD. Southbound				COSUMNES RIVER BLVD. Westbound				FRANKLIN BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	0	70	34	104	80	0	41	121	84	296	0	380	0	0	0	0	605
07:15	0	140	59	199	105	0	42	147	59	353	0	412	0	0	0	0	758
07:30	0	224	103	327	104	0	60	164	72	353	0	425	0	0	0	0	916
07:45	0	224	117	341	89	0	57	146	70	406	0	476	0	0	0	0	963
Total	0	658	313	971	378	0	200	578	285	1408	0	1693	0	0	0	0	3242
08:00	0	103	64	167	94	0	55	149	51	313	0	364	0	0	0	0	680
08:15	0	106	41	147	96	0	58	154	121	383	0	504	0	0	0	0	805
08:30	0	79	48	127	67	0	49	116	86	223	0	309	0	0	0	0	552
08:45	0	79	50	129	71	0	49	120	74	183	0	257	0	0	0	0	506
Total	0	367	203	570	328	0	211	539	332	1102	0	1434	0	0	0	0	2543

*** BREAK ***

16:00	0	244	73	317	108	0	95	203	63	172	0	235	0	0	0	0	755
16:15	0	228	84	312	75	0	82	157	65	147	0	212	0	0	0	0	681
16:30	0	237	104	341	73	0	100	173	73	149	0	222	0	0	0	0	736
16:45	0	297	73	370	82	0	80	162	48	191	0	239	0	0	0	0	771
Total	0	1006	334	1340	338	0	357	695	249	659	0	908	0	0	0	0	2943
17:00	0	278	101	379	90	0	106	196	56	179	0	235	0	0	0	0	810
17:15	0	323	97	420	69	0	93	162	67	164	0	231	0	0	0	0	813
17:30	0	270	90	360	66	0	148	214	70	186	0	256	0	0	0	0	830
17:45	0	317	95	412	83	0	120	203	88	167	0	255	0	0	0	0	870
Total	0	1188	383	1571	308	0	467	775	281	696	0	977	0	0	0	0	3323
Grand Total	0	3219	1233	4452	1352	0	1235	2587	1147	3865	0	5012	0	0	0	0	12051
Apprch %	0	72.3	27.7		52.3	0	47.7		22.9	77.1	0		0	0	0		
Total %	0	26.7	10.2	36.9	11.2	0	10.2	21.5	9.5	32.1	0	41.6	0	0	0	0	

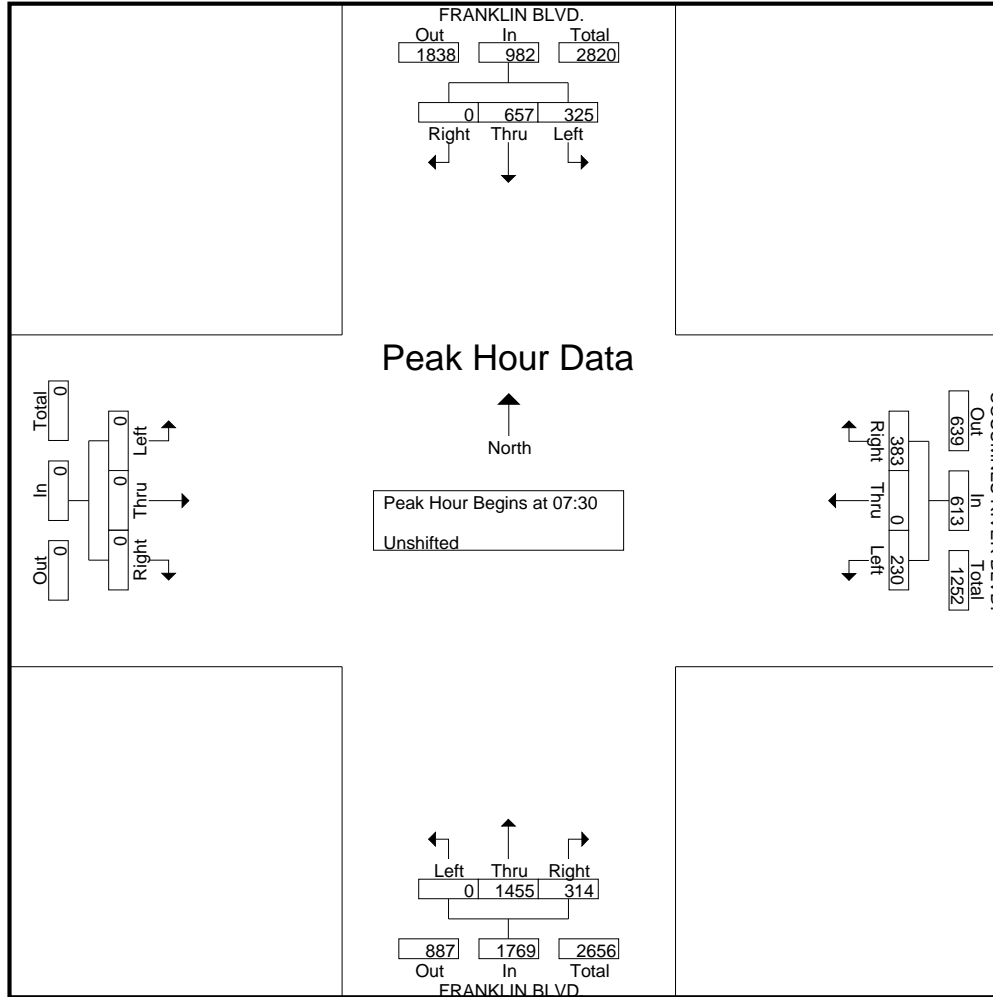
Start Time	FRANKLIN BLVD. Southbound				COSUMNES RIVER BLVD. Westbound				FRANKLIN BLVD. Northbound				Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30																	
07:30	0	224	103	327	104	0	60	164	72	353	0	425	0	0	0	0	916
07:45	0	224	117	341	89	0	57	146	70	406	0	476	0	0	0	0	963
08:00	0	103	64	167	94	0	55	149	51	313	0	364	0	0	0	0	680
08:15	0	106	41	147	96	0	58	154	121	383	0	504	0	0	0	0	805
Total Volume	0	657	325	982	383	0	230	613	314	1455	0	1769	0	0	0	0	3364
% App. Total	0	66.9	33.1		62.5	0	37.5		17.8	82.2	0		0	0	0		
PHF	.000	.733	.694	.720	.921	.000	.958	.934	.649	.896	.000	.877	.000	.000	.000	.000	.873

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Site Code : 00000000
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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

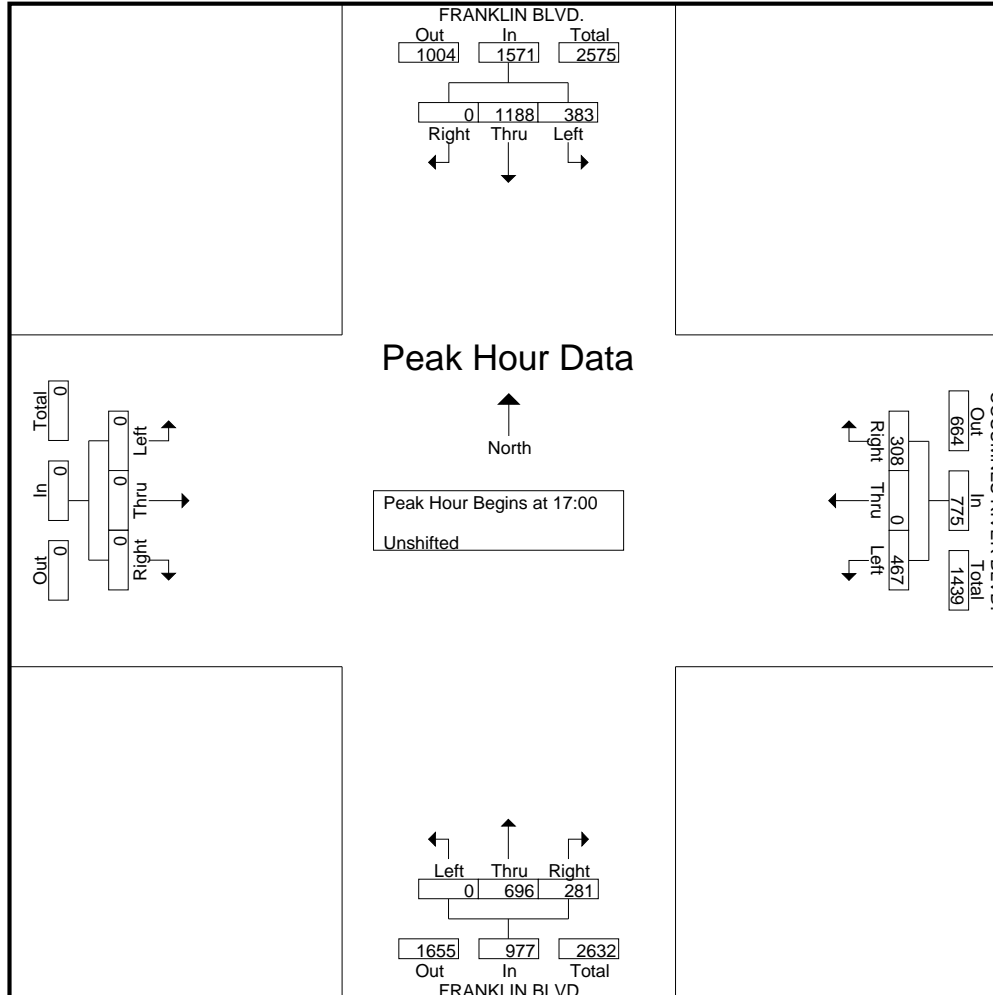
17:00	0	278	101	379	90	0	106	196	56	179	0	235	0	0	0	0	810
17:15	0	323	97	420	69	0	93	162	67	164	0	231	0	0	0	0	813
17:30	0	270	90	360	66	0	148	214	70	186	0	256	0	0	0	0	830
17:45	0	317	95	412	83	0	120	203	88	167	0	255	0	0	0	0	870
Total Volume	0	1188	383	1571	308	0	467	775	281	696	0	977	0	0	0	0	3323
% App. Total	0	75.6	24.4		39.7	0	60.3		28.8	71.2	0		0	0	0		
PHF	.000	.920	.948	.935	.856	.000	.789	.905	.798	.935	.000	.954	.000	.000	.000	.000	.955

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File Name : COSUMNES-FRANKLIN-F
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File Name : MACK-FRANKLIN-F
Site Code : 00000000
Start Date : 2/8/2007
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Groups Printed- Unshifted

Start Time	FRANKLIN BLVD. Southbound				MACK RD. Westbound				FRANKLIN BLVD. Northbound				MACK RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00 AM	10	75	21	106	40	132	20	192	38	251	135	424	9	60	20	89	811
07:15 AM	12	81	27	120	48	185	25	258	41	255	144	440	12	65	27	104	922
07:30 AM	15	90	26	131	50	211	28	289	44	249	154	447	19	91	31	141	1008
07:45 AM	20	76	33	129	50	197	40	287	49	244	159	452	28	71	36	135	1003
Total	57	322	107	486	188	725	113	1026	172	999	592	1763	68	287	114	469	3744
08:00 AM	13	48	35	96	48	158	19	225	46	256	155	457	11	59	42	112	890
08:15 AM	8	58	23	89	40	133	19	192	59	227	144	430	10	61	30	101	812
08:30 AM	4	76	28	108	40	124	32	196	33	204	130	367	8	73	27	108	779
08:45 AM	10	76	35	121	44	104	28	176	22	170	114	306	15	78	41	134	737
Total	35	258	121	414	172	519	98	789	160	857	543	1560	44	271	140	455	3218

*** BREAK ***

04:00 PM	24	178	91	293	45	199	54	298	35	136	99	270	116	174	28	318	1179
04:15 PM	13	167	76	256	65	179	64	308	41	124	112	277	122	221	32	375	1216
04:30 PM	26	194	88	308	57	180	50	287	27	118	109	254	166	191	26	383	1232
04:45 PM	31	166	75	272	66	195	72	333	32	109	110	251	168	246	31	445	1301
Total	94	705	330	1129	233	753	240	1226	135	487	430	1052	572	832	117	1521	4928
05:00 PM	25	211	93	329	49	205	65	319	39	135	113	287	184	205	25	414	1349
05:15 PM	32	231	73	336	40	208	74	322	42	116	106	264	170	173	23	366	1288
05:30 PM	26	215	85	326	54	167	72	293	39	121	109	269	155	162	21	338	1226
05:45 PM	39	166	64	269	63	194	64	321	41	108	102	251	158	224	29	411	1252
Total	122	823	315	1260	206	774	275	1255	161	480	430	1071	667	764	98	1529	5115
Grand Total	308	2108	873	3289	799	2771	726	4296	628	2823	1995	5446	1351	2154	469	3974	17005
Apprch %	9.4	64.1	26.5		18.6	64.5	16.9		11.5	51.8	36.6		34	54.2	11.8		
Total %	1.8	12.4	5.1	19.3	4.7	16.3	4.3	25.3	3.7	16.6	11.7	32	7.9	12.7	2.8	23.4	

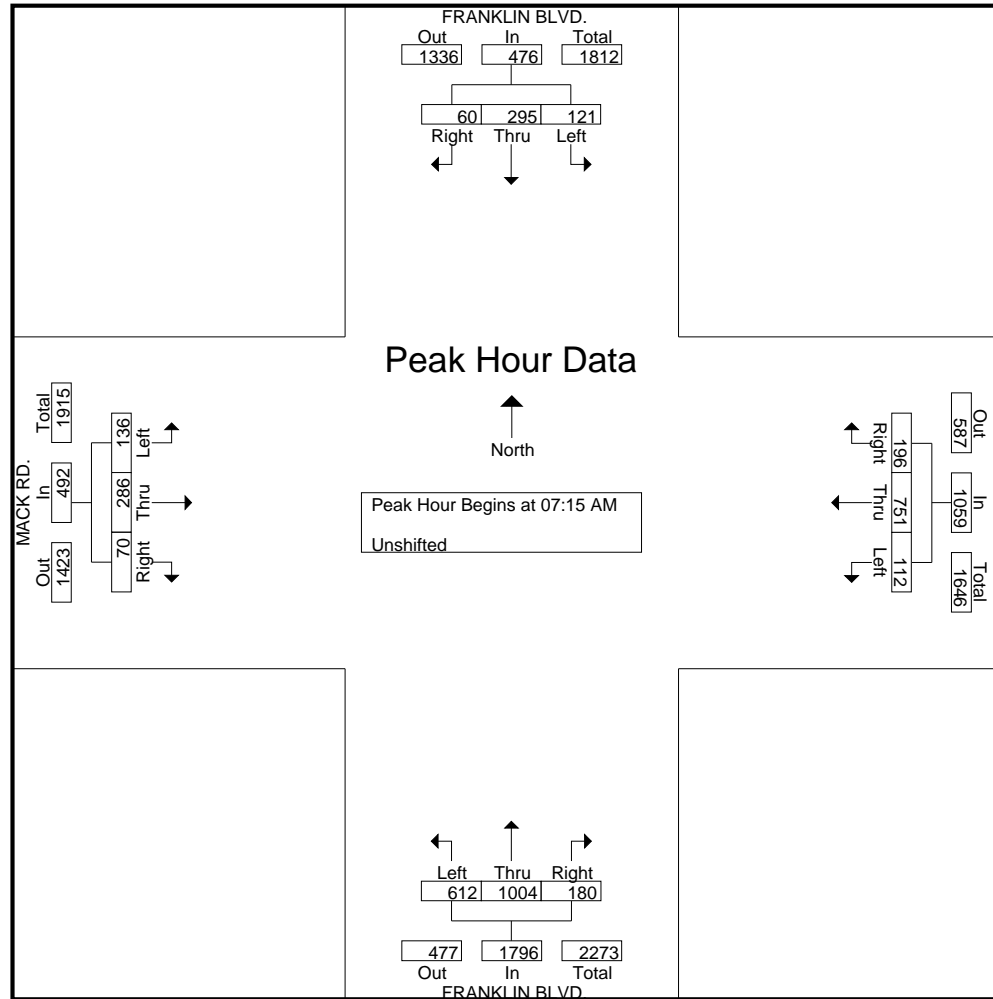
Start Time	FRANKLIN BLVD. Southbound				MACK RD. Westbound				FRANKLIN BLVD. Northbound				MACK RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	12	81	27	120	48	185	25	258	41	255	144	440	12	65	27	104	922
07:30 AM	15	90	26	131	50	211	28	289	44	249	154	447	19	91	31	141	1008
07:45 AM	20	76	33	129	50	197	40	287	49	244	159	452	28	71	36	135	1003
08:00 AM	13	48	35	96	48	158	19	225	46	256	155	457	11	59	42	112	890
Total Volume	60	295	121	476	196	751	112	1059	180	1004	612	1796	70	286	136	492	3823
% App. Total	12.6	62	25.4		18.5	70.9	10.6		10	55.9	34.1		14.2	58.1	27.6		
PHF	.750	.819	.864	.908	.980	.890	.700	.916	.918	.980	.962	.982	.625	.786	.810	.872	.948

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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:30 PM

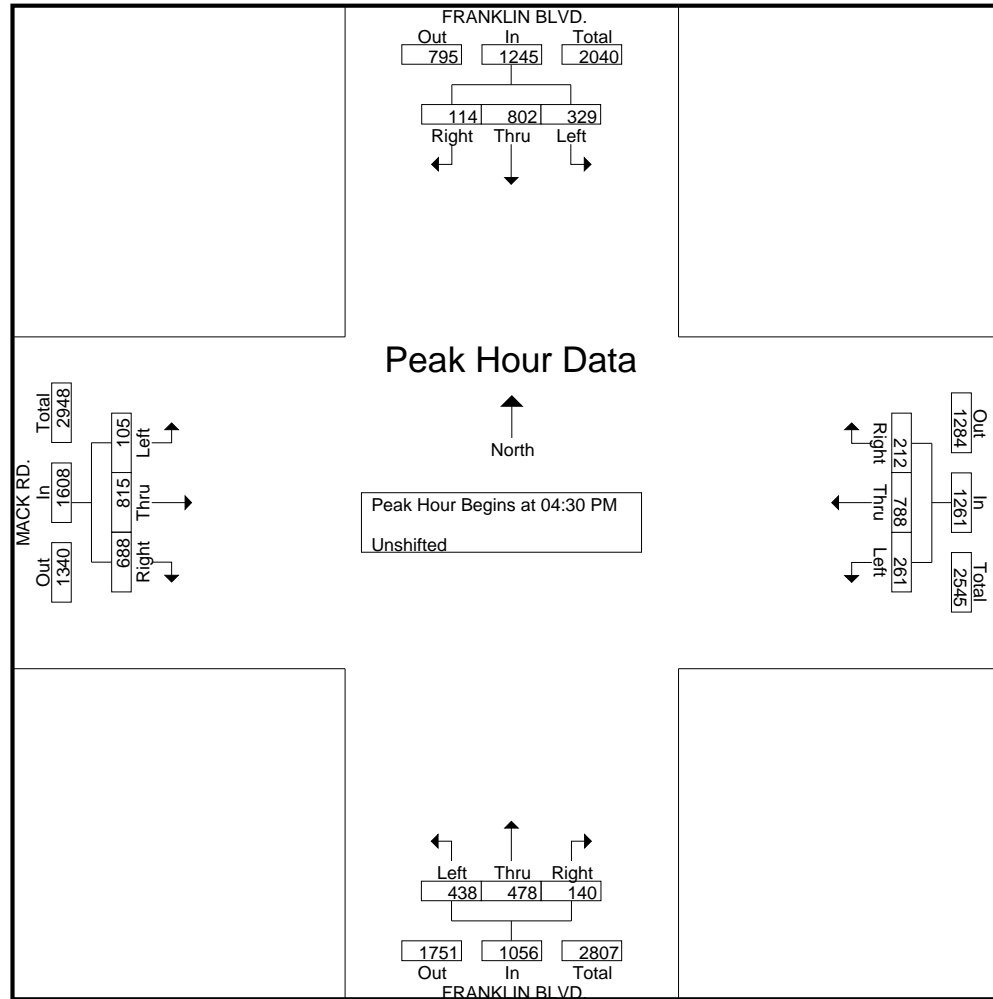
04:30 PM	26	194	88	308	57	180	50	287	27	118	109	254	166	191	26	383	1232
04:45 PM	31	166	75	272	66	195	72	333	32	109	110	251	168	246	31	445	1301
05:00 PM	25	211	93	329	49	205	65	319	39	135	113	287	184	205	25	414	1349
05:15 PM	32	231	73	336	40	208	74	322	42	116	106	264	170	173	23	366	1288
Total Volume	114	802	329	1245	212	788	261	1261	140	478	438	1056	688	815	105	1608	5170
% App. Total	9.2	64.4	26.4		16.8	62.5	20.7		13.3	45.3	41.5		42.8	50.7	6.5		
PHF	.891	.868	.884	.926	.803	.947	.882	.947	.833	.885	.969	.920	.935	.828	.847	.903	.958

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Site Code : 00000000

Start Date : 2/7/2007

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Groups Printed- Unshifted

Start Time	24TH ST. Southbound				MEADOWVIEW RD. Westbound				24TH ST. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	17	20	32	69	52	188	17	257	15	32	13	60	9	145	41	195	581
07:15	19	21	35	75	60	206	27	293	15	40	21	76	11	138	48	197	641
07:30	16	39	38	93	103	245	36	384	35	59	13	107	7	202	42	251	835
07:45	24	40	50	114	116	242	55	413	36	85	20	141	18	164	61	243	911
Total	76	120	155	351	331	881	135	1347	101	216	67	384	45	649	192	886	2968
08:00	31	25	44	100	116	202	21	339	23	42	13	78	17	156	46	219	736
08:15	29	14	47	90	81	186	19	286	17	40	7	64	17	150	32	199	639
08:30	24	20	38	82	75	185	16	276	15	30	8	53	5	128	29	162	573
08:45	18	11	34	63	51	158	11	220	19	35	7	61	10	128	43	181	525
Total	102	70	163	335	323	731	67	1121	74	147	35	256	49	562	150	761	2473

*** BREAK ***

16:00	51	42	101	194	71	167	17	255	34	32	6	72	9	231	40	280	801
16:15	44	54	106	204	59	191	31	281	17	28	9	54	10	235	38	283	822
16:30	45	44	127	216	52	187	32	271	21	34	18	73	14	209	48	271	831
16:45	39	43	107	189	55	194	31	280	25	39	14	78	16	263	45	324	871
Total	179	183	441	803	237	739	111	1087	97	133	47	277	49	938	171	1158	3325
17:00	61	53	133	247	58	166	37	261	17	28	16	61	18	220	34	272	841
17:15	48	74	120	242	55	227	23	305	28	44	5	77	13	256	31	300	924
17:30	66	47	150	263	54	174	34	262	21	30	8	59	10	224	43	277	861
17:45	56	50	160	266	56	194	27	277	24	31	14	69	18	206	52	276	888
Total	231	224	563	1018	223	761	121	1105	90	133	43	266	59	906	160	1125	3514
Grand Total	588	597	1322	2507	1114	3112	434	4660	362	629	192	1183	202	3055	673	3930	12280
Apprch %	23.5	23.8	52.7		23.9	66.8	9.3		30.6	53.2	16.2		5.1	77.7	17.1		
Total %	4.8	4.9	10.8	20.4	9.1	25.3	3.5	37.9	2.9	5.1	1.6	9.6	1.6	24.9	5.5	32	

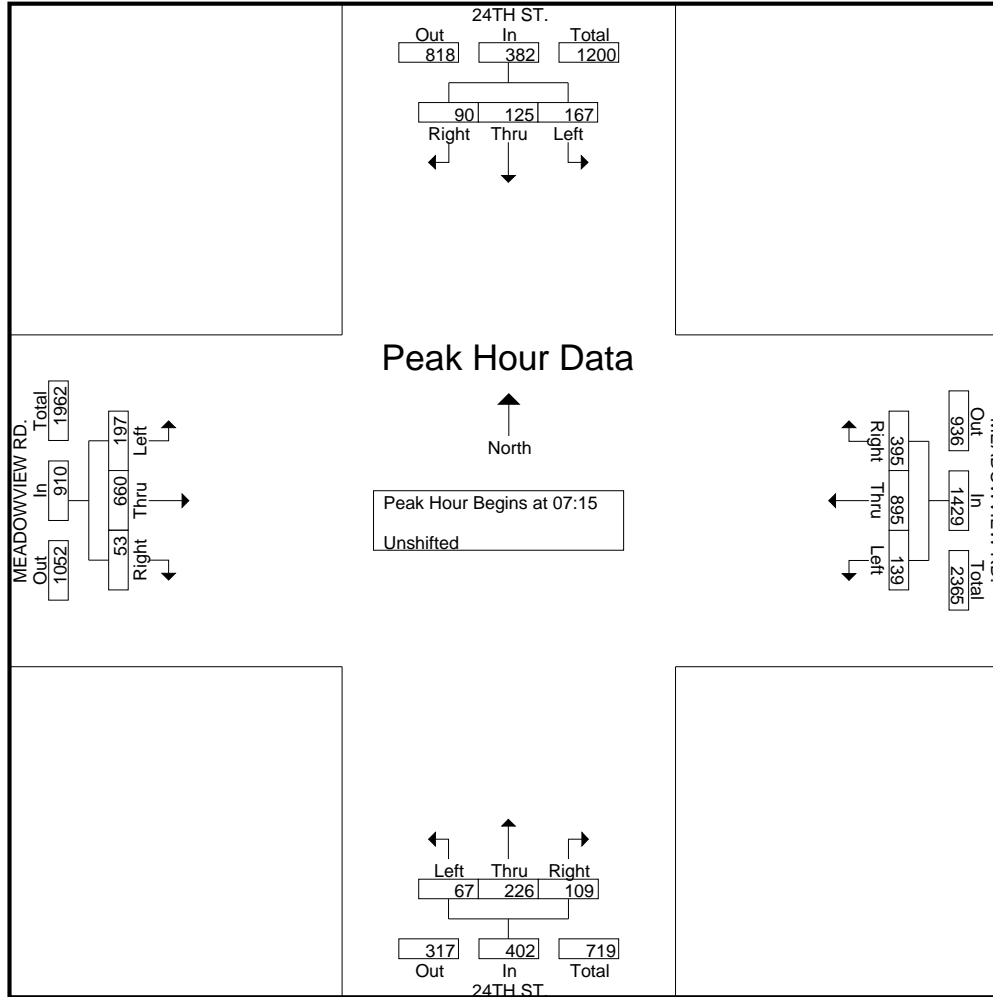
Start Time	24TH ST. Southbound				MEADOWVIEW RD. Westbound				24TH ST. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	19	21	35	75	60	206	27	293	15	40	21	76	11	138	48	197	641
07:30	16	39	38	93	103	245	36	384	35	59	13	107	7	202	42	251	835
07:45	24	40	50	114	116	242	55	413	36	85	20	141	18	164	61	243	911
08:00	31	25	44	100	116	202	21	339	23	42	13	78	17	156	46	219	736
Total Volume	90	125	167	382	395	895	139	1429	109	226	67	402	53	660	197	910	3123
% App. Total	23.6	32.7	43.7		27.6	62.6	9.7		27.1	56.2	16.7		5.8	72.5	21.6		
PHF	.726	.781	.835	.838	.851	.913	.632	.865	.757	.665	.798	.713	.736	.817	.807	.906	.857

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

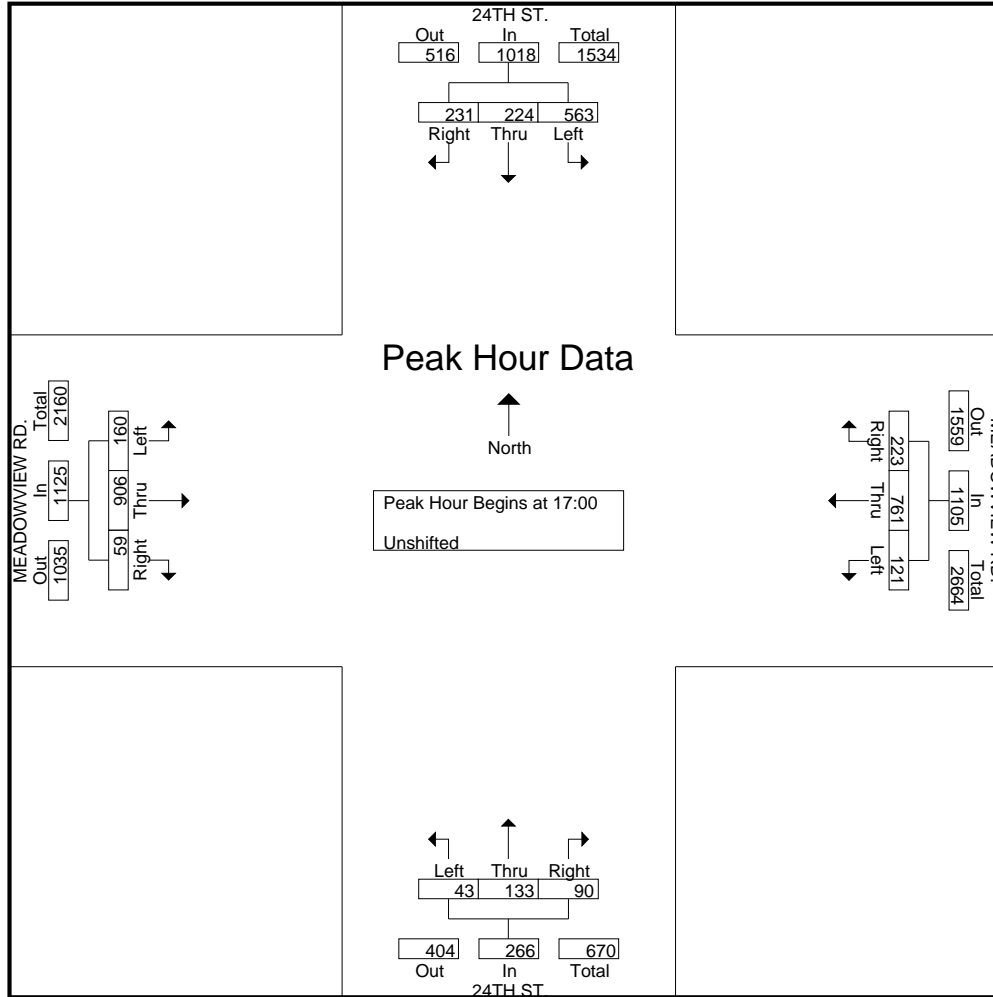
17:00	61	53	133	247	58	166	37	261	17	28	16	61	18	220	34	272	841
17:15	48	74	120	242	55	227	23	305	28	44	5	77	13	256	31	300	924
17:30	66	47	150	263	54	174	34	262	21	30	8	59	10	224	43	277	861
17:45	56	50	160	266	56	194	27	277	24	31	14	69	18	206	52	276	888
Total Volume	231	224	563	1018	223	761	121	1105	90	133	43	266	59	906	160	1125	3514
% App. Total	22.7	22	55.3		20.2	68.9	11		33.8	50	16.2		5.2	80.5	14.2		
PHF	.875	.757	.880	.957	.961	.838	.818	.906	.804	.756	.672	.864	.819	.885	.769	.938	.951

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Groups Printed- Unshifted

Start Time	DETROIT BLVD. Southbound				MEADOWVIEW RD. Westbound				DETROIT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00	6	0	8	14	35	292	11	338	26	1	15	42	9	195	10	214	608
07:15	10	2	12	24	76	339	14	429	35	1	18	54	11	180	16	207	714
07:30	4	1	16	21	65	369	33	467	49	1	45	95	25	268	11	304	887
07:45	12	0	10	22	42	380	27	449	50	1	43	94	34	223	6	263	828
Total	32	3	46	81	218	1380	85	1683	160	4	121	285	79	866	43	988	3037
08:00	6	0	13	19	46	347	29	422	31	0	25	56	18	228	13	259	756
08:15	6	0	9	15	21	255	18	294	31	2	21	54	9	198	3	210	573
08:30	5	1	9	15	35	282	13	330	30	1	11	42	5	207	3	215	602
08:45	3	0	7	10	19	234	12	265	24	1	11	36	7	198	6	211	522
Total	20	1	38	59	121	1118	72	1311	116	4	68	188	39	831	25	895	2453

*** BREAK ***

16:00	18	29	45	92	19	252	33	304	36	10	17	63	21	346	15	382	841
16:15	14	1	56	71	16	253	42	311	34	0	16	50	26	361	4	391	823
16:30	13	1	52	66	11	258	36	305	38	0	19	57	23	332	7	362	790
16:45	12	0	57	69	17	266	52	335	36	1	25	62	26	304	6	336	802
Total	57	31	210	298	63	1029	163	1255	144	11	77	232	96	1343	32	1471	3256
17:00	11	3	80	94	18	287	44	349	30	1	17	48	15	360	6	381	872
17:15	14	1	61	76	27	308	36	371	39	0	17	56	20	427	4	451	954
17:30	16	1	70	87	13	260	38	311	34	0	16	50	16	361	4	381	829
17:45	14	0	78	92	15	284	39	338	32	1	14	47	20	338	7	365	842
Total	55	5	289	349	73	1139	157	1369	135	2	64	201	71	1486	21	1578	3497
Grand Total	164	40	583	787	475	4666	477	5618	555	21	330	906	285	4526	121	4932	12243
Apprch %	20.8	5.1	74.1		8.5	83.1	8.5		61.3	2.3	36.4		5.8	91.8	2.5		
Total %	1.3	0.3	4.8	6.4	3.9	38.1	3.9	45.9	4.5	0.2	2.7	7.4	2.3	37	1	40.3	

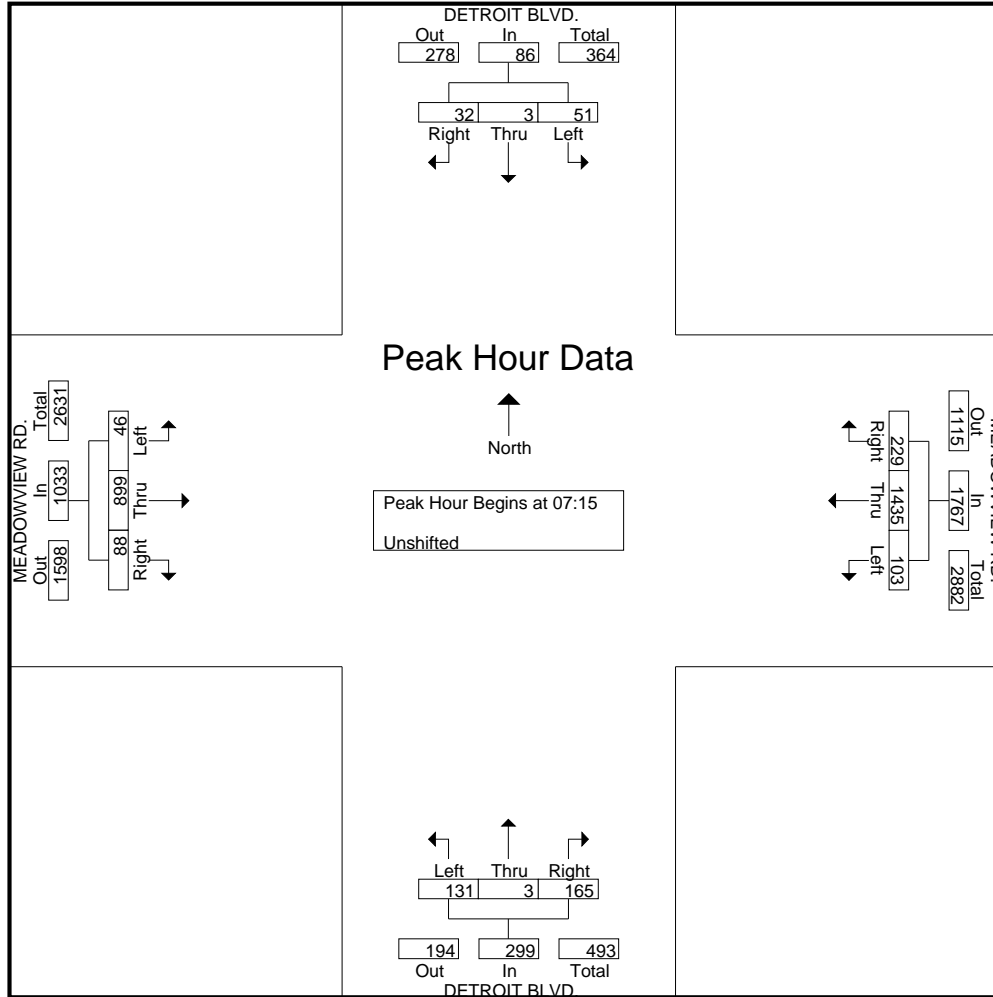
Start Time	DETROIT BLVD. Southbound				MEADOWVIEW RD. Westbound				DETROIT BLVD. Northbound				MEADOWVIEW RD. Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	10	2	12	24	76	339	14	429	35	1	18	54	11	180	16	207	714
07:30	4	1	16	21	65	369	33	467	49	1	45	95	25	268	11	304	887
07:45	12	0	10	22	42	380	27	449	50	1	43	94	34	223	6	263	828
08:00	6	0	13	19	46	347	29	422	31	0	25	56	18	228	13	259	756
Total Volume	32	3	51	86	229	1435	103	1767	165	3	131	299	88	899	46	1033	3185
% App. Total	37.2	3.5	59.3		13	81.2	5.8		55.2	1	43.8		8.5	87	4.5		
PHF	.667	.375	.797	.896	.753	.944	.780	.946	.825	.750	.728	.787	.647	.839	.719	.850	.898

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Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 17:00

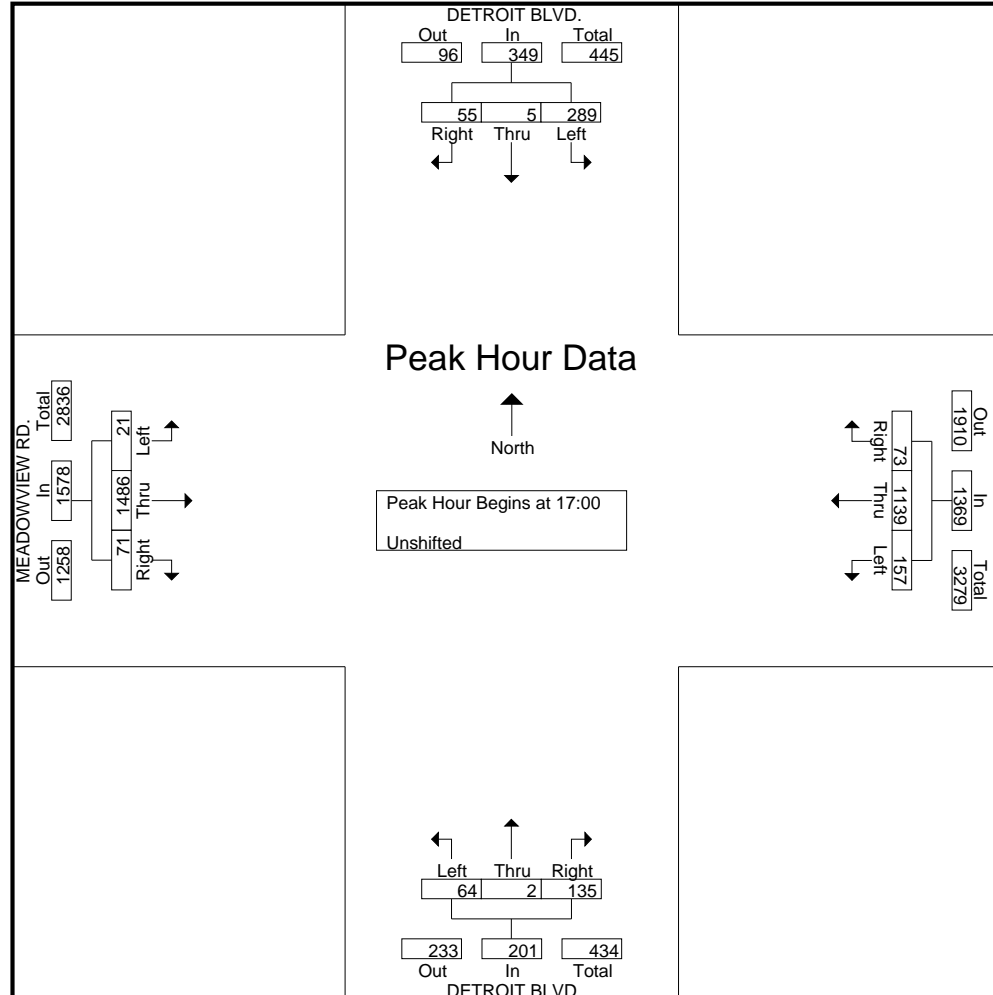
17:00	11	3	80	94	18	287	44	349	30	1	17	48	15	360	6	381	872
17:15	14	1	61	76	27	308	36	371	39	0	17	56	20	427	4	451	954
17:30	16	1	70	87	13	260	38	311	34	0	16	50	16	361	4	381	829
17:45	14	0	78	92	15	284	39	338	32	1	14	47	20	338	7	365	842
Total Volume	55	5	289	349	73	1139	157	1369	135	2	64	201	71	1486	21	1578	3497
% App. Total	15.8	1.4	82.8		5.3	83.2	11.5		67.2	1	31.8		4.5	94.2	1.3		
PHF	.859	.417	.903	.928	.676	.925	.892	.923	.865	.500	.941	.897	.888	.870	.750	.875	.916

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #1: Pocket Road/I-5 SB Ramps

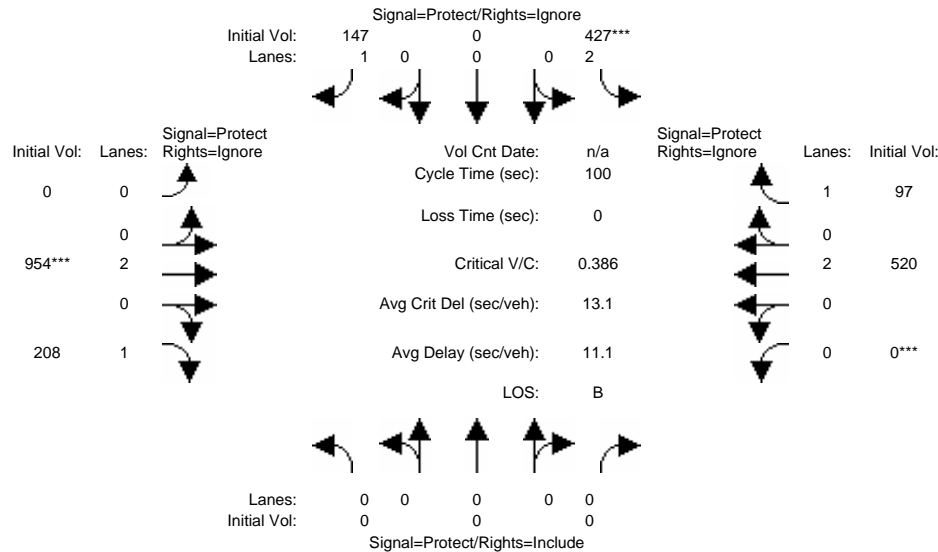


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.32	0.00	0.00	0.00	0.68	0.00	0.00	0.68	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	4.8	0.0	0.0	0.0	6.0	0.0	0.0	2.8	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.6	0.0	0.0	0.0	0.6	0.0	0.0	0.3	0.0
HCM2KQueue:	0.0	0.0	0.0	5.4	0.0	0.0	0.0	6.6	0.0	0.0	3.1	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.20	1.20	1.18	1.20	1.20	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	6.4	0.0	0.0	0.0	7.8	0.0	0.0	3.7	0.0
85thFactor:	1.60	1.60	1.60	1.55	1.60	1.60	1.60	1.54	1.60	1.60	1.57	1.60
85thHCM2kQ:	0.0	0.0	0.0	8.4	0.0	0.0	0.0	10.2	0.0	0.0	4.8	0.0
90thFactor:	1.80	1.80	1.80	1.71	1.80	1.80	1.80	1.69	1.80	1.80	1.74	1.80
90thHCM2kQ:	0.0	0.0	0.0	9.2	0.0	0.0	0.0	11.2	0.0	0.0	5.4	0.0
95thFactor:	2.10	2.10	2.10	1.94	2.10	2.10	2.10	1.92	2.10	2.10	2.01	2.10
95thHCM2kQ:	0.0	0.0	0.0	10.5	0.0	0.0	0.0	12.7	0.0	0.0	6.2	0.0
98thFactor:	2.70	2.70	2.70	2.36	2.70	2.70	2.70	2.30	2.70	2.70	2.49	2.70
98thHCM2kQ:	0.0	0.0	0.0	12.7	0.0	0.0	0.0	15.2	0.0	0.0	7.6	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	83.2	0.0	0.0	0.0	102	0.0	0.0	48.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 36.662 pounds
 5.939 gallons
 Carbon Dioxide: 114.386 pounds
 Carbon Monoxide: 8.270 pounds
 Hydrocarbons: 1.306 pounds
 Nitrogen Oxides: 0.357 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 33.462 pounds
 5.421 gallons
 Carbon Dioxide: 104.400 pounds
 Carbon Monoxide: 8.025 pounds
 Hydrocarbons: 1.254 pounds
 Nitrogen Oxides: 0.313 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing PM

Intersection #1: Pocket Road/I-5 SB Ramps

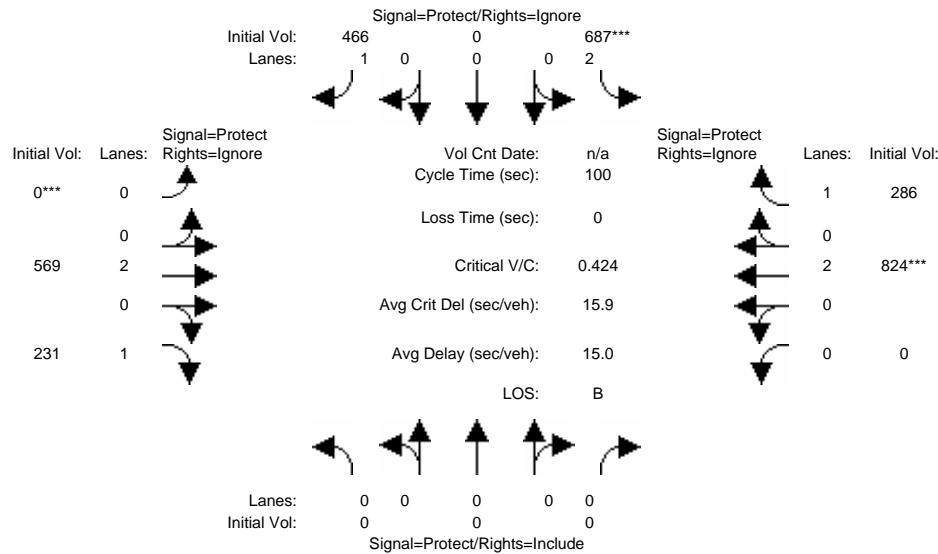


Table containing traffic engineering data including: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.46	0.00	0.00	0.00	0.54	0.00	0.00	0.54	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	6.6	0.0	0.0	0.0	4.6	0.0	0.0	7.2	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.7	0.0	0.0	0.0	0.4	0.0	0.0	0.7	0.0
HCM2KQueue:	0.0	0.0	0.0	7.3	0.0	0.0	0.0	5.0	0.0	0.0	7.9	0.0
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.19	1.20	1.20	1.18	1.20
70thHCM2kQ:	0.0	0.0	0.0	8.6	0.0	0.0	0.0	5.9	0.0	0.0	9.4	0.0
85thFactor:	1.60	1.60	1.60	1.54	1.60	1.60	1.60	1.55	1.60	1.60	1.53	1.60
85thHCM2kQ:	0.0	0.0	0.0	11.2	0.0	0.0	0.0	7.7	0.0	0.0	12.2	0.0
90thFactor:	1.80	1.80	1.80	1.68	1.80	1.80	1.80	1.71	1.80	1.80	1.67	1.80
90thHCM2kQ:	0.0	0.0	0.0	12.3	0.0	0.0	0.0	8.5	0.0	0.0	13.3	0.0
95thFactor:	2.10	2.10	2.10	1.90	2.10	2.10	2.10	1.96	2.10	2.10	1.89	2.10
95thHCM2kQ:	0.0	0.0	0.0	13.9	0.0	0.0	0.0	9.7	0.0	0.0	15.0	0.0
98thFactor:	2.70	2.70	2.70	2.27	2.70	2.70	2.70	2.38	2.70	2.70	2.24	2.70
98thHCM2kQ:	0.0	0.0	0.0	16.6	0.0	0.0	0.0	11.9	0.0	0.0	17.8	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	114.9	0.0	0.0	0.0	78.1	0.0	0.0	123	0.0

Name: year 1995 composite fleet
Fuel Consumption: 46.538 pounds
7.539 gallons
Carbon Dioxide: 145.197 pounds
Carbon Monoxide: 10.839 pounds
Hydrocarbons: 1.810 pounds
Nitrogen Oxides: 0.445 pounds

Name: year 2000 composite fleet
Fuel Consumption: 42.921 pounds
6.953 gallons
Carbon Dioxide: 133.915 pounds
Carbon Monoxide: 10.559 pounds
Hydrocarbons: 1.753 pounds
Nitrogen Oxides: 0.392 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #2: Meadowview Road/I-5 NB Ramps

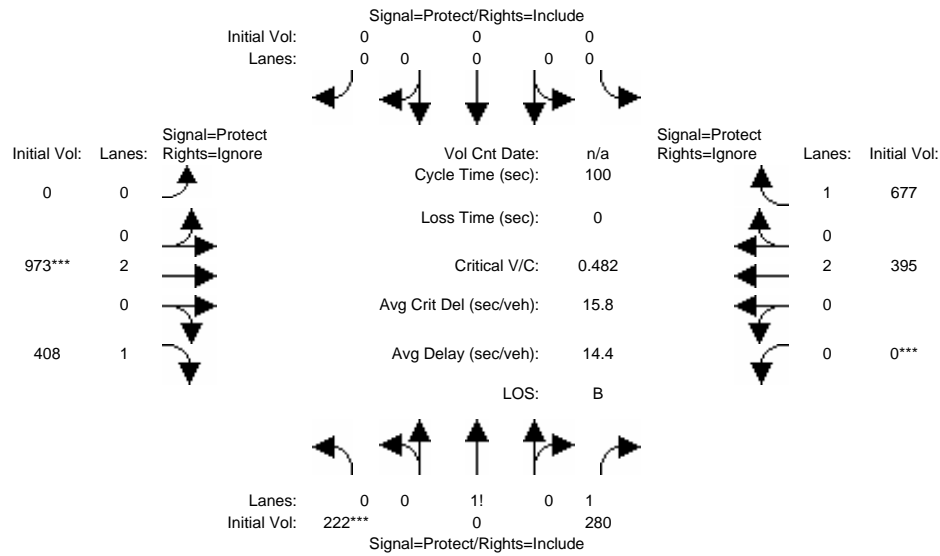


Table containing detailed traffic engineering data including: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.44	0.00	0.44	0.00	0.00	0.00	0.00	0.56	0.00	0.00	0.56	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	7.1	0.0	3.6	0.0	0.0	0.0	0.0	8.6	0.0	0.0	2.9	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.9	0.0	0.4	0.0	0.0	0.0	0.0	0.9	0.0	0.0	0.2	0.0
HCM2KQueue:	8.1	0.0	3.9	0.0	0.0	0.0	0.0	9.5	0.0	0.0	3.1	0.0
70thFactor:	1.18	1.20	1.19	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.19	1.20
70thHCM2kQ:	9.5	0.0	4.7	0.0	0.0	0.0	0.0	11.2	0.0	0.0	3.7	0.0
85thFactor:	1.53	1.60	1.56	1.60	1.60	1.60	1.60	1.52	1.60	1.60	1.57	1.60
85thHCM2kQ:	12.3	0.0	6.1	0.0	0.0	0.0	0.0	14.4	0.0	0.0	4.9	0.0
90thFactor:	1.67	1.80	1.73	1.80	1.80	1.80	1.80	1.65	1.80	1.80	1.74	1.80
90thHCM2kQ:	13.4	0.0	6.8	0.0	0.0	0.0	0.0	15.7	0.0	0.0	5.4	0.0
95thFactor:	1.88	2.10	1.98	2.10	2.10	2.10	2.10	1.85	2.10	2.10	2.00	2.10
95thHCM2kQ:	15.2	0.0	7.8	0.0	0.0	0.0	0.0	17.6	0.0	0.0	6.2	0.0
98thFactor:	2.24	2.70	2.44	2.70	2.70	2.70	2.70	2.18	2.70	2.70	2.49	2.70
98thHCM2kQ:	18.0	0.0	9.6	0.0	0.0	0.0	0.0	20.7	0.0	0.0	7.7	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	39.4	0.0	44.4	0.0	0.0	0.0	0.0	147	0.0	0.0	48.9	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 40.962 pounds
 6.636 gallons
 Carbon Dioxide: 127.801 pounds
 Carbon Monoxide: 9.498 pounds
 Hydrocarbons: 1.573 pounds
 Nitrogen Oxides: 0.394 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 37.720 pounds
 6.111 gallons
 Carbon Dioxide: 117.686 pounds
 Carbon Monoxide: 9.248 pounds
 Hydrocarbons: 1.522 pounds
 Nitrogen Oxides: 0.347 pounds

DISCLAIMER
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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.41	0.00	0.41	0.00	0.00	0.00	0.00	0.59	0.00	0.00	0.59	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	7.3	0.0	2.6	0.0	0.0	0.0	0.0	9.0	0.0	0.0	6.5	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	1.0	0.0	0.3	0.0	0.0	0.0	0.0	1.0	0.0	0.0	0.6	0.0
HCM2KQueue:	8.3	0.0	2.8	0.0	0.0	0.0	0.0	10.0	0.0	0.0	7.2	0.0
70thFactor:	1.18	1.20	1.19	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.18	1.20
70thHCM2kQ:	9.8	0.0	3.4	0.0	0.0	0.0	0.0	11.8	0.0	0.0	8.5	0.0
85thFactor:	1.53	1.60	1.57	1.60	1.60	1.60	1.60	1.51	1.60	1.60	1.54	1.60
85thHCM2kQ:	12.7	0.0	4.4	0.0	0.0	0.0	0.0	15.2	0.0	0.0	11.0	0.0
90thFactor:	1.66	1.80	1.75	1.80	1.80	1.80	1.80	1.64	1.80	1.80	1.68	1.80
90thHCM2kQ:	13.8	0.0	4.9	0.0	0.0	0.0	0.0	16.5	0.0	0.0	12.1	0.0
95thFactor:	1.88	2.10	2.01	2.10	2.10	2.10	2.10	1.84	2.10	2.10	1.90	2.10
95thHCM2kQ:	15.6	0.0	5.7	0.0	0.0	0.0	0.0	18.5	0.0	0.0	13.7	0.0
98thFactor:	2.23	2.70	2.51	2.70	2.70	2.70	2.70	2.16	2.70	2.70	2.28	2.70
98thHCM2kQ:	18.5	0.0	7.1	0.0	0.0	0.0	0.0	21.7	0.0	0.0	16.4	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	49.3	0.0	29.0	0.0	0.0	0.0	0.0	154	0.0	0.0	112	0.0

Name: year 1995 composite fleet
Fuel Consumption: 49.748 pounds
8.059 gallons
Carbon Dioxide: 155.215 pounds
Carbon Monoxide: 11.461 pounds
Hydrocarbons: 1.874 pounds
Nitrogen Oxides: 0.484 pounds

Name: year 2000 composite fleet
Fuel Consumption: 45.704 pounds
7.404 gallons
Carbon Dioxide: 142.596 pounds
Carbon Monoxide: 11.149 pounds
Hydrocarbons: 1.809 pounds
Nitrogen Oxides: 0.425 pounds

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.08	0.08	0.08	0.07	0.08	0.08	0.37	0.63	0.63	0.05	0.32	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	2.4	2.6	2.6	2.1	2.3	2.3	9.5	6.6	6.6	1.0	9.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	1.4	1.4	1.4	1.4	1.4	1.4	1.6	0.7	0.7	0.6	1.6	0.0
HCM2KQueue:	3.8	4.0	4.0	3.5	3.7	3.7	11.1	7.3	7.3	1.7	10.6	0.0
70thFactor:	1.19	1.19	1.19	1.19	1.19	1.19	1.18	1.18	1.18	1.20	1.18	1.20
70thHCM2kQ:	4.5	4.8	4.8	4.1	4.4	4.4	13.0	8.6	8.6	2.0	12.4	0.0
85thFactor:	1.56	1.56	1.56	1.57	1.57	1.57	1.51	1.54	1.54	1.58	1.51	1.60
85thHCM2kQ:	5.9	6.2	6.2	5.4	5.8	5.8	16.7	11.2	11.2	2.6	16.0	0.0
90thFactor:	1.73	1.73	1.73	1.74	1.73	1.73	1.63	1.68	1.68	1.77	1.64	1.80
90thHCM2kQ:	6.5	6.9	6.9	6.0	6.4	6.4	18.0	12.2	12.2	2.9	17.3	0.0
95thFactor:	1.99	1.98	1.98	1.99	1.99	1.99	1.82	1.90	1.90	2.05	1.83	2.10
95thHCM2kQ:	7.5	7.9	7.9	6.9	7.4	7.4	20.2	13.8	13.8	3.4	19.4	0.0
98thFactor:	2.45	2.44	2.44	2.47	2.45	2.45	2.13	2.27	2.27	2.58	2.14	2.70
98thHCM2kQ:	9.2	9.7	9.7	8.6	9.1	9.1	23.5	16.5	16.5	4.3	22.7	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	21.3	30.4	13.8	19.0	28.4	11.4	85.3	97.5	15.4	9.2	154	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 80.279 pounds
 13.005 gallons
 Carbon Dioxide: 250.470 pounds
 Carbon Monoxide: 19.780 pounds
 Hydrocarbons: 3.655 pounds
 Nitrogen Oxides: 0.698 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 75.584 pounds
 12.245 gallons
 Carbon Dioxide: 235.822 pounds
 Carbon Monoxide: 19.413 pounds
 Hydrocarbons: 3.580 pounds
 Nitrogen Oxides: 0.625 pounds

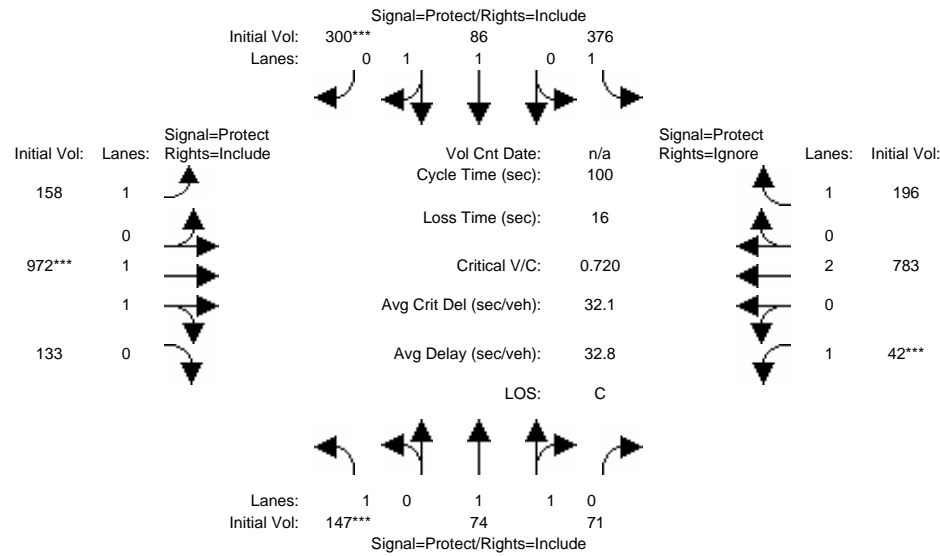
DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Existing PM

Intersection #3: Meadowview Road/Freeport Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound									
Movement:	L	T	R	L	T	R	L	T	R	L	T	R							
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0							
Volume Module:																			
Base Vol:	147	74	71	376	86	300	158	972	133	42	783	196							
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00							
Initial Bse:	147	74	71	376	86	300	158	972	133	42	783	196							
User Adj:	1.00	1.00	0.60	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00							
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00							
PHF Volume:	147	74	43	376	86	300	158	972	133	42	783	0							
Reduced Vol:	0	0	0	0	0	0	0	0	0	0	0	0							
FCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00							
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00							
Final Vol.:	147	74	43	376	86	300	158	972	133	42	783	0							
Saturation Flow Module:																			
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900							
Adjustment:	0.95	0.90	0.90	0.95	0.84	0.84	0.95	0.93	0.93	0.95	0.95	1.00							
Lanes:	1.00	1.27	0.73	1.00	1.00	1.00	1.00	1.76	0.24	1.00	2.00	1.00							
Final Sat.:	1805	2165	1246	1805	1594	1594	1805	3118	427	1805	3610	1900							
Capacity Analysis Module:																			
Vol/Sat:	0.08	0.03	0.03	0.21	0.05	0.19	0.09	0.31	0.31	0.02	0.22	0.00							
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****								
Green/Cycle:	0.11	0.05	0.05	0.32	0.26	0.26	0.13	0.43	0.43	0.03	0.33	0.00							
Volume/Cap:	0.72	0.65	0.65	0.65	0.21	0.72	0.65	0.72	0.72	0.72	0.65	0.00							
Delay/Veh:	54.6	54.4	54.4	31.6	28.9	38.3	47.4	25.0	25.0	83.1	29.8	0.0							
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00							
AdjDel/Veh:	54.6	54.4	54.4	31.6	28.9	38.3	47.4	25.0	25.0	83.1	29.8	0.0							
LOS by Move:	D	D	D	C	C	D	D	C	C	F	C	A							
HCM2kAvgQ:	6	3	3	11	2	10	6	16	16	3	12	0							
HCM Ops Adjusted Lane Utilization Module:																			
Lanes:	1	0	1	0	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1
Lane Group:	L	RT	RT	RT	L	RT	RT	RT	L	RT	RT	RT	L	T	R	L	T	R	
#LnsInGrps:	1	2	2	2	1	2	2	2	1	2	2	2	1	2	1	1	2	1	
HCM Ops Input Saturation Adj Module:																			
Lane Width:	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	
CrosswalkWid	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	
% Hev Veh:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Grade:	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
Parking/Hr:	No	No	No	No	No	No	No	No	No	No	No	No	No	No	No	No	No	No	
Bus Stp/Hr:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Area Type:	<	<	<	<	<	<	<	<	<	Other	>	>	>	>	>	>	>	>	>
Cnft Ped/Hr:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
ExclusiveRT:	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	
% RT Pctct:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
HCM Ops f(lt) Adj Case Module:																			
f(lt) Case:	1	xxxx	xxxx	1	xxxx	xxxx	1	xxxx	xxxx	1	xxxx	xxxx	1	xxxx	xxxx	1	xxxx	xxxx	1
HCM Ops Saturation Adj Module:																			
Ln Wid Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	xxxxxx	1.00	1.00	xxxxxx	
Hev Veh Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	xxxxxx	1.00	1.00	xxxxxx	
Grade Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	xxxxxx	1.00	1.00	xxxxxx	
Parking Adj:	xxxxx	1.00	1.00	xxxxx	1.00	1.00	1.00	xxxxx	1.00	1.00	1.00	xxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	
Bus Stp Adj:	xxxxx	1.00	1.00	xxxxx	1.00	1.00	1.00	xxxxx	1.00	1.00	1.00	xxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	
Area Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	xxxxxx	1.00	1.00	xxxxxx	
RT Adj:	xxxxx	0.95	0.95	xxxxx	0.88	0.88	xxxxx	0.98	0.98	xxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx
LT Adj:	0.95	xxxxx	xxxxxx	0.95	xxxxx	xxxxxx	0.95	xxxxx	xxxxxx	0.95	xxxxx	xxxxxx	0.95	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx
PedBike Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
HCM Sat Adj:	0.95	0.95	0.95	0.95	0.88	0.88	0.95	0.98	0.98	0.98	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Usr Sat Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
MLF Sat Adj:	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	1.00	0.95	0.95	1.00	0.95	1.00	0.95	1.00	0.95		
Fnl Sat Adj:	0.95	0.90	0.90	0.95	0.84	0.84	0.95	0.93	0.93	0.93	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00
Delay Adjustment Factor Module:																			
Coordinated:	<	<	<	<	<	<	<	<	<	No	>	>	>	>	>	>	>	>	>
Signal Type:	<	<	<	<	<	<	<	<	<	Actuated	>	>	>	>	>	>	>	>	>
DelAdjPctr:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.05	0.05	0.32	0.26	0.26	0.13	0.43	0.43	0.03	0.33	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.9	1.7	1.7	8.9	2.0	8.0	4.2	13.3	13.3	1.2	9.8	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	2.0	1.4	1.4	1.7	0.3	2.2	1.6	2.4	2.4	1.5	1.8	0.0
HCM2KQueue:	5.9	3.1	3.1	10.7	2.2	10.2	5.8	15.7	15.7	2.6	11.5	0.0
70thFactor:	1.19	1.19	1.19	1.18	1.19	1.18	1.19	1.17	1.17	1.19	1.17	1.20
70thHCM2kQ:	7.1	3.6	3.6	12.5	2.7	12.0	6.9	18.3	18.3	3.2	13.6	0.0
85thFactor:	1.55	1.57	1.57	1.51	1.58	1.51	1.55	1.48	1.48	1.57	1.50	1.60
85thHCM2kQ:	9.2	4.8	4.8	16.1	3.5	15.5	9.0	23.2	23.2	4.2	17.4	0.0
90thFactor:	1.70	1.74	1.74	1.63	1.76	1.64	1.70	1.58	1.58	1.75	1.62	1.80
90thHCM2kQ:	10.1	5.3	5.3	17.4	3.9	16.8	9.9	24.8	24.8	4.6	18.8	0.0
95thFactor:	1.93	2.01	2.01	1.83	2.03	1.84	1.93	1.75	1.75	2.02	1.82	2.10
95thHCM2kQ:	11.5	6.1	6.1	19.5	4.5	18.8	11.2	27.5	27.5	5.3	21.0	0.0
98thFactor:	2.33	2.49	2.49	2.14	2.54	2.16	2.34	2.00	2.00	2.52	2.11	2.70
98thHCM2kQ:	13.9	7.6	7.6	22.8	5.6	22.0	13.6	31.3	31.3	6.6	24.4	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	35.5	18.1	10.4	80.5	16.8	68.2	37.5	200	27.4	10.4	167	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 110.343 pounds
 17.876 gallons
 Carbon Dioxide: 344.271 pounds
 Carbon Monoxide: 27.679 pounds
 Hydrocarbons: 5.252 pounds
 Nitrogen Oxides: 0.939 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 104.558 pounds
 16.938 gallons
 Carbon Dioxide: 326.220 pounds
 Carbon Monoxide: 27.223 pounds
 Hydrocarbons: 5.161 pounds
 Nitrogen Oxides: 0.846 pounds

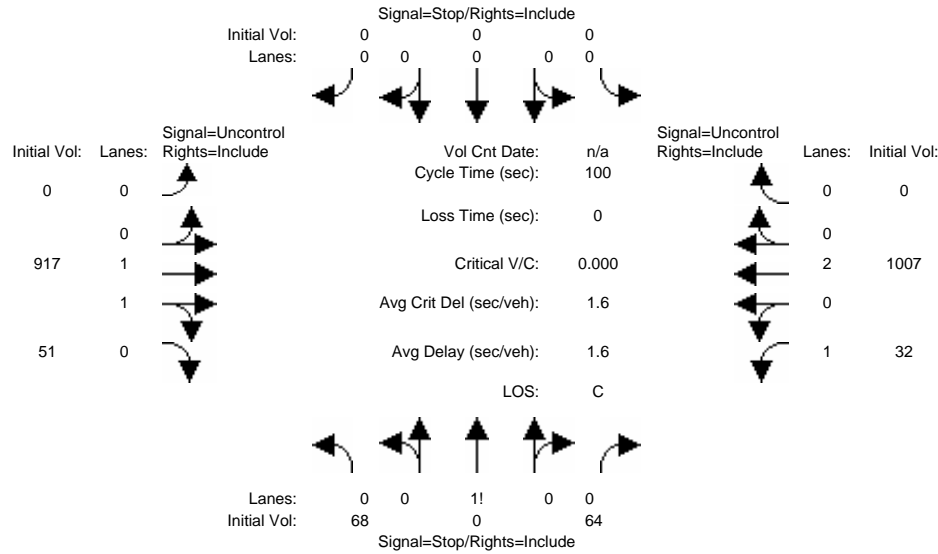
DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Existing AM

Intersection #4: Meadowview Road/Manorside Drive



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	68	0	64	0	0	0	0	917	51	32	1007	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	68	0	64	0	0	0	0	917	51	32	1007	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	68	0	64	0	0	0	0	917	51	32	1007	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	68	0	64	0	0	0	0	917	51	32	1007	0

Critical Gap Module:	North Bound			South Bound			East Bound			West Bound		
Critical Gap:	6.8	xxxxx	6.9	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	4.1	xxxxx	xxxxx
FollowUpTim:	3.5	xxxxx	3.3	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	2.2	xxxxx	xxxxx

Capacity Module:	North Bound			South Bound			East Bound			West Bound		
Cnflict Vol:	1510	xxxxx	484	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	968	xxxxx	xxxxx
Potent Cap.:	113	xxxxx	534	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	720	xxxxx	xxxxx
Move Cap.:	109	xxxxx	534	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	720	xxxxx	xxxxx
Total Cap.:	235	0	xxxxx	0	0	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Volume/Cap:	0.29	xxxxx	0.12	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.04	xxxxx	xxxxx

Level of Service Module:	North Bound			South Bound			East Bound			West Bound		
2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.1	xxxxx	xxxxx
Control Del:	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	10.2	xxxxx	xxxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	B	*	*

Two-Stage Gap Acceptance - Stage One Module:	North Bound			South Bound			East Bound			West Bound		
Cnflict Vol:	943	0	xxxxxx	0	0	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx
Potent Cap.:	344	900	xxxxxx	900	900	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx
Move Cap.:	344	900	xxxxxx	860	860	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx

Two-Stage Gap Acceptance - Stage Two Module:	North Bound			South Bound			East Bound			West Bound		
Cnflict Vol:	568	0	xxxxxx	0	0	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx
Potent Cap.:	536	900	xxxxxx	900	900	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx
Move Cap.:	513	860	xxxxxx	792	900	xxxxxx	xxxxx	xxxxxx	xxxxxx	xxxxx	xxxxxx	xxxxxx

Peak Hour Delay Signal Warrant Report

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	68	0	64	0	0	0	0	917	51	32	1007	0
ApproachDel:	23.7			xxxxxxx			xxxxxxx			xxxxxxx		

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.9]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=132]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=2139]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	68	0	64	0	0	0	0	917	51	32	1007	0

Major Street Volume: 2007
Minor Approach Volume: 132
Minor Approach Volume Threshold: 45 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Existing PM

Intersection #4: Meadowview Road/Manorside Drive

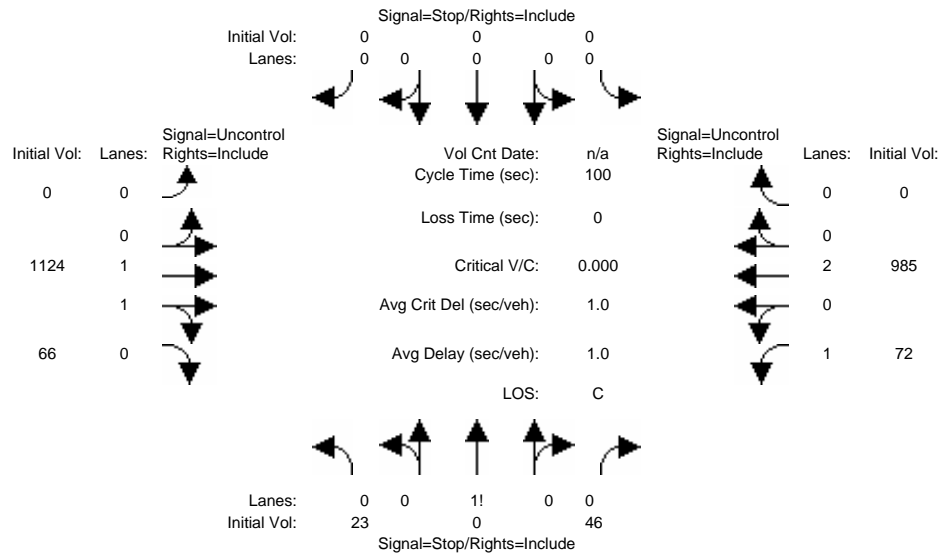


Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Vol.

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Critical Gap, FollowUpPim, Capacity Module, and Level of Service Module.

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Two-Stage Gap Acceptance - Stage One Module.

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Two-Stage Gap Acceptance - Stage Two Module.

Peak Hour Delay Signal Warrant Report

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach, Movement, Volume Module, and Level of Service Module. Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.4]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=69]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=2316]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	23	0	46	0	0	0	0	1124	66	72	985	0

Major Street Volume: 2247
Minor Approach Volume: 69
Minor Approach Volume Threshold: 6 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #5: Meadowview Road/24th Street

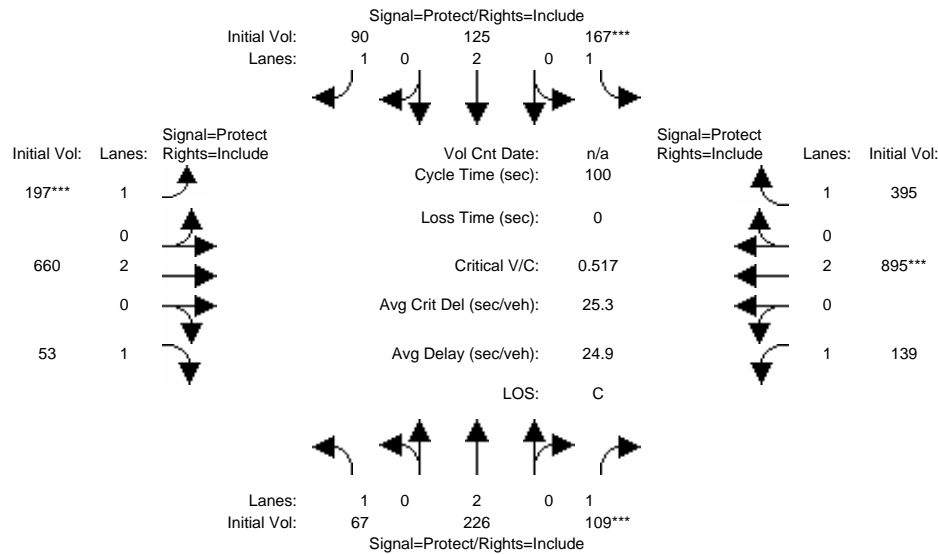


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.13	0.13	0.18	0.19	0.19	0.21	0.49	0.49	0.20	0.48	0.48
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.7	3.1	2.8	4.2	1.5	2.2	4.8	6.1	0.8	3.3	9.1	7.6
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.4	0.9	1.0	1.0	0.2	0.4	1.0	0.6	0.1	0.6	1.1	1.0
HCM2KQueue:	2.1	3.9	3.8	5.2	1.8	2.6	5.9	6.7	0.9	3.9	10.1	8.6
70thFactor:	1.19	1.19	1.19	1.19	1.20	1.19	1.19	1.18	1.20	1.19	1.18	1.18
70thHCM2kQ:	2.5	4.7	4.5	6.2	2.1	3.1	7.0	7.9	1.0	4.7	11.9	10.1
85thFactor:	1.58	1.56	1.56	1.55	1.58	1.58	1.55	1.54	1.59	1.56	1.51	1.53
85thHCM2kQ:	3.3	6.2	6.0	8.1	2.8	4.1	9.1	10.3	1.4	6.1	15.3	13.1
90thFactor:	1.76	1.73	1.73	1.71	1.77	1.75	1.70	1.69	1.78	1.73	1.64	1.66
90thHCM2kQ:	3.7	6.8	6.6	8.9	3.1	4.5	10.0	11.2	1.5	6.8	16.6	14.2
95thFactor:	2.03	1.98	1.99	1.95	2.04	2.02	1.93	1.91	2.07	1.98	1.84	1.87
95thHCM2kQ:	4.3	7.8	7.6	10.2	3.6	5.2	11.3	12.8	1.8	7.8	18.6	16.1
98thFactor:	2.55	2.44	2.45	2.37	2.57	2.52	2.34	2.30	2.64	2.44	2.16	2.22
98thHCM2kQ:	5.4	9.6	9.3	12.4	4.6	6.5	13.7	15.3	2.3	9.6	21.8	19.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	15.2	52.4	25.4	37.8	26.4	19.4	43.6	104	7.0	29.9	155	68.0

Name: year 1995 composite fleet
 Fuel Consumption: 92.480 pounds
 14.982 gallons
 Carbon Dioxide: 288.537 pounds
 Carbon Monoxide: 22.640 pounds
 Hydrocarbons: 4.132 pounds
 Nitrogen Oxides: 0.819 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 86.846 pounds
 14.069 gallons
 Carbon Dioxide: 270.959 pounds
 Carbon Monoxide: 22.200 pounds
 Hydrocarbons: 4.043 pounds
 Nitrogen Oxides: 0.732 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing PM

Intersection #5: Meadowview Road/24th Street

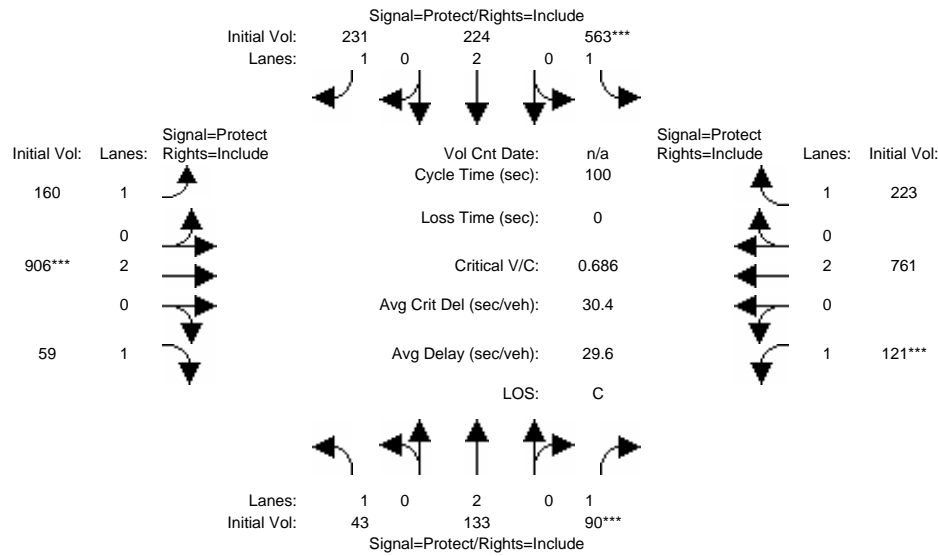


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.08	0.08	0.08	0.45	0.46	0.46	0.14	0.37	0.37	0.10	0.33	0.33
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.1	1.9	2.4	12.4	1.9	4.0	4.2	11.2	1.1	3.3	9.5	4.8
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.4	0.8	1.7	2.1	0.2	0.4	1.6	2.0	0.1	1.7	1.7	0.7
HCM2KQueue:	1.6	2.6	4.1	14.4	2.0	4.5	5.8	13.2	1.2	5.0	11.2	5.6
70th%Factor:	1.20	1.19	1.19	1.17	1.20	1.19	1.19	1.17	1.20	1.19	1.18	1.19
70th%HCM2kQ:	1.9	3.1	4.9	16.9	2.4	5.3	6.9	15.5	1.4	5.9	13.2	6.6
85th%Factor:	1.58	1.57	1.56	1.49	1.58	1.56	1.55	1.49	1.59	1.55	1.51	1.55
85th%HCM2kQ:	2.5	4.1	6.4	21.4	3.2	7.0	9.0	19.8	1.9	7.8	16.9	8.6
90th%Factor:	1.77	1.75	1.73	1.59	1.76	1.72	1.70	1.61	1.78	1.71	1.63	1.70
90th%HCM2kQ:	2.8	4.6	7.1	23.0	3.6	7.7	9.9	21.3	2.1	8.5	18.3	9.5
95th%Factor:	2.05	2.02	1.98	1.77	2.04	1.97	1.93	1.79	2.06	1.95	1.82	1.94
95th%HCM2kQ:	3.2	5.3	8.1	25.5	4.2	8.8	11.2	23.7	2.5	9.8	20.4	10.8
98th%Factor:	2.59	2.52	2.43	2.03	2.55	2.41	2.34	2.06	2.61	2.38	2.12	2.35
98th%HCM2kQ:	4.0	6.6	9.9	29.3	5.2	10.8	13.6	27.3	3.1	11.9	23.8	13.1

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	10.2	31.7	21.9	111.5	32.3	36.4	37.9	192	9.7	29.3	162	43.6

Name: year 1995 composite fleet
 Fuel Consumption: 116.158 pounds
 18.818 gallons
 Carbon Dioxide: 362.413 pounds
 Carbon Monoxide: 28.880 pounds
 Hydrocarbons: 5.404 pounds
 Nitrogen Oxides: 1.004 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 109.703 pounds
 17.772 gallons
 Carbon Dioxide: 342.275 pounds
 Carbon Monoxide: 28.373 pounds
 Hydrocarbons: 5.302 pounds
 Nitrogen Oxides: 0.902 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Existing AM

Intersection #6: Meadowview Road/Detroit Boulevard

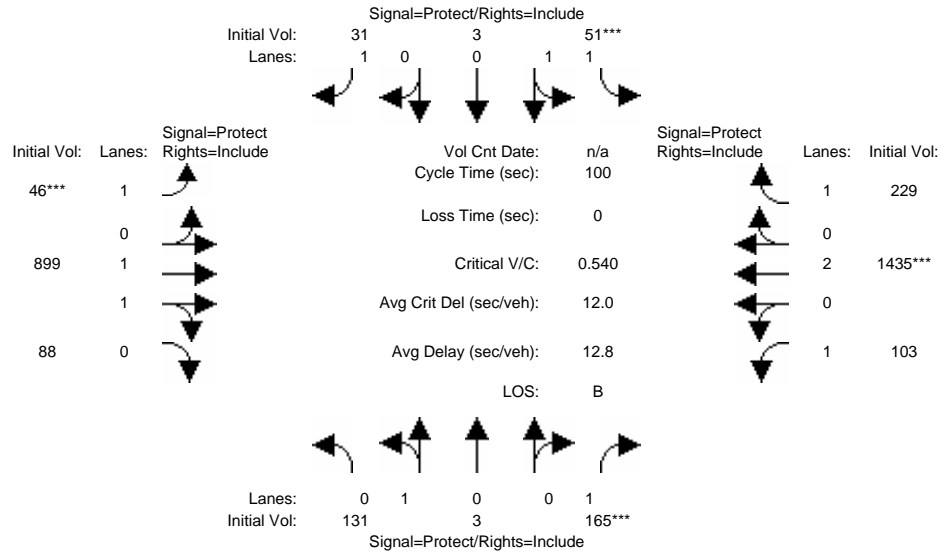


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.17	0.19	0.19	0.03	0.04	0.04	0.05	0.65	0.65	0.13	0.74	0.74
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.3	3.3	4.1	0.7	0.7	0.8	1.2	7.0	7.0	2.6	9.2	2.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.7	0.6	1.1	0.9	0.5	0.7	1.0	0.7	0.7	0.7	1.2	0.2
HCM2KQueue:	4.1	3.9	5.2	1.6	1.2	1.5	2.2	7.7	7.7	3.3	10.3	2.2
70thFactor:	1.19	1.19	1.19	1.20	1.20	1.20	1.19	1.18	1.18	1.19	1.18	1.19
70thHCM2kQ:	4.8	4.6	6.2	2.0	1.4	1.8	2.7	9.1	9.1	4.0	12.2	2.6
85thFactor:	1.56	1.56	1.55	1.58	1.59	1.59	1.58	1.53	1.53	1.57	1.51	1.58
85thHCM2kQ:	6.3	6.1	8.1	2.6	1.9	2.4	3.5	11.8	11.8	5.2	15.7	3.5
90thFactor:	1.73	1.73	1.71	1.77	1.78	1.77	1.76	1.67	1.67	1.74	1.64	1.76
90thHCM2kQ:	7.0	6.7	9.0	2.9	2.1	2.7	3.9	12.9	12.9	5.8	17.0	3.9
95thFactor:	1.98	1.98	1.95	2.05	2.06	2.05	2.03	1.89	1.89	2.00	1.84	2.03
95thHCM2kQ:	8.0	7.7	10.2	3.3	2.5	3.1	4.5	14.6	14.6	6.7	19.0	4.5
98thFactor:	2.43	2.44	2.37	2.58	2.61	2.59	2.54	2.25	2.25	2.47	2.15	2.54
98thHCM2kQ:	9.9	9.5	12.4	4.2	3.1	3.9	5.7	17.4	17.4	8.3	22.3	5.6

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	29.3	0.7	37.3	12.6	0.7	7.5	11.2	109	10.7	23.7	157	17.6

Name: year 1995 composite fleet
 Fuel Consumption: 65.414 pounds
 10.597 gallons
 Carbon Dioxide: 204.091 pounds
 Carbon Monoxide: 14.976 pounds
 Hydrocarbons: 2.433 pounds
 Nitrogen Oxides: 0.627 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 60.004 pounds
 9.721 gallons
 Carbon Dioxide: 187.211 pounds
 Carbon Monoxide: 14.561 pounds
 Hydrocarbons: 2.346 pounds
 Nitrogen Oxides: 0.552 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Existing PM

Intersection #6: Meadowview Road/Detroit Boulevard

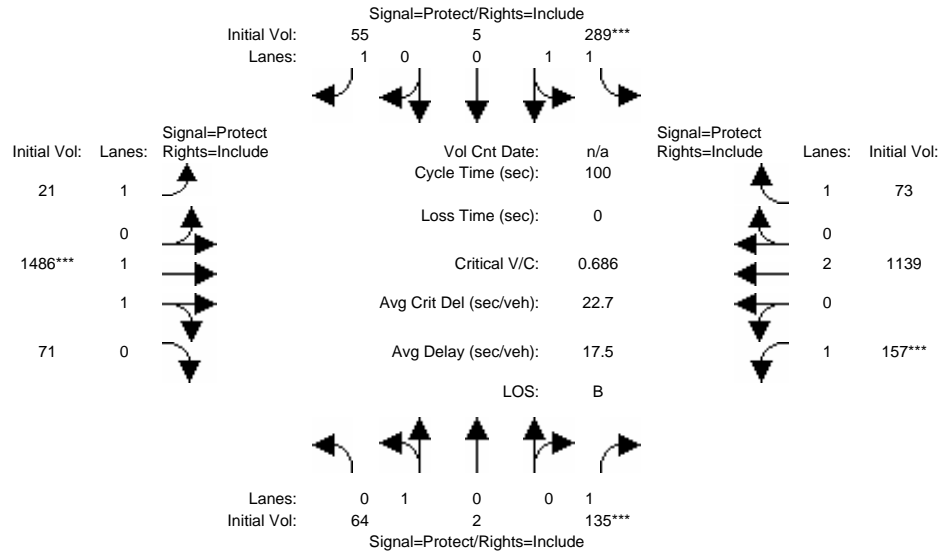


Table with columns for Approach, Movement, and four bound directions (North, South, East, West). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.07	0.12	0.12	0.12	0.17	0.17	0.03	0.63	0.63	0.13	0.73	0.73
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.8	1.7	3.6	3.9	3.7	1.3	0.6	14.8	14.8	4.2	6.5	0.6
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.9	0.4	1.8	1.8	0.9	0.3	0.6	2.1	2.1	1.8	0.8	0.1
HCM2KQueue:	2.6	2.1	5.4	5.7	4.6	1.6	1.2	16.9	16.9	6.0	7.3	0.6
70thFactor:	1.19	1.19	1.19	1.19	1.19	1.20	1.20	1.17	1.17	1.19	1.18	1.20
70thHCM2kQ:	3.1	2.5	6.4	6.8	5.5	1.9	1.5	19.7	19.7	7.1	8.6	0.8
85thFactor:	1.57	1.58	1.55	1.55	1.56	1.58	1.59	1.47	1.47	1.55	1.54	1.59
85thHCM2kQ:	4.1	3.3	8.3	8.9	7.2	2.5	1.9	24.8	24.8	9.3	11.1	1.0
90thFactor:	1.75	1.76	1.71	1.70	1.72	1.77	1.78	1.57	1.57	1.70	1.68	1.79
90thHCM2kQ:	4.6	3.7	9.2	9.7	7.9	2.8	2.2	26.5	26.5	10.2	12.2	1.1
95thFactor:	2.02	2.03	1.95	1.94	1.96	2.05	2.06	1.74	1.74	1.93	1.90	2.08
95thHCM2kQ:	5.3	4.2	10.4	11.1	9.1	3.2	2.5	29.3	29.3	11.6	13.8	1.3
98thFactor:	2.52	2.55	2.36	2.34	2.40	2.59	2.61	1.97	1.97	2.33	2.27	2.65
98thHCM2kQ:	6.6	5.3	12.7	13.4	11.1	4.1	3.2	33.3	33.3	14.0	16.5	1.7

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	15.4	0.5	32.3	69.3	1.1	11.9	5.2	241	11.5	37.5	111	5.1

Name: year 1995 composite fleet

Fuel Consumption: 84.239 pounds
13.647 gallons
Carbon Dioxide: 262.825 pounds
Carbon Monoxide: 19.924 pounds
Hydrocarbons: 3.431 pounds
Nitrogen Oxides: 0.781 pounds

Name: year 2000 composite fleet

Fuel Consumption: 78.140 pounds
12.659 gallons
Carbon Dioxide: 243.796 pounds
Carbon Monoxide: 19.452 pounds
Hydrocarbons: 3.334 pounds
Nitrogen Oxides: 0.692 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #7: Mack Road/Franklin Boulevard

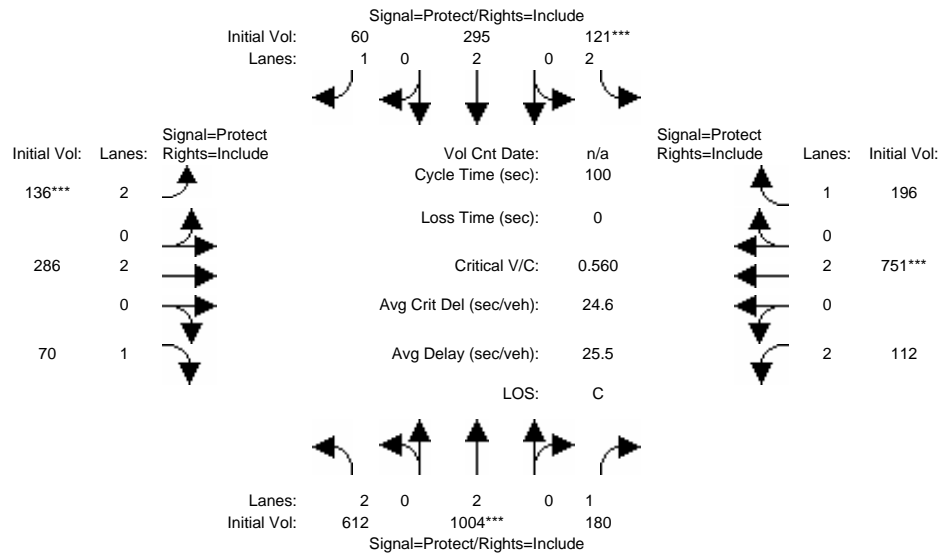


Table containing traffic engineering data including Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.38	0.50	0.50	0.06	0.18	0.18	0.07	0.31	0.31	0.13	0.37	0.37
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.6	10.2	2.8	1.7	3.9	1.4	1.9	3.1	1.4	1.4	8.7	3.9
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.8	1.2	0.3	1.1	0.8	0.3	1.1	0.3	0.2	0.3	1.2	0.5
HCM2KQueue:	7.4	11.5	3.1	2.8	4.7	1.7	3.0	3.4	1.6	1.8	9.9	4.4
70thFactor:	1.18	1.18	1.19	1.19	1.19	1.20	1.19	1.19	1.20	1.20	1.18	1.19
70thHCM2kQ:	8.8	13.5	3.7	3.3	5.6	2.0	3.6	4.1	1.9	2.1	11.7	5.2
85thFactor:	1.53	1.50	1.57	1.57	1.56	1.58	1.57	1.57	1.58	1.58	1.52	1.56
85thHCM2kQ:	11.4	17.3	4.9	4.3	7.3	2.7	4.7	5.4	2.5	2.8	15.1	6.8
90thFactor:	1.68	1.63	1.74	1.75	1.72	1.77	1.74	1.74	1.77	1.77	1.64	1.72
90thHCM2kQ:	12.4	18.6	5.4	4.8	8.0	3.0	5.2	6.0	2.7	3.1	16.3	7.5
95thFactor:	1.90	1.82	2.00	2.01	1.96	2.05	2.01	2.00	2.05	2.04	1.85	1.97
95thHCM2kQ:	14.1	20.8	6.3	5.6	9.2	3.4	6.0	6.9	3.2	3.6	18.3	8.6
98thFactor:	2.27	2.11	2.49	2.51	2.40	2.58	2.50	2.47	2.59	2.57	2.17	2.41
98thHCM2kQ:	16.8	24.2	7.8	6.9	11.2	4.3	7.4	8.5	4.0	4.6	21.5	10.6

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	114.8	175	25.5	29.4	66.0	12.8	32.9	53.2	12.5	25.3	149	35.0

Name: year 1995 composite fleet
 Fuel Consumption: 115.064 pounds
 18.640 gallons
 Carbon Dioxide: 359.000 pounds
 Carbon Monoxide: 28.233 pounds
 Hydrocarbons: 5.169 pounds
 Nitrogen Oxides: 1.018 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 108.136 pounds
 17.518 gallons
 Carbon Dioxide: 337.386 pounds
 Carbon Monoxide: 27.690 pounds
 Hydrocarbons: 5.059 pounds
 Nitrogen Oxides: 0.911 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing PM

Intersection #7: Mack Road/Franklin Boulevard

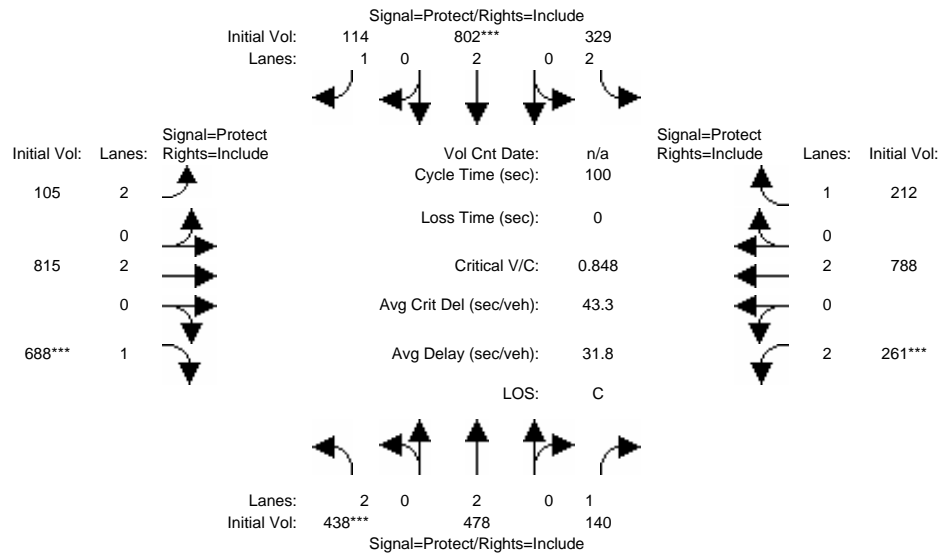


Table with columns for Approach, Movement, and four bound directions (North, South, East, West). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.15	0.24	0.24	0.17	0.26	0.26	0.07	0.50	0.50	0.09	0.52	0.52
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.1	6.1	3.2	4.3	11.1	2.5	1.4	7.7	16.6	3.7	7.1	3.3
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	3.4	1.2	0.6	1.2	3.9	0.4	0.7	0.8	4.3	2.9	0.7	0.3
HCM2KQueue:	9.5	7.3	3.8	5.5	15.1	2.9	2.1	8.5	20.9	6.5	7.8	3.6
70thFactor:	1.18	1.18	1.19	1.19	1.17	1.19	1.19	1.18	1.16	1.18	1.18	1.19
70thHCM2kQ:	11.2	8.6	4.5	6.5	17.6	3.4	2.5	10.0	24.2	7.8	9.2	4.3
85thFactor:	1.52	1.54	1.56	1.55	1.48	1.57	1.58	1.53	1.45	1.54	1.53	1.57
85thHCM2kQ:	14.4	11.2	5.9	8.5	22.3	4.5	3.3	12.9	30.3	10.1	12.0	5.6
90thFactor:	1.65	1.68	1.73	1.70	1.59	1.75	1.76	1.66	1.54	1.69	1.67	1.73
90thHCM2kQ:	15.6	12.3	6.6	9.3	23.9	5.0	3.7	14.1	32.2	11.0	13.0	6.2
95thFactor:	1.85	1.90	1.99	1.94	1.76	2.01	2.03	1.87	1.69	1.92	1.89	1.99
95thHCM2kQ:	17.6	13.9	7.5	10.6	26.5	5.8	4.3	15.9	35.3	12.5	14.7	7.2
98thFactor:	2.18	2.27	2.45	2.36	2.01	2.50	2.55	2.22	1.90	2.30	2.25	2.46
98thHCM2kQ:	20.7	16.6	9.3	12.9	30.3	7.2	5.4	18.8	39.7	15.1	17.6	8.8

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	106.7	105	29.1	75.3	190	22.6	25.1	131	149.1	64.3	121	29.3

Name: year 1995 composite fleet
 Fuel Consumption: 178.729 pounds
 28.954 gallons
 Carbon Dioxide: 557.633 pounds
 Carbon Monoxide: 44.724 pounds
 Hydrocarbons: 8.470 pounds
 Nitrogen Oxides: 1.511 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 169.248 pounds
 27.418 gallons
 Carbon Dioxide: 528.053 pounds
 Carbon Monoxide: 43.979 pounds
 Hydrocarbons: 8.320 pounds
 Nitrogen Oxides: 1.361 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #8: Cosumnes River Boulevard/Franklin Boulevard

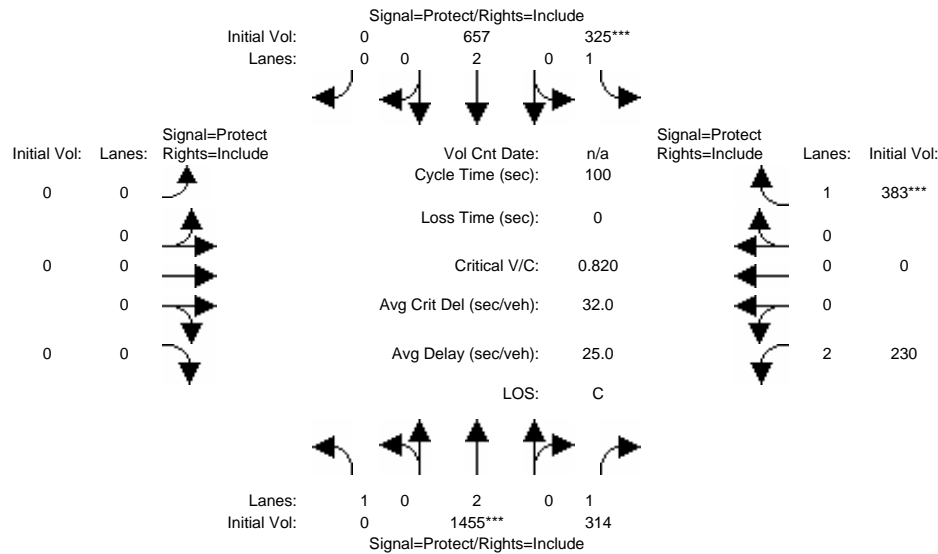


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.49	0.49	0.22	0.71	0.00	0.00	0.00	0.00	0.29	0.00	0.29
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	18.1	5.5	8.6	3.4	0.0	0.0	0.0	0.0	2.5	0.0	9.9
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	1.00	1.00	1.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Q2:	0.0	3.9	0.6	3.3	0.3	0.0	0.0	0.0	0.0	0.3	0.0	3.4
HCM2KQueue:	0.0	22.0	6.2	11.9	3.7	0.0	0.0	0.0	0.0	2.8	0.0	13.4
70thFactor:	1.20	1.16	1.19	1.17	1.19	1.20	1.20	1.20	1.20	1.19	1.20	1.17
70thHCM2kQ:	0.0	25.4	7.3	14.0	4.5	0.0	0.0	0.0	0.0	3.3	0.0	15.6
85thFactor:	1.60	1.44	1.54	1.50	1.56	1.60	1.60	1.60	1.60	1.57	1.60	1.49
85thHCM2kQ:	0.0	31.7	9.5	17.9	5.8	0.0	0.0	0.0	0.0	4.4	0.0	19.9
90thFactor:	1.80	1.53	1.69	1.62	1.73	1.80	1.80	1.80	1.80	1.75	1.80	1.61
90thHCM2kQ:	0.0	33.7	10.4	19.3	6.5	0.0	0.0	0.0	0.0	4.9	0.0	21.4
95thFactor:	2.10	1.68	1.93	1.81	1.99	2.10	2.10	2.10	2.10	2.01	2.10	1.79
95thHCM2kQ:	0.0	36.9	11.9	21.6	7.4	0.0	0.0	0.0	0.0	5.6	0.0	23.8
98thFactor:	2.70	1.88	2.32	2.10	2.45	2.70	2.70	2.70	2.70	2.51	2.70	2.06
98thHCM2kQ:	0.0	41.4	14.3	25.0	9.2	0.0	0.0	0.0	0.0	7.0	0.0	27.5

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	31.0	49.6	77.3	58.1	0.0	0.0	0.0	0.0	43.7	0.0	89.2

Name: year 1995 composite fleet
 Fuel Consumption: 99.787 pounds
 16.165 gallons
 Carbon Dioxide: 311.335 pounds
 Carbon Monoxide: 24.437 pounds
 Hydrocarbons: 4.463 pounds
 Nitrogen Oxides: 0.883 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 93.720 pounds
 15.183 gallons
 Carbon Dioxide: 292.407 pounds
 Carbon Monoxide: 23.962 pounds
 Hydrocarbons: 4.366 pounds
 Nitrogen Oxides: 0.789 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.32	0.32	0.36	0.68	0.00	0.00	0.00	0.00	0.32	0.00	0.32
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	8.5	6.4	8.7	8.3	0.0	0.0	0.0	0.0	5.2	0.0	7.2
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	1.00	1.00	1.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Q2:	0.0	1.4	1.1	1.4	0.9	0.0	0.0	0.0	0.0	0.7	0.0	1.4
HCM2KQueue:	0.0	9.9	7.5	10.1	9.2	0.0	0.0	0.0	0.0	5.9	0.0	8.6
70thFactor:	1.20	1.18	1.18	1.18	1.18	1.20	1.20	1.20	1.20	1.19	1.20	1.18
70thHCM2kQ:	0.0	11.7	8.9	11.9	10.9	0.0	0.0	0.0	0.0	7.1	0.0	10.1
85thFactor:	1.60	1.52	1.53	1.51	1.52	1.60	1.60	1.60	1.60	1.55	1.60	1.53
85thHCM2kQ:	0.0	15.1	11.5	15.3	14.0	0.0	0.0	0.0	0.0	9.2	0.0	13.1
90thFactor:	1.80	1.64	1.67	1.64	1.65	1.80	1.80	1.80	1.80	1.70	1.80	1.66
90thHCM2kQ:	0.0	16.3	12.6	16.6	15.2	0.0	0.0	0.0	0.0	10.1	0.0	14.3
95thFactor:	2.10	1.85	1.90	1.84	1.86	2.10	2.10	2.10	2.10	1.93	2.10	1.87
95thHCM2kQ:	0.0	18.3	14.2	18.6	17.1	0.0	0.0	0.0	0.0	11.5	0.0	16.1
98thFactor:	2.70	2.17	2.26	2.16	2.19	2.70	2.70	2.70	2.70	2.33	2.70	2.22
98thHCM2kQ:	0.0	21.5	17.0	21.8	20.2	0.0	0.0	0.0	0.0	13.9	0.0	19.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	146	57.5	78.2	142	0.0	0.0	0.0	0.0	91.6	0.0	64.7

Name: year 1995 composite fleet
 Fuel Consumption: 89.265 pounds
 14.461 gallons
 Carbon Dioxide: 278.508 pounds
 Carbon Monoxide: 21.516 pounds
 Hydrocarbons: 3.824 pounds
 Nitrogen Oxides: 0.812 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 83.349 pounds
 13.503 gallons
 Carbon Dioxide: 260.048 pounds
 Carbon Monoxide: 21.055 pounds
 Hydrocarbons: 3.730 pounds
 Nitrogen Oxides: 0.722 pounds

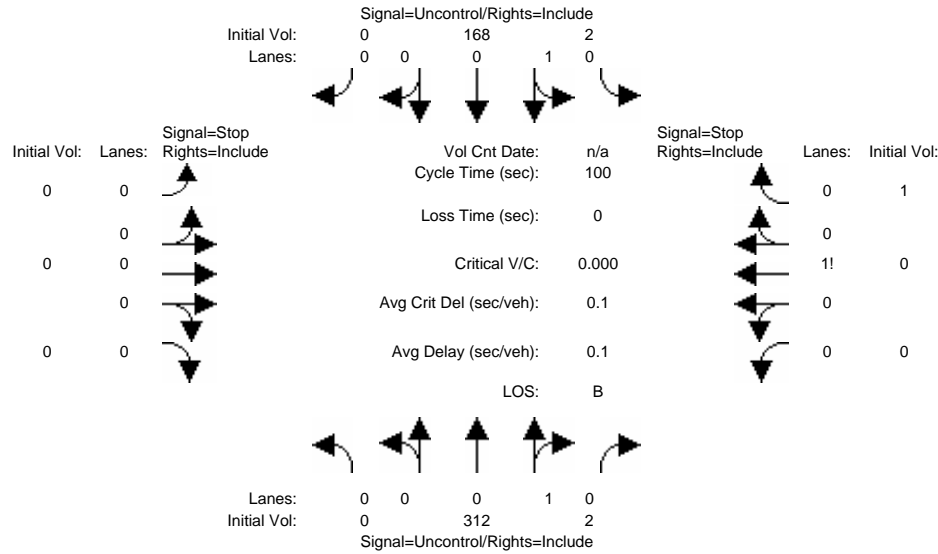
DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Existing AM

Intersection #9: Stonecrest Avenue/Freeport Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Volume Module:													
Base Vol:	0	312	2	2	168	0	0	0	0	0	0	1	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	312	2	2	168	0	0	0	0	0	0	1	
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
PHF Volume:	0	339	2	2	183	0	0	0	0	0	0	1	
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	
Final Vol.:	0	339	2	2	183	0	0	0	0	0	0	1	
Critical Gap Module:													
Critical Gap:	xxxxx xxxxx xxxxx			4.1	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx	6.2	xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx			6.2	xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx		
FollowUpTim:	xxxxx xxxxx xxxxx			2.2	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	3.3	xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx			3.3	xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx		
Capacity Module:													
Conflict Vol:	xxxx xxxxx xxxxx xxxxx	341	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	340	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	702	
Potent Cap.:	xxxx xxxxx xxxxx xxxxx	1218	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	702	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	702	
Move Cap.:	xxxx xxxxx xxxxx xxxxx	1218	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	702	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	702	
Volume/Cap:	xxxx xxxxx xxxxx xxxxx	0.00	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	0.00	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	0.00	
Level of Service Module:													
2Way95thQ:	xxxx xxxxx xxxxx xxxxx	0.0	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	0.0	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	0.0	
Control Del:	xxxxxx xxxxx xxxxx xxxxx	8.0	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	10.1	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	10.1	
LOS by Move:	*	*	*	*	A	*	*	*	*	*	*	B	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx
SharedQueue:	xxxxxx xxxxx xxxxx xxxxx	0.0	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd ConDel:	xxxxxx xxxxx xxxxx xxxxx	8.0	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx	xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	10.1
ApproachLOS:													B
HevVeh:	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Grade:	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peds/Hour:	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Walk Speed:	4.00 feet/sec												
LaneWidth:	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet	12 feet
Time Period:	0.25 hour												

Peak Hour Delay Signal Warrant Report

Intersection #9 Stonecrest Avenue/Freeport Boulevard

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign			
Lanes:	0	0	1	0	0	1	0	0	0	0	0	0	1
Initial Vol:	0	312	2	2	168	0	0	0	0	0	0	0	1
ApproachDel:	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	xxxxxxx	10.1

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.0]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=1]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=485]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #9 Stonecrest Avenue/Freeport Boulevard

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	0 0 0 1 0	0 1 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 312 2	2 168 0	0 0 0 0	0 0 0 1
Major Street Volume:	484			
Minor Approach Volume:	1			
Minor Approach Volume Threshold:	413			

SIGNAL WARRANT DISCLAIMER

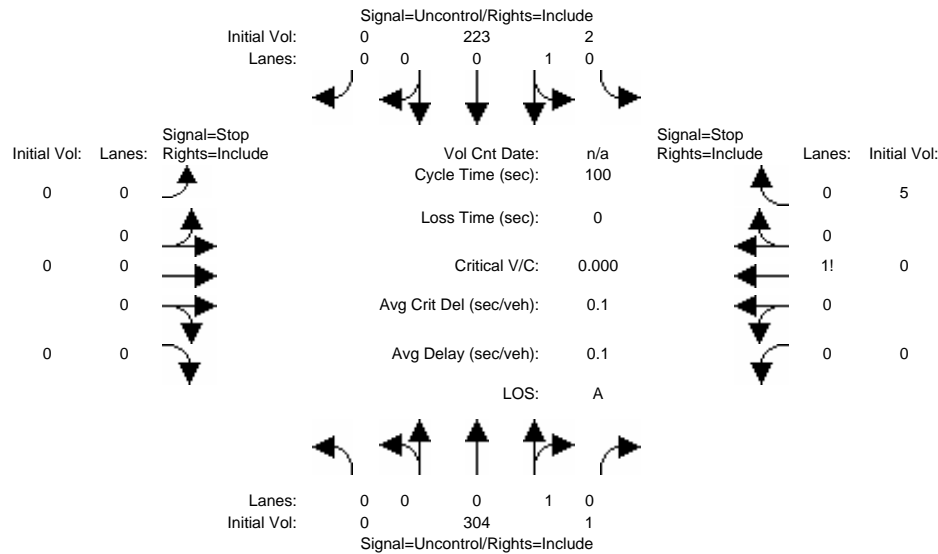
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Existing PM

Intersection #9: Stonecrest Avenue/Freeport Boulevard



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 304 1	2 223 0	0 0 0	0 0 5
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	0 304 1	2 223 0	0 0 0	0 0 5
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Volume:	0 304 1	2 223 0	0 0 0	0 0 5
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 304 1	2 223 0	0 0 0	0 0 5
Critical Gap Module:				
Critical Gap:	4.1	4.1	4.1	6.2
FollowUpTim:	2.2	2.2	2.2	3.3
Capacity Module:				
Cnflict Vol:	305	305	305	305
Potent Cap.:	1256	1256	1256	735
Move Cap.:	1256	1256	1256	735
Volume/Cap.:	0.00	0.00	0.00	0.01
Level of Service Module:				
2Way95thQ:	0.0	0.0	0.0	0.0
Control Del:	7.9	7.9	7.9	9.9
LOS by Move:	A	A	A	A
Shared Cap.:	0.0	0.0	0.0	0.0
Shrd ConDel:	7.9	7.9	7.9	9.9
Shared LOS:	A	A	A	A
ApproachDel:	9.9	9.9	9.9	9.9
ApproachLOS:	A	A	A	A
HevVeh:	2%	2%	0%	2%
Grade:	0%	0%	0%	0%
Peds/Hour:	0	0	0	0
Pedestrian Walk Speed:	4.00 feet/sec	4.00 feet/sec	4.00 feet/sec	4.00 feet/sec
LaneWidth:	12 feet	12 feet	12 feet	12 feet
Time Period:	0.25 hour	0.25 hour	0.25 hour	0.25 hour

Peak Hour Delay Signal Warrant Report

Intersection #9 Stonecrest Avenue/Freeport Boulevard

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	0 0 0 1 0	0 1 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 304 1	2 223 0	0 0 0 0	0 0 0 5
ApproachDel:	9.9	9.9	9.9	9.9

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.0]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=5]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=535]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #9 Stonecrest Avenue/Freeport Boulevard

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Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:      0 0 0 1 0      0 1 0 0 0      0 0 0 0 0      0 0 0 0 1
Initial Vol: 0 304 1      2 223 0      0 0 0 0      0 0 0 5
-----|-----|-----|-----|-----|
Major Street Volume:          530
Minor Approach Volume:        5
Minor Approach Volume Threshold: 389
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #32:

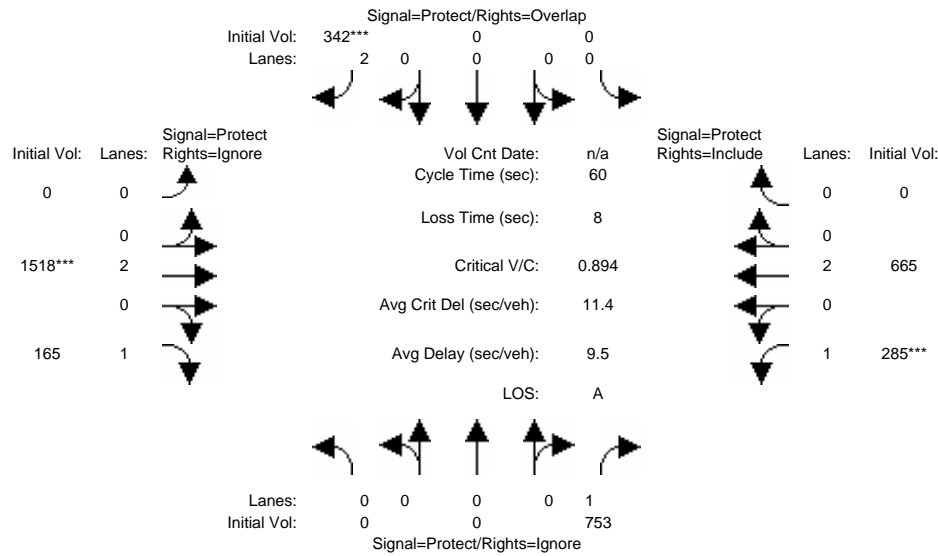


Table containing traffic engineering data: Approach: North Bound, South Bound, East Bound, West Bound; Movement: L-T-R; Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, etc.; Saturation Flow Module; Capacity Analysis Module; HCM Ops Adjusted Lane Utilization Module; HCM Ops Input Saturation Adj Module; HCM Ops f(lt) Adj Case Module; HCM Ops Saturation Adj Module; Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	3.4	0.0	6.5	0.0	4.8	2.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	2.7	0.0	1.6	0.0	2.3	0.4	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	6.0	0.0	8.1	0.0	7.1	2.8	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.19	1.20	1.18	1.20	1.18	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	7.2	0.0	9.5	0.0	8.4	3.3	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.55	1.60	1.53	1.60	1.54	1.57	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	9.3	0.0	12.3	0.0	10.9	4.4	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.70	1.80	1.67	1.80	1.68	1.75	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	10.2	0.0	13.4	0.0	11.9	4.9	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.93	2.10	1.88	2.10	1.90	2.01	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	11.6	0.0	15.2	0.0	13.5	5.6	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.33	2.70	2.24	2.70	2.28	2.51	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	14.1	0.0	18.0	0.0	16.1	7.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	89.0	0.0	186	0.0	71.4	68.1	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 56.051 pounds
 9.080 gallons
 Carbon Dioxide: 174.878 pounds
 Carbon Monoxide: 12.451 pounds
 Hydrocarbons: 1.884 pounds
 Nitrogen Oxides: 0.575 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 50.835 pounds
 8.235 gallons
 Carbon Dioxide: 158.606 pounds
 Carbon Monoxide: 12.050 pounds
 Hydrocarbons: 1.801 pounds
 Nitrogen Oxides: 0.501 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing PM

Intersection #32:

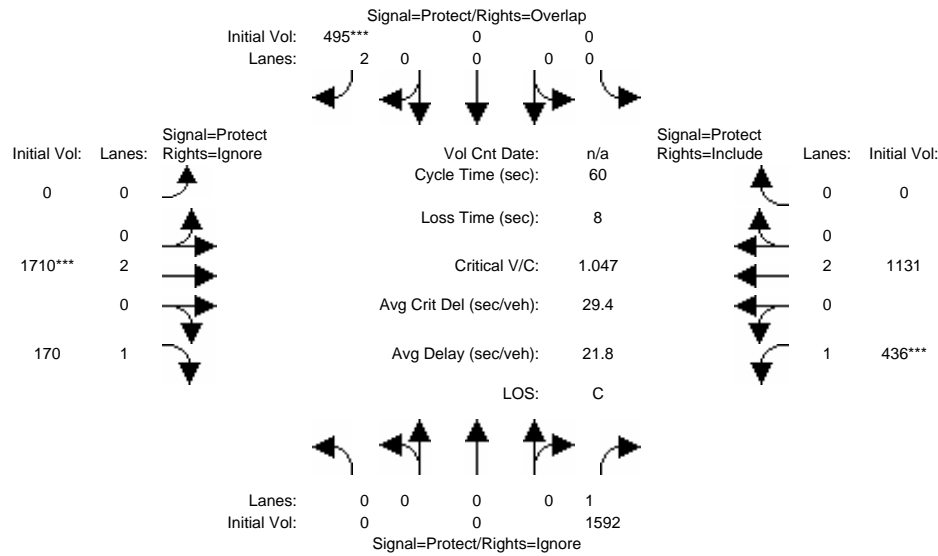


Table with columns for Approach, Movement, and four Bound directions (North, South, East, West). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	4.7	0.0	7.0	0.0	7.3	4.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	6.7	0.0	1.7	0.0	8.1	0.8	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	11.4	0.0	8.7	0.0	15.4	5.2	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.18	1.20	1.18	1.20	1.17	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	13.4	0.0	10.2	0.0	18.0	6.2	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.50	1.60	1.52	1.60	1.48	1.55	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	17.2	0.0	13.2	0.0	22.8	8.1	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.63	1.80	1.66	1.80	1.59	1.71	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	18.6	0.0	14.4	0.0	24.4	8.9	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.82	2.10	1.87	2.10	1.76	1.95	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	20.8	0.0	16.2	0.0	27.0	10.2	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.12	2.70	2.21	2.70	2.01	2.37	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	24.2	0.0	19.2	0.0	30.9	12.3	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	124.9	0.0	199	0.0	109.9	125	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 101.725 pounds
 16.480 gallons
 Carbon Dioxide: 317.383 pounds
 Carbon Monoxide: 24.599 pounds
 Hydrocarbons: 4.432 pounds
 Nitrogen Oxides: 0.884 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 95.197 pounds
 15.422 gallons
 Carbon Dioxide: 297.014 pounds
 Carbon Monoxide: 24.094 pounds
 Hydrocarbons: 4.328 pounds
 Nitrogen Oxides: 0.790 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Existing AM

Intersection #33:

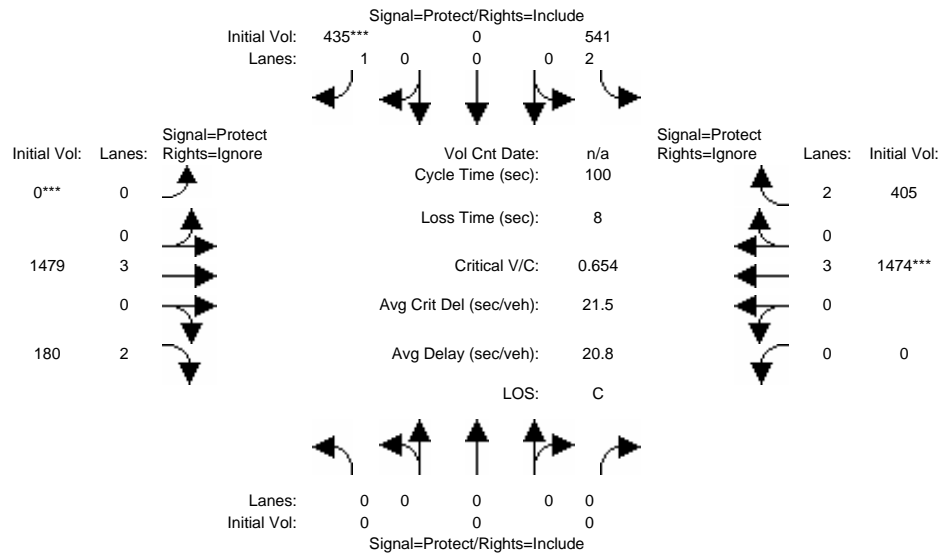


Table with columns for Approach (North, South, East, West) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.45	0.00	0.45	0.00	0.47	0.00	0.00	0.47	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	5.6	0.0	10.3	0.0	12.5	0.0	0.0	12.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.6	0.0	1.8	0.0	1.8	0.0	0.0	1.8	0.0
HCM2KQueue:	0.0	0.0	0.0	6.2	0.0	12.0	0.0	14.3	0.0	0.0	14.3	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.17	1.20	1.17	1.20	1.20	1.17	1.20
70thHCM2kQ:	0.0	0.0	0.0	7.3	0.0	14.1	0.0	16.8	0.0	0.0	16.7	0.0
85thFactor:	1.60	1.60	1.60	1.54	1.60	1.50	1.60	1.49	1.60	1.60	1.49	1.60
85thHCM2kQ:	0.0	0.0	0.0	9.5	0.0	18.1	0.0	21.3	0.0	0.0	21.2	0.0
90thFactor:	1.80	1.80	1.80	1.69	1.80	1.62	1.80	1.60	1.80	1.80	1.60	1.80
90thHCM2kQ:	0.0	0.0	0.0	10.5	0.0	19.5	0.0	22.9	0.0	0.0	22.7	0.0
95thFactor:	2.10	2.10	2.10	1.93	2.10	1.81	2.10	1.77	2.10	2.10	1.77	2.10
95thHCM2kQ:	0.0	0.0	0.0	11.9	0.0	21.8	0.0	25.4	0.0	0.0	25.3	0.0
98thFactor:	2.70	2.70	2.70	2.32	2.70	2.10	2.70	2.03	2.70	2.70	2.03	2.70
98thHCM2kQ:	0.0	0.0	0.0	14.4	0.0	25.2	0.0	29.1	0.0	0.0	29.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	97.6	0.0	92.3	0.0	307	0.0	0.0	306	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 114.496 pounds
 18.548 gallons
 Carbon Dioxide: 357.228 pounds
 Carbon Monoxide: 27.551 pounds
 Hydrocarbons: 4.861 pounds
 Nitrogen Oxides: 1.064 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 106.783 pounds
 17.299 gallons
 Carbon Dioxide: 333.163 pounds
 Carbon Monoxide: 26.948 pounds
 Hydrocarbons: 4.739 pounds
 Nitrogen Oxides: 0.945 pounds

DISCLAIMER
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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.53	0.00	0.53	0.00	0.39	0.00	0.00	0.39	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	11.1	0.0	10.5	0.0	10.6	0.0	0.0	10.2	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.5	0.0	1.6	0.0	1.6	0.0	0.0	1.5	0.0
HCM2KQueue:	0.0	0.0	0.0	12.6	0.0	12.2	0.0	12.2	0.0	0.0	11.8	0.0
70thFactor:	1.20	1.20	1.20	1.17	1.20	1.17	1.20	1.17	1.20	1.20	1.17	1.20
70thHCM2kQ:	0.0	0.0	0.0	14.8	0.0	14.3	0.0	14.3	0.0	0.0	13.8	0.0
85thFactor:	1.60	1.60	1.60	1.50	1.60	1.50	1.60	1.50	1.60	1.60	1.50	1.60
85thHCM2kQ:	0.0	0.0	0.0	18.8	0.0	18.2	0.0	18.3	0.0	0.0	17.7	0.0
90thFactor:	1.80	1.80	1.80	1.61	1.80	1.62	1.80	1.62	1.80	1.80	1.62	1.80
90thHCM2kQ:	0.0	0.0	0.0	20.3	0.0	19.7	0.0	19.7	0.0	0.0	19.1	0.0
95thFactor:	2.10	2.10	2.10	1.80	2.10	1.81	2.10	1.80	2.10	2.10	1.81	2.10
95thHCM2kQ:	0.0	0.0	0.0	22.6	0.0	22.0	0.0	22.0	0.0	0.0	21.3	0.0
98thFactor:	2.70	2.70	2.70	2.08	2.70	2.09	2.70	2.09	2.70	2.70	2.10	2.70
98thHCM2kQ:	0.0	0.0	0.0	26.2	0.0	25.5	0.0	25.5	0.0	0.0	24.8	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	193.8	0.0	94.8	0.0	259	0.0	0.0	252	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 116.032 pounds
 18.797 gallons
 Carbon Dioxide: 362.021 pounds
 Carbon Monoxide: 28.071 pounds
 Hydrocarbons: 5.002 pounds
 Nitrogen Oxides: 1.067 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 108.436 pounds
 17.567 gallons
 Carbon Dioxide: 338.320 pounds
 Carbon Monoxide: 27.476 pounds
 Hydrocarbons: 4.881 pounds
 Nitrogen Oxides: 0.949 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.21	0.00	0.21	0.00	0.00	0.00	0.00	0.71	0.71	0.00	0.71	0.71
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	5.1	0.0	5.8	0.0	0.0	0.0	0.0	7.3	6.9	0.0	7.3	11.7
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00
Q2:	1.1	0.0	1.8	0.0	0.0	0.0	0.0	0.8	1.0	0.0	0.8	2.0
HCM2KQueue:	6.3	0.0	7.6	0.0	0.0	0.0	0.0	8.1	7.9	0.0	8.1	13.7
70thFactor:	1.19	1.20	1.18	1.20	1.20	1.20	1.20	1.18	1.18	1.20	1.18	1.17
70thHCM2kQ:	7.5	0.0	9.0	0.0	0.0	0.0	0.0	9.6	9.3	0.0	9.6	16.0
85thFactor:	1.54	1.60	1.53	1.60	1.60	1.60	1.60	1.53	1.53	1.60	1.53	1.49
85thHCM2kQ:	9.7	0.0	11.7	0.0	0.0	0.0	0.0	12.4	12.1	0.0	12.4	20.4
90thFactor:	1.69	1.80	1.67	1.80	1.80	1.80	1.80	1.67	1.67	1.80	1.67	1.60
90thHCM2kQ:	10.6	0.0	12.8	0.0	0.0	0.0	0.0	13.5	13.2	0.0	13.5	21.9
95thFactor:	1.92	2.10	1.89	2.10	2.10	2.10	2.10	1.88	1.89	2.10	1.88	1.78
95thHCM2kQ:	12.1	0.0	14.5	0.0	0.0	0.0	0.0	15.3	14.9	0.0	15.2	24.3
98thFactor:	2.32	2.70	2.26	2.70	2.70	2.70	2.70	2.24	2.24	2.70	2.24	2.05
98thHCM2kQ:	14.6	0.0	17.2	0.0	0.0	0.0	0.0	18.2	17.7	0.0	18.1	28.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	89.7	0.0	92.3	0.0	0.0	0.0	0.0	179	62.5	0.0	178	185.3

Name: year 1995 composite fleet
 Fuel Consumption: 116.624 pounds
 18.893 gallons
 Carbon Dioxide: 363.868 pounds
 Carbon Monoxide: 26.371 pounds
 Hydrocarbons: 4.167 pounds
 Nitrogen Oxides: 1.148 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 106.484 pounds
 17.250 gallons
 Carbon Dioxide: 332.230 pounds
 Carbon Monoxide: 25.592 pounds
 Hydrocarbons: 4.005 pounds
 Nitrogen Oxides: 1.006 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.20	0.00	0.20	0.00	0.00	0.00	0.00	0.72	0.72	0.00	0.72	0.72
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	5.0	0.0	6.5	0.0	0.0	0.0	0.0	9.2	15.2	0.0	9.3	3.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00
Q2:	1.2	0.0	2.6	0.0	0.0	0.0	0.0	1.1	3.0	0.0	1.2	0.4
HCM2KQueue:	6.1	0.0	9.1	0.0	0.0	0.0	0.0	10.4	18.2	0.0	10.5	3.4
70thFactor:	1.19	1.20	1.18	1.20	1.20	1.20	1.20	1.18	1.16	1.20	1.18	1.19
70thHCM2kQ:	7.3	0.0	10.7	0.0	0.0	0.0	0.0	12.2	21.2	0.0	12.3	4.1
85thFactor:	1.54	1.60	1.52	1.60	1.60	1.60	1.60	1.51	1.46	1.60	1.51	1.57
85thHCM2kQ:	9.4	0.0	13.8	0.0	0.0	0.0	0.0	15.7	26.7	0.0	15.8	5.3
90thFactor:	1.69	1.80	1.65	1.80	1.80	1.80	1.80	1.64	1.56	1.80	1.64	1.74
90thHCM2kQ:	10.4	0.0	15.0	0.0	0.0	0.0	0.0	17.0	28.5	0.0	17.1	5.9
95thFactor:	1.93	2.10	1.86	2.10	2.10	2.10	2.10	1.84	1.72	2.10	1.84	2.00
95thHCM2kQ:	11.8	0.0	16.9	0.0	0.0	0.0	0.0	19.0	31.3	0.0	19.2	6.8
98thFactor:	2.32	2.70	2.20	2.70	2.70	2.70	2.70	2.15	1.95	2.70	2.15	2.47
98thHCM2kQ:	14.2	0.0	19.9	0.0	0.0	0.0	0.0	22.3	35.5	0.0	22.5	8.4

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	86.5	0.0	102.8	0.0	0.0	0.0	0.0	226	137.0	0.0	229	48.0

Name: year 1995 composite fleet
 Fuel Consumption: 123.048 pounds
 19.934 gallons
 Carbon Dioxide: 383.910 pounds
 Carbon Monoxide: 27.956 pounds
 Hydrocarbons: 4.459 pounds
 Nitrogen Oxides: 1.205 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 112.531 pounds
 18.230 gallons
 Carbon Dioxide: 351.097 pounds
 Carbon Monoxide: 27.147 pounds
 Hydrocarbons: 4.291 pounds
 Nitrogen Oxides: 1.057 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #1: Pocket Road/I-5 SB Ramps

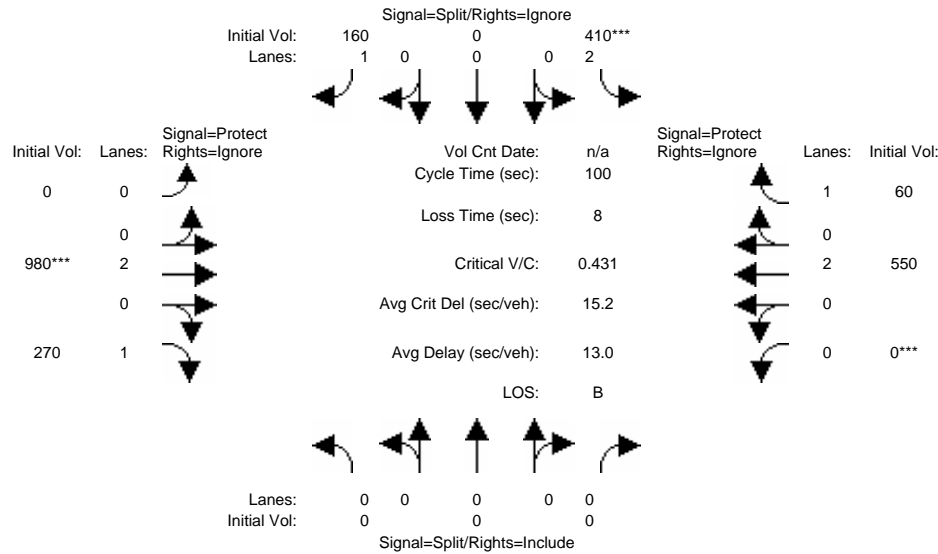


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.28	0.00	0.00	0.00	0.64	0.00	0.00	0.64	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	4.8	0.0	0.0	0.0	7.1	0.0	0.0	3.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.7	0.0	0.0	0.0	0.8	0.0	0.0	0.3	0.0
HCM2KQueue:	0.0	0.0	0.0	5.6	0.0	0.0	0.0	7.8	0.0	0.0	3.7	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.20	1.20	1.18	1.20	1.20	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	6.6	0.0	0.0	0.0	9.3	0.0	0.0	4.4	0.0
85thFactor:	1.60	1.60	1.60	1.55	1.60	1.60	1.60	1.53	1.60	1.60	1.57	1.60
85thHCM2kQ:	0.0	0.0	0.0	8.6	0.0	0.0	0.0	12.0	0.0	0.0	5.8	0.0
90thFactor:	1.80	1.80	1.80	1.70	1.80	1.80	1.80	1.67	1.80	1.80	1.73	1.80
90thHCM2kQ:	0.0	0.0	0.0	9.5	0.0	0.0	0.0	13.1	0.0	0.0	6.4	0.0
95thFactor:	2.10	2.10	2.10	1.94	2.10	2.10	2.10	1.89	2.10	2.10	1.99	2.10
95thHCM2kQ:	0.0	0.0	0.0	10.8	0.0	0.0	0.0	14.8	0.0	0.0	7.4	0.0
98thFactor:	2.70	2.70	2.70	2.35	2.70	2.70	2.70	2.25	2.70	2.70	2.45	2.70
98thHCM2kQ:	0.0	0.0	0.0	13.1	0.0	0.0	0.0	17.6	0.0	0.0	9.1	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/T-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	84.1	0.0	0.0	0.0	121	0.0	0.0	58.2	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 40.304 pounds
 6.529 gallons
 Carbon Dioxide: 125.748 pounds
 Carbon Monoxide: 9.247 pounds
 Hydrocarbons: 1.506 pounds
 Nitrogen Oxides: 0.388 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 36.991 pounds
 5.992 gallons
 Carbon Dioxide: 115.410 pounds
 Carbon Monoxide: 8.992 pounds
 Hydrocarbons: 1.453 pounds
 Nitrogen Oxides: 0.341 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #1: Pocket Road/I-5 SB Ramps

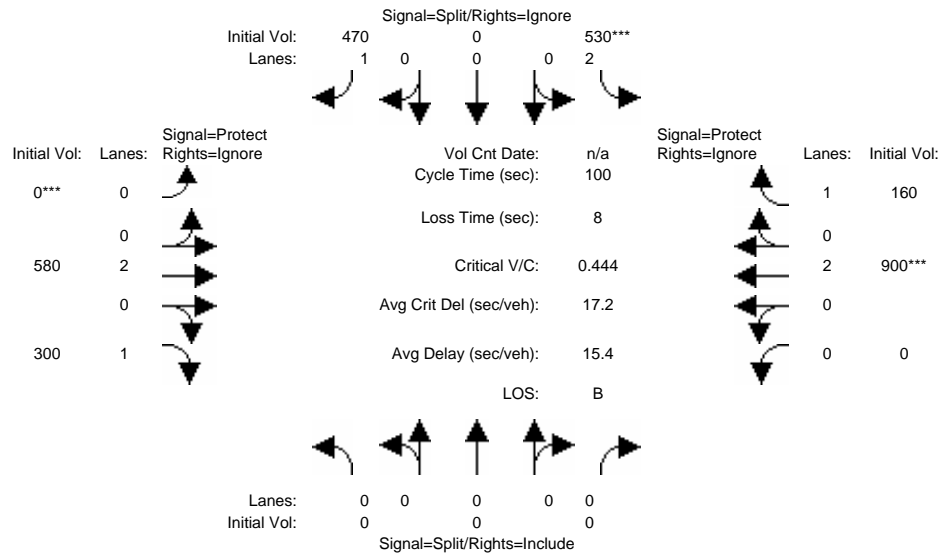


Table with columns for Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.35	0.00	0.00	0.00	0.57	0.00	0.00	0.57	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	5.9	0.0	0.0	0.0	4.3	0.0	0.0	7.5	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.4	0.0	0.0	0.8	0.0
HCM2KQueue:	0.0	0.0	0.0	6.6	0.0	0.0	0.0	4.7	0.0	0.0	8.3	0.0
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.19	1.20	1.20	1.18	1.20
70thHCM2kQ:	0.0	0.0	0.0	7.9	0.0	0.0	0.0	5.6	0.0	0.0	9.8	0.0
85thFactor:	1.60	1.60	1.60	1.54	1.60	1.60	1.60	1.56	1.60	1.60	1.53	1.60
85thHCM2kQ:	0.0	0.0	0.0	10.2	0.0	0.0	0.0	7.4	0.0	0.0	12.7	0.0
90thFactor:	1.80	1.80	1.80	1.69	1.80	1.80	1.80	1.72	1.80	1.80	1.66	1.80
90thHCM2kQ:	0.0	0.0	0.0	11.2	0.0	0.0	0.0	8.1	0.0	0.0	13.9	0.0
95thFactor:	2.10	2.10	2.10	1.91	2.10	2.10	2.10	1.96	2.10	2.10	1.88	2.10
95thHCM2kQ:	0.0	0.0	0.0	12.7	0.0	0.0	0.0	9.3	0.0	0.0	15.7	0.0
98thFactor:	2.70	2.70	2.70	2.30	2.70	2.70	2.70	2.39	2.70	2.70	2.23	2.70
98thHCM2kQ:	0.0	0.0	0.0	15.3	0.0	0.0	0.0	11.3	0.0	0.0	18.6	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	102.2	0.0	0.0	0.0	74.1	0.0	0.0	129	0.0

Name: year 1995 composite fleet
Fuel Consumption: 45.530 pounds
7.376 gallons
Carbon Dioxide: 142.053 pounds
Carbon Monoxide: 10.634 pounds
Hydrocarbons: 1.786 pounds
Nitrogen Oxides: 0.433 pounds

Name: year 2000 composite fleet
Fuel Consumption: 42.036 pounds
6.810 gallons
Carbon Dioxide: 131.152 pounds
Carbon Monoxide: 10.363 pounds
Hydrocarbons: 1.731 pounds
Nitrogen Oxides: 0.382 pounds

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #2: Meadowview Road/I-5 NB Ramps

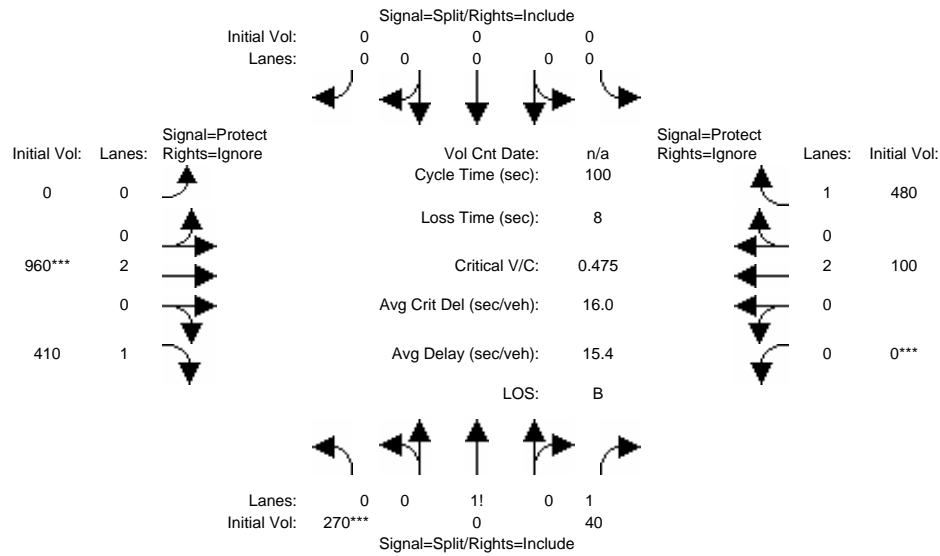


Table containing traffic engineering data: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.35	0.00	0.35	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.57	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.3	0.0	0.7	0.0	0.0	0.0	0.0	8.3	0.0	0.0	0.6	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.9	0.0	0.1	0.0	0.0	0.0	0.0	0.9	0.0	0.0	0.1	0.0
HCM2KQueue:	7.2	0.0	0.8	0.0	0.0	0.0	0.0	9.2	0.0	0.0	0.7	0.0
70thFactor:	1.18	1.20	1.20	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.20
70thHCM2kQ:	8.5	0.0	0.9	0.0	0.0	0.0	0.0	10.8	0.0	0.0	0.8	0.0
85thFactor:	1.54	1.60	1.59	1.60	1.60	1.60	1.60	1.52	1.60	1.60	1.59	1.60
85thHCM2kQ:	11.0	0.0	1.2	0.0	0.0	0.0	0.0	13.9	0.0	0.0	1.1	0.0
90thFactor:	1.68	1.80	1.79	1.80	1.80	1.80	1.80	1.65	1.80	1.80	1.79	1.80
90thHCM2kQ:	12.0	0.0	1.4	0.0	0.0	0.0	0.0	15.1	0.0	0.0	1.2	0.0
95thFactor:	1.90	2.10	2.08	2.10	2.10	2.10	2.10	1.86	2.10	2.10	2.08	2.10
95thHCM2kQ:	13.6	0.0	1.6	0.0	0.0	0.0	0.0	17.0	0.0	0.0	1.4	0.0
98thFactor:	2.28	2.70	2.64	2.70	2.70	2.70	2.70	2.19	2.70	2.70	2.65	2.70
98thHCM2kQ:	16.3	0.0	2.0	0.0	0.0	0.0	0.0	20.1	0.0	0.0	1.8	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	52.7	0.0	6.7	0.0	0.0	0.0	0.0	141	0.0	0.0	11.0	0.0

Name: year 1995 composite fleet
Fuel Consumption: 31.043 pounds
5.029 gallons
Carbon Dioxide: 96.855 pounds
Carbon Monoxide: 7.249 pounds
Hydrocarbons: 1.216 pounds
Nitrogen Oxides: 0.296 pounds

Name: year 2000 composite fleet
Fuel Consumption: 28.655 pounds
4.642 gallons
Carbon Dioxide: 89.404 pounds
Carbon Monoxide: 7.064 pounds
Hydrocarbons: 1.178 pounds
Nitrogen Oxides: 0.261 pounds

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.45	0.00	0.45	0.00	0.00	0.00	0.00	0.47	0.00	0.00	0.47	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	8.2	0.0	2.1	0.0	0.0	0.0	0.0	9.1	0.0	0.0	3.3	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	1.1	0.0	0.2	0.0	0.0	0.0	0.0	1.1	0.0	0.0	0.3	0.0
HCM2KQueue:	9.3	0.0	2.3	0.0	0.0	0.0	0.0	10.2	0.0	0.0	3.6	0.0
70thFactor:	1.18	1.20	1.19	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.19	1.20
70thHCM2kQ:	10.9	0.0	2.8	0.0	0.0	0.0	0.0	12.1	0.0	0.0	4.2	0.0
85thFactor:	1.52	1.60	1.58	1.60	1.60	1.60	1.60	1.51	1.60	1.60	1.57	1.60
85thHCM2kQ:	14.1	0.0	3.6	0.0	0.0	0.0	0.0	15.5	0.0	0.0	5.6	0.0
90thFactor:	1.65	1.80	1.76	1.80	1.80	1.80	1.80	1.64	1.80	1.80	1.73	1.80
90thHCM2kQ:	15.3	0.0	4.0	0.0	0.0	0.0	0.0	16.8	0.0	0.0	6.2	0.0
95thFactor:	1.86	2.10	2.03	2.10	2.10	2.10	2.10	1.84	2.10	2.10	1.99	2.10
95thHCM2kQ:	17.2	0.0	4.7	0.0	0.0	0.0	0.0	18.8	0.0	0.0	7.1	0.0
98thFactor:	2.19	2.70	2.54	2.70	2.70	2.70	2.70	2.15	2.70	2.70	2.46	2.70
98thHCM2kQ:	20.3	0.0	5.8	0.0	0.0	0.0	0.0	22.1	0.0	0.0	8.8	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	59.9	0.0	22.5	0.0	0.0	0.0	0.0	156	0.0	0.0	55.9	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 43.384 pounds
 7.028 gallons
 Carbon Dioxide: 135.359 pounds
 Carbon Monoxide: 10.306 pounds
 Hydrocarbons: 1.783 pounds
 Nitrogen Oxides: 0.406 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 40.289 pounds
 6.527 gallons
 Carbon Dioxide: 125.701 pounds
 Carbon Monoxide: 10.065 pounds
 Hydrocarbons: 1.733 pounds
 Nitrogen Oxides: 0.359 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.25	0.25	0.08	0.22	0.22	0.30	0.49	0.49	0.11	0.29	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.8	5.3	5.3	1.9	4.4	4.4	5.9	7.8	7.8	1.0	6.2	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	0.6	0.9	0.9	0.9	0.8	0.8	0.9	0.9	0.9	0.3	0.9	0.0
HCM2KQueue:	2.4	6.2	6.2	2.7	5.3	5.3	6.9	8.7	8.7	1.3	7.1	0.0
70thFactor:	1.19	1.19	1.19	1.19	1.19	1.19	1.18	1.18	1.18	1.20	1.18	1.20
70thHCM2kQ:	2.8	7.3	7.3	3.3	6.3	6.3	8.1	10.2	10.2	1.5	8.4	0.0
85thFactor:	1.58	1.54	1.54	1.57	1.55	1.55	1.54	1.52	1.52	1.59	1.54	1.60
85thHCM2kQ:	3.7	9.6	9.6	4.3	8.2	8.2	10.6	13.2	13.2	2.0	10.9	0.0
90thFactor:	1.76	1.69	1.69	1.75	1.71	1.71	1.68	1.66	1.66	1.78	1.68	1.80
90thHCM2kQ:	4.1	10.5	10.5	4.8	9.0	9.0	11.6	14.4	14.4	2.3	11.9	0.0
95thFactor:	2.03	1.93	1.93	2.02	1.95	1.95	1.91	1.87	1.87	2.06	1.90	2.10
95thHCM2kQ:	4.8	11.9	11.9	5.5	10.3	10.3	13.1	16.2	16.2	2.6	13.5	0.0
98thFactor:	2.53	2.32	2.32	2.51	2.37	2.37	2.29	2.21	2.21	2.61	2.28	2.70
98thHCM2kQ:	6.0	14.4	14.4	6.9	12.5	12.5	15.7	19.2	19.2	3.3	16.2	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	16.2	79.3	10.7	16.8	65.2	10.9	53.4	123	10.0	9.1	105	0.0

Name: year 1995 composite fleet
Fuel Consumption: 80.140 pounds
12.983 gallons
Carbon Dioxide: 250.036 pounds
Carbon Monoxide: 19.821 pounds
Hydrocarbons: 3.678 pounds
Nitrogen Oxides: 0.699 pounds

Name: year 2000 composite fleet
Fuel Consumption: 75.540 pounds
12.237 gallons
Carbon Dioxide: 235.683 pounds
Carbon Monoxide: 19.460 pounds
Hydrocarbons: 3.605 pounds
Nitrogen Oxides: 0.627 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.12	0.12	0.36	0.37	0.37	0.13	0.39	0.39	0.05	0.31	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.5	2.2	2.2	8.3	9.8	9.8	4.0	10.6	10.6	1.4	9.3	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	1.6	0.6	0.6	1.3	1.8	1.8	1.7	1.8	1.8	1.1	1.9	0.0
HCM2KQueue:	5.0	2.8	2.8	9.6	11.6	11.6	5.7	12.3	12.3	2.4	11.2	0.0
70thFactor:	1.19	1.19	1.19	1.18	1.17	1.17	1.19	1.17	1.17	1.19	1.18	1.20
70thHCM2kQ:	6.0	3.3	3.3	11.3	13.6	13.6	6.7	14.5	14.5	2.9	13.1	0.0
85thFactor:	1.55	1.57	1.57	1.52	1.50	1.50	1.55	1.50	1.50	1.58	1.51	1.60
85thHCM2kQ:	7.8	4.4	4.4	14.5	17.4	17.4	8.8	18.5	18.5	3.8	16.8	0.0
90thFactor:	1.71	1.75	1.75	1.65	1.62	1.62	1.70	1.62	1.62	1.75	1.63	1.80
90thHCM2kQ:	8.6	4.9	4.9	15.8	18.8	18.8	9.7	19.9	19.9	4.2	18.2	0.0
95thFactor:	1.95	2.01	2.01	1.85	1.82	1.82	1.94	1.80	1.80	2.02	1.82	2.10
95thHCM2kQ:	9.8	5.6	5.6	17.8	21.0	21.0	11.0	22.2	22.2	4.9	20.4	0.0
98thFactor:	2.38	2.51	2.51	2.18	2.11	2.11	2.35	2.09	2.09	2.53	2.12	2.70
98thHCM2kQ:	12.0	7.0	7.0	20.9	24.4	24.4	13.3	25.7	25.7	6.1	23.7	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	31.1	25.4	11.5	74.5	106	62.2	35.8	170	10.3	12.2	159	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 111.920 pounds
 18.131 gallons
 Carbon Dioxide: 349.189 pounds
 Carbon Monoxide: 27.935 pounds
 Hydrocarbons: 5.256 pounds
 Nitrogen Oxides: 0.964 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 105.844 pounds
 17.147 gallons
 Carbon Dioxide: 330.232 pounds
 Carbon Monoxide: 27.457 pounds
 Hydrocarbons: 5.160 pounds
 Nitrogen Oxides: 0.867 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP AM

Intersection #4: Meadowview Road/Manorside Drive

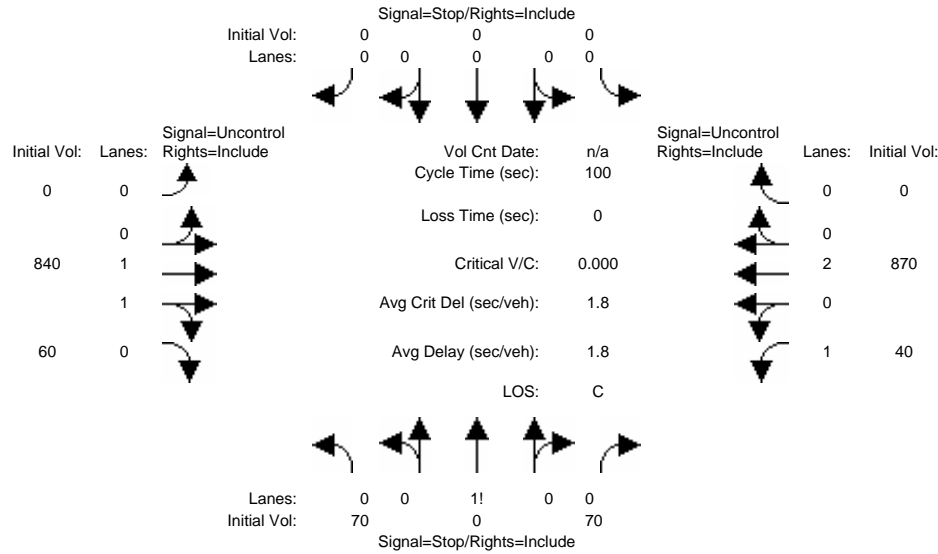


Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Volume Module, and Final Vol.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Capacity Module and Level of Service Module.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Shared Queue, Shared ConDel, Shared LOS, and ApproachDel.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include HevVeh, Grade, Ped/ Hour, and Lane Width.

Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Two-Stage Gap Acceptance - Stage One Module.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Two-Stage Gap Acceptance - Stage Two Module.

Peak Hour Delay Signal Warrant Report

Intersection #4 Meadowview Road/Manorside Drive
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.9]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=140]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=1950]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	0
Initial Vol:	70	0	70	0	0	0	0	840	60	40	870	0

Major Street Volume: 1810
Minor Approach Volume: 140
Minor Approach Volume Threshold: 80 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP PM

Intersection #4: Meadowview Road/Manorside Drive

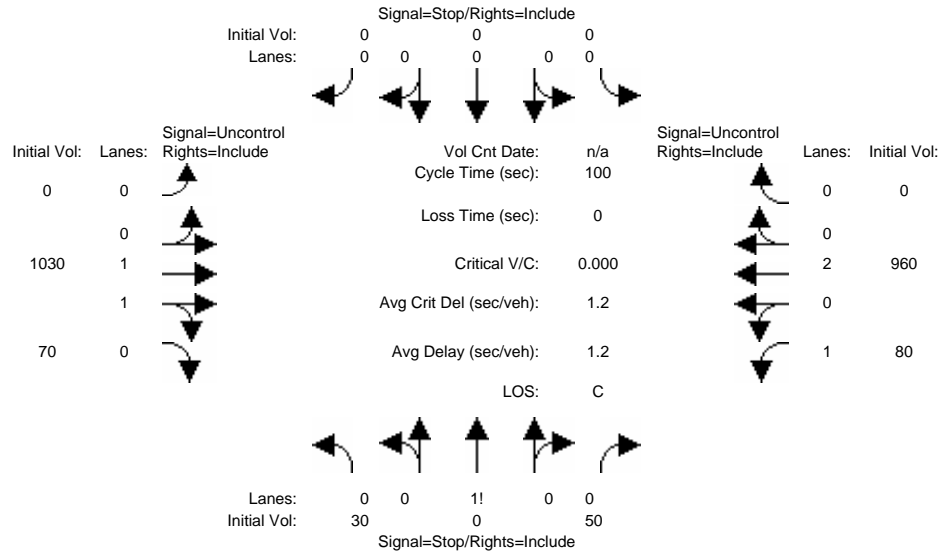


Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Volume Module, Critical Gap Module, Capacity Module, Level of Service Module, and Pedestrian data.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Level of Service Module, Pedestrian, and Two-Stage Gap Acceptance data.

Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]
Two-Stage Gap Acceptance - Stage One Module:
Two-Stage Gap Acceptance - Stage Two Module:

Peak Hour Delay Signal Warrant Report

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=80]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=2220]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

-----|-----|-----|-----|-----|

Approach: North Bound South Bound East Bound West Bound

Movement: L T R L T R L T R L T R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Initial Vol: 30 0 50 0 0 0 0 0 0 1030 70 80 960 0

-----|-----|-----|-----|-----|

Major Street Volume: 2140

Minor Approach Volume: 80

Minor Approach Volume Threshold: 23 [less than minimum of 100]

-----|-----|-----|-----|-----|

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #5: Meadowview Road/24th Street

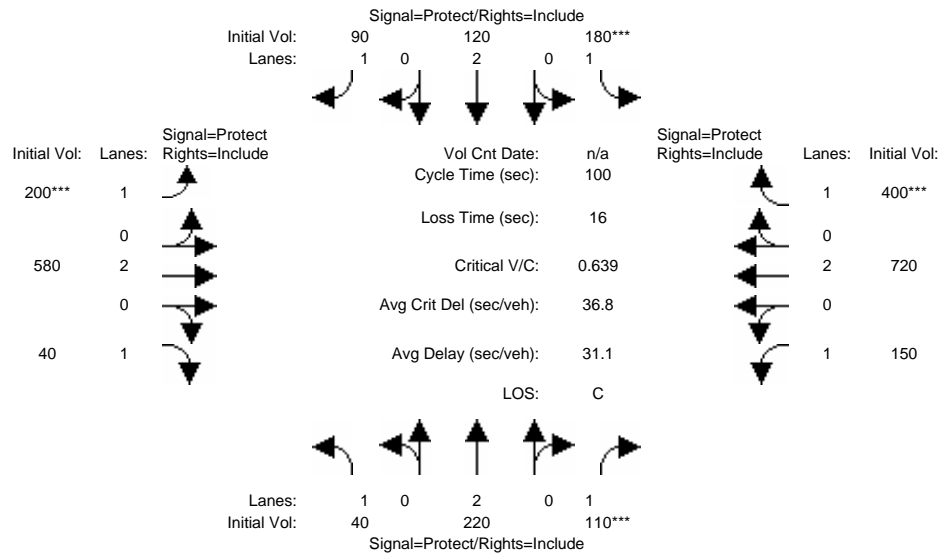


Table with columns for Approach, Movement, and various performance metrics (Volume Module, Saturation Flow Module, Capacity Analysis Module, etc.) for North, South, East, and West bounds.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.11	0.11	0.16	0.16	0.16	0.18	0.38	0.38	0.20	0.40	0.40
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.0	3.1	2.9	4.7	1.5	2.2	5.2	6.3	0.7	3.7	8.0	9.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.3	1.2	1.5	1.6	0.3	0.6	1.6	0.8	0.1	0.7	1.0	1.7
HCM2KQueue:	1.3	4.3	4.4	6.3	1.8	2.8	6.7	7.1	0.8	4.4	9.0	10.7
70thFactor:	1.20	1.19	1.19	1.19	1.20	1.19	1.18	1.18	1.20	1.19	1.18	1.18
70thHCM2kQ:	1.5	5.1	5.3	7.4	2.2	3.3	8.0	8.4	0.9	5.2	10.7	12.5
85thFactor:	1.59	1.56	1.56	1.54	1.58	1.57	1.54	1.54	1.59	1.56	1.52	1.51
85thHCM2kQ:	2.0	6.6	6.9	9.7	2.9	4.4	10.4	10.9	1.2	6.9	13.7	16.1
90thFactor:	1.78	1.72	1.72	1.69	1.77	1.75	1.69	1.68	1.78	1.72	1.65	1.63
90thHCM2kQ:	2.2	7.3	7.6	10.6	3.2	4.9	11.4	11.9	1.4	7.6	14.9	17.4
95thFactor:	2.06	1.97	1.97	1.92	2.04	2.01	1.91	1.90	2.07	1.97	1.86	1.83
95thHCM2kQ:	2.6	8.4	8.7	12.0	3.7	5.6	12.9	13.5	1.6	8.7	16.8	19.5
98thFactor:	2.61	2.42	2.41	2.32	2.57	2.51	2.30	2.28	2.64	2.41	2.20	2.14
98thHCM2kQ:	3.3	10.3	10.6	14.5	4.6	7.0	15.5	16.1	2.1	10.6	19.9	22.8

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	9.1	52.3	26.3	42.1	26.2	20.1	46.4	108	6.4	33.0	137	80.9

Name: year 1995 composite fleet
 Fuel Consumption: 97.248 pounds
 15.754 gallons
 Carbon Dioxide: 303.414 pounds
 Carbon Monoxide: 24.285 pounds
 Hydrocarbons: 4.579 pounds
 Nitrogen Oxides: 0.831 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 92.004 pounds
 14.905 gallons
 Carbon Dioxide: 287.052 pounds
 Carbon Monoxide: 23.873 pounds
 Hydrocarbons: 4.496 pounds
 Nitrogen Oxides: 0.747 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #5: Meadowview Road/24th Street

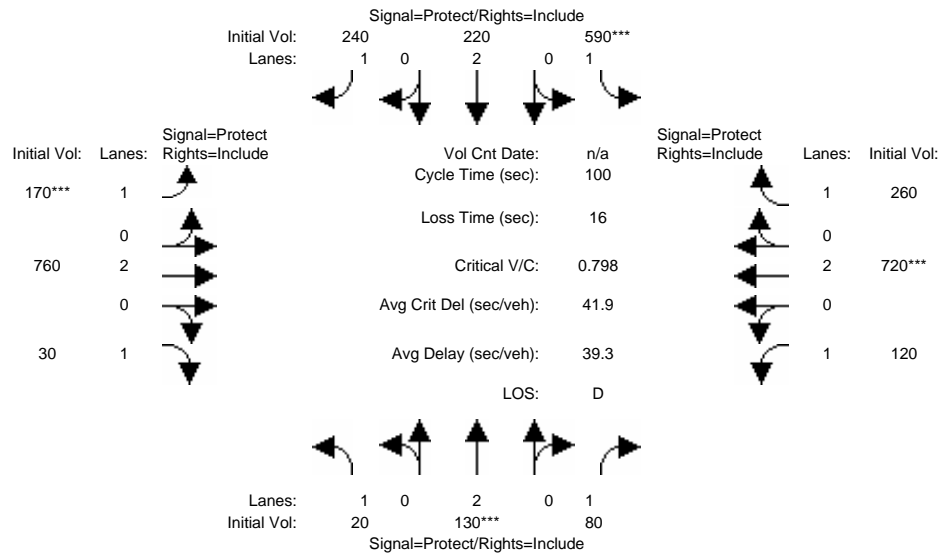


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.07	0.07	0.41	0.36	0.36	0.12	0.28	0.28	0.09	0.25	0.25
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.5	1.8	2.2	14.6	2.2	5.0	4.6	10.2	0.6	3.3	9.9	6.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.1	1.0	1.8	3.7	0.2	0.7	2.9	2.9	0.1	2.3	3.5	1.8
HCM2KQueue:	0.6	2.8	4.0	18.4	2.4	5.8	7.5	13.1	0.7	5.5	13.4	8.3
70th%Factor:	1.20	1.19	1.19	1.16	1.19	1.19	1.18	1.17	1.20	1.19	1.17	1.18
70th%HCM2kQ:	0.7	3.4	4.7	21.4	2.9	6.8	8.8	15.4	0.8	6.6	15.7	9.8
85th%Factor:	1.59	1.57	1.56	1.46	1.58	1.55	1.53	1.49	1.59	1.55	1.49	1.53
85th%HCM2kQ:	1.0	4.4	6.2	26.9	3.8	8.9	11.5	19.6	1.1	8.6	20.0	12.7
90th%Factor:	1.79	1.75	1.73	1.56	1.75	1.70	1.68	1.61	1.79	1.70	1.60	1.66
90th%HCM2kQ:	1.1	4.9	6.8	28.6	4.2	9.8	12.5	21.1	1.2	9.4	21.5	13.8
95th%Factor:	2.08	2.01	1.98	1.72	2.02	1.94	1.90	1.79	2.08	1.94	1.78	1.88
95th%HCM2kQ:	1.3	5.7	7.8	31.5	4.9	11.2	14.2	23.5	1.4	10.7	23.9	15.6
98th%Factor:	2.65	2.51	2.44	1.94	2.53	2.34	2.26	2.06	2.65	2.35	2.06	2.23
98th%HCM2kQ:	1.6	7.1	9.6	35.7	6.1	13.5	16.9	27.1	1.8	13.0	27.6	18.5

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	4.5	31.4	19.6	131.5	37.7	45.4	41.5	175	5.5	29.4	170	58.5

Name: year 1995 composite fleet
 Fuel Consumption: 133.662 pounds
 21.653 gallons
 Carbon Dioxide: 417.026 pounds
 Carbon Monoxide: 34.027 pounds
 Hydrocarbons: 6.620 pounds
 Nitrogen Oxides: 1.092 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 127.401 pounds
 20.639 gallons
 Carbon Dioxide: 397.492 pounds
 Carbon Monoxide: 33.532 pounds
 Hydrocarbons: 6.521 pounds
 Nitrogen Oxides: 0.990 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.17	0.17	0.17	0.07	0.07	0.07	0.05	0.51	0.51	0.10	0.55	0.55
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.8	3.8	4.4	0.9	0.9	1.1	1.4	9.0	9.0	1.6	12.7	3.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.0	1.0	1.6	0.4	0.4	0.5	1.1	1.1	1.1	0.5	1.7	0.4
HCM2KQueue:	4.7	4.7	6.0	1.3	1.3	1.6	2.4	10.1	10.1	2.1	14.4	3.9
70thFactor:	1.19	1.19	1.19	1.20	1.20	1.20	1.19	1.18	1.18	1.19	1.17	1.19
70thHCM2kQ:	5.6	5.6	7.1	1.6	1.6	1.9	2.9	11.9	11.9	2.5	16.8	4.6
85thFactor:	1.56	1.56	1.55	1.59	1.59	1.58	1.58	1.51	1.51	1.58	1.49	1.56
85thHCM2kQ:	7.4	7.4	9.2	2.1	2.1	2.5	3.8	15.3	15.3	3.3	21.4	6.1
90thFactor:	1.72	1.72	1.70	1.77	1.77	1.77	1.75	1.64	1.64	1.76	1.59	1.73
90thHCM2kQ:	8.1	8.1	10.1	2.3	2.3	2.8	4.2	16.6	16.6	3.7	22.9	6.7
95thFactor:	1.96	1.96	1.93	2.06	2.06	2.05	2.02	1.84	1.84	2.03	1.77	1.98
95thHCM2kQ:	9.3	9.3	11.5	2.7	2.7	3.3	4.9	18.6	18.6	4.2	25.4	7.7
98thFactor:	2.39	2.39	2.33	2.60	2.60	2.58	2.53	2.16	2.16	2.55	2.03	2.44
98thHCM2kQ:	11.4	11.4	13.9	3.4	3.4	4.1	6.1	21.8	21.8	5.3	29.2	9.5

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	31.8	2.3	39.6	14.2	2.4	9.5	12.2	141	13.4	14.0	216	31.7

Name: year 1995 composite fleet
Fuel Consumption: 79.400 pounds
12.863 gallons
Carbon Dioxide: 247.728 pounds
Carbon Monoxide: 19.136 pounds
Hydrocarbons: 3.396 pounds
Nitrogen Oxides: 0.727 pounds

Name: year 2000 composite fleet
Fuel Consumption: 74.122 pounds
12.008 gallons
Carbon Dioxide: 231.259 pounds
Carbon Monoxide: 18.724 pounds
Hydrocarbons: 3.312 pounds
Nitrogen Oxides: 0.646 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP PM

Intersection #6: Meadowview Road/Detroit Boulevard

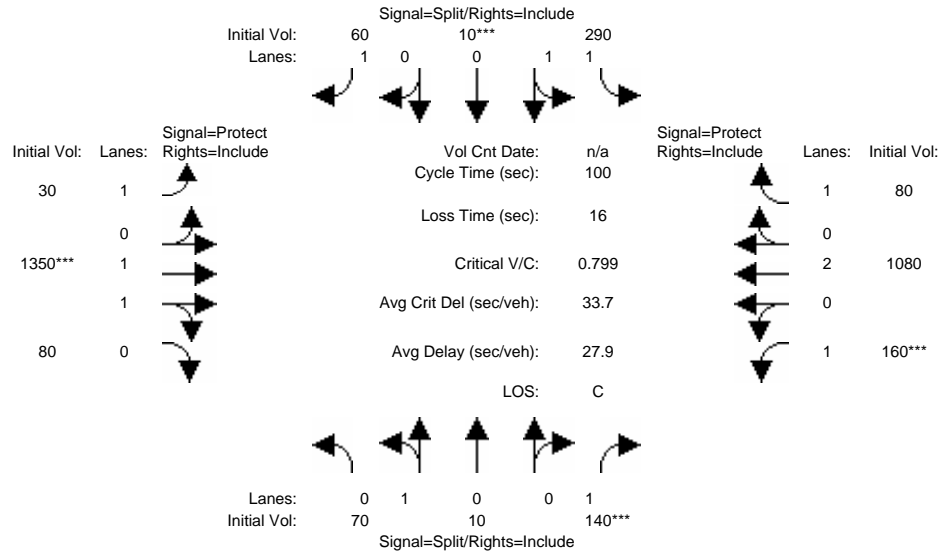


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.11	0.11	0.11	0.11	0.11	0.09	0.51	0.51	0.11	0.54	0.54
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	2.1	2.1	3.8	4.1	4.1	1.5	0.8	17.3	17.3	4.3	10.6	1.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.7	0.7	2.5	2.6	2.6	0.5	0.2	3.5	3.5	2.6	1.3	0.1
HCM2KQueue:	2.7	2.7	6.3	6.6	6.6	2.1	1.0	20.7	20.7	6.9	11.8	1.2
70thFactor:	1.19	1.19	1.19	1.18	1.18	1.19	1.20	1.16	1.16	1.18	1.17	1.20
70thHCM2kQ:	3.2	3.2	7.5	7.9	7.9	2.5	1.2	24.0	24.0	8.2	13.9	1.4
85thFactor:	1.57	1.57	1.54	1.54	1.54	1.58	1.59	1.45	1.45	1.54	1.50	1.59
85thHCM2kQ:	4.3	4.3	9.7	10.2	10.2	3.3	1.6	30.1	30.1	10.7	17.8	1.9
90thFactor:	1.75	1.75	1.69	1.69	1.69	1.76	1.78	1.54	1.54	1.68	1.62	1.78
90thHCM2kQ:	4.8	4.8	10.7	11.2	11.2	3.7	1.8	32.0	32.0	11.7	19.2	2.1
95thFactor:	2.02	2.02	1.92	1.91	1.91	2.03	2.07	1.69	1.69	1.91	1.81	2.06
95thHCM2kQ:	5.5	5.5	12.1	12.7	12.7	4.2	2.1	35.0	35.0	13.3	21.4	2.5
98thFactor:	2.51	2.51	2.32	2.30	2.30	2.55	2.63	1.90	1.90	2.29	2.10	2.61
98thHCM2kQ:	6.8	6.8	14.6	15.3	15.3	5.3	2.6	39.5	39.5	15.9	24.9	3.1

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	16.3	2.3	34.1	70.8	2.4	13.9	7.0	279	16.5	39.0	180	9.8

Name: year 1995 composite fleet
 Fuel Consumption: 107.075 pounds
 17.346 gallons
 Carbon Dioxide: 334.073 pounds
 Carbon Monoxide: 26.488 pounds
 Hydrocarbons: 4.915 pounds
 Nitrogen Oxides: 0.935 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 100.932 pounds
 16.351 gallons
 Carbon Dioxide: 314.907 pounds
 Carbon Monoxide: 26.006 pounds
 Hydrocarbons: 4.817 pounds
 Nitrogen Oxides: 0.838 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #7: Mack Road/Franklin Boulevard

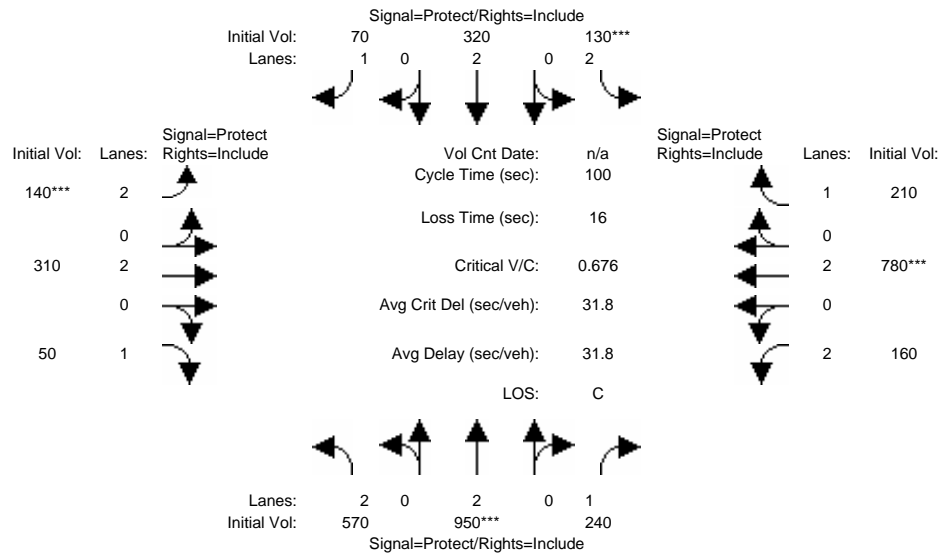


Table containing detailed traffic engineering data including:
- Approach and Movement details for North, South, East, and West bounds.
- Volume Module with Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, etc.
- Saturation Flow Module with Sat/Lane, Adjustment, Lanes, and Final Sat.
- Capacity Analysis Module with Vol/Sat, Crit Moves, Green/Cycle, etc.
- HCM Ops Adjusted Lane Utilization Module with Lanes, Lane Group, and #LnsInGrps.
- HCM Ops Input Saturation Adj Module with Lane Width, CrosswalkWid, etc.
- HCM Ops f(lt) Adj Case Module with f(lt) Case.
- HCM Ops Saturation Adj Module with Ln Wid Adj, Hev Veh Adj, etc.
- Delay Adjustment Factor Module with Coordinated, Signal Type, and DelAdjPctr.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.29	0.40	0.40	0.06	0.16	0.16	0.06	0.25	0.25	0.14	0.33	0.33
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.9	11.4	4.7	1.8	4.3	1.7	2.0	3.7	1.1	2.1	9.9	4.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.2	2.0	0.6	1.5	1.2	0.4	1.5	0.5	0.1	0.5	1.9	0.7
HCM2KQueue:	8.2	13.4	5.3	3.3	5.5	2.1	3.5	4.3	1.2	2.6	11.8	5.2
70thFactor:	1.18	1.17	1.19	1.19	1.19	1.19	1.19	1.19	1.20	1.19	1.17	1.19
70thHCM2kQ:	9.6	15.7	6.3	4.0	6.6	2.5	4.2	5.1	1.5	3.0	13.8	6.2
85thFactor:	1.53	1.49	1.55	1.57	1.55	1.58	1.57	1.56	1.59	1.58	1.50	1.55
85thHCM2kQ:	12.5	20.0	8.3	5.2	8.6	3.3	5.5	6.7	1.9	4.0	17.7	8.1
90thFactor:	1.67	1.60	1.71	1.74	1.70	1.76	1.74	1.72	1.78	1.75	1.62	1.71
90thHCM2kQ:	13.6	21.5	9.1	5.8	9.4	3.7	6.1	7.4	2.2	4.5	19.1	8.9
95thFactor:	1.88	1.78	1.95	2.00	1.94	2.03	1.99	1.97	2.06	2.02	1.81	1.95
95thHCM2kQ:	15.4	23.9	10.4	6.7	10.7	4.2	7.0	8.5	2.5	5.2	21.4	10.1
98thFactor:	2.23	2.06	2.36	2.47	2.35	2.55	2.46	2.42	2.61	2.52	2.10	2.37
98thHCM2kQ:	18.2	27.6	12.6	8.3	13.0	5.3	8.6	10.4	3.2	6.4	24.8	12.3

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	120.7	196	42.6	31.9	73.9	15.4	34.3	64.0	9.7	36.1	169	40.8

Name: year 1995 composite fleet
 Fuel Consumption: 136.257 pounds
 22.074 gallons
 Carbon Dioxide: 425.123 pounds
 Carbon Monoxide: 34.087 pounds
 Hydrocarbons: 6.441 pounds
 Nitrogen Oxides: 1.165 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 128.982 pounds
 20.895 gallons
 Carbon Dioxide: 402.424 pounds
 Carbon Monoxide: 33.514 pounds
 Hydrocarbons: 6.326 pounds
 Nitrogen Oxides: 1.048 pounds

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #7: Mack Road/Franklin Boulevard

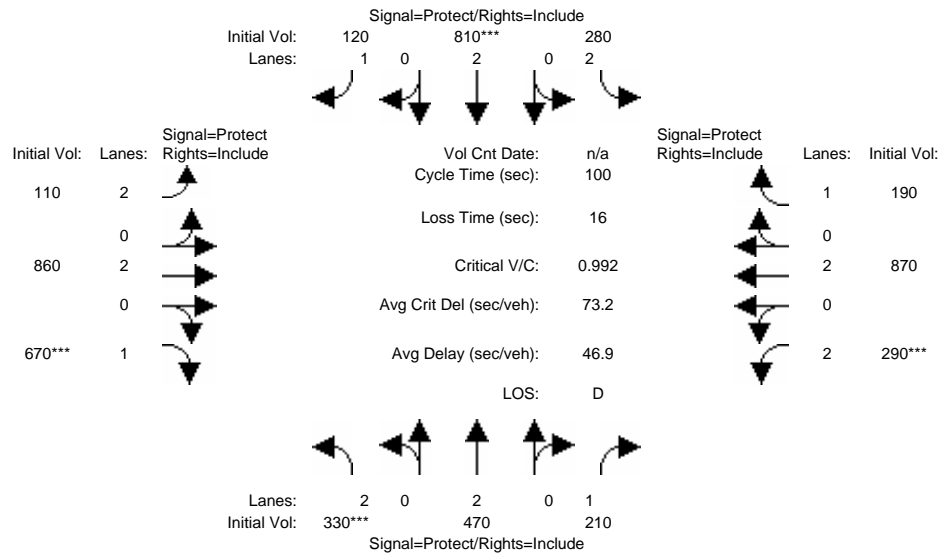


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.10	0.20	0.20	0.12	0.23	0.23	0.09	0.43	0.43	0.09	0.43	0.43
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.7	6.3	5.4	3.8	11.8	2.8	1.5	9.5	18.5	4.1	9.7	3.4
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	4.5	1.7	1.7	1.6	7.1	0.5	0.6	1.3	8.8	4.2	1.3	0.4
HCM2KQueue:	9.2	8.0	7.0	5.4	18.9	3.3	2.0	10.8	27.3	8.4	11.0	3.8
70thFactor:	1.18	1.18	1.18	1.19	1.16	1.19	1.20	1.18	1.15	1.18	1.18	1.19
70thHCM2kQ:	10.9	9.5	8.3	6.5	22.0	3.9	2.4	12.7	31.4	9.9	13.0	4.6
85thFactor:	1.52	1.53	1.54	1.55	1.46	1.57	1.58	1.51	1.42	1.53	1.51	1.56
85thHCM2kQ:	14.1	12.3	10.8	8.4	27.6	5.1	3.2	16.3	38.8	12.8	16.6	6.0
90thFactor:	1.65	1.67	1.68	1.70	1.56	1.74	1.76	1.63	1.50	1.66	1.63	1.73
90thHCM2kQ:	15.3	13.4	11.8	9.3	29.4	5.7	3.6	17.6	41.0	14.0	18.0	6.6
95thFactor:	1.86	1.88	1.91	1.94	1.71	2.00	2.04	1.83	1.63	1.88	1.83	1.98
95thHCM2kQ:	17.2	15.1	13.4	10.6	32.3	6.5	4.2	19.8	44.5	15.7	20.1	7.6
98thFactor:	2.19	2.24	2.28	2.36	1.93	2.48	2.55	2.14	1.82	2.22	2.13	2.44
98thHCM2kQ:	20.3	18.0	16.1	12.8	36.5	8.1	5.2	23.1	49.8	18.7	23.4	9.4

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	82.4	108	48.2	66.7	202	25.0	26.0	163	166.5	72.4	166	31.0

Name: year 1995 composite fleet
 Fuel Consumption: 235.463 pounds
 38.145 gallons
 Carbon Dioxide: 734.645 pounds
 Carbon Monoxide: 60.761 pounds
 Hydrocarbons: 12.107 pounds
 Nitrogen Oxides: 1.828 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 225.722 pounds
 36.567 gallons
 Carbon Dioxide: 704.251 pounds
 Carbon Monoxide: 59.992 pounds
 Hydrocarbons: 11.953 pounds
 Nitrogen Oxides: 1.670 pounds

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #8: Cosumnes River Boulevard/Franklin Boulevard

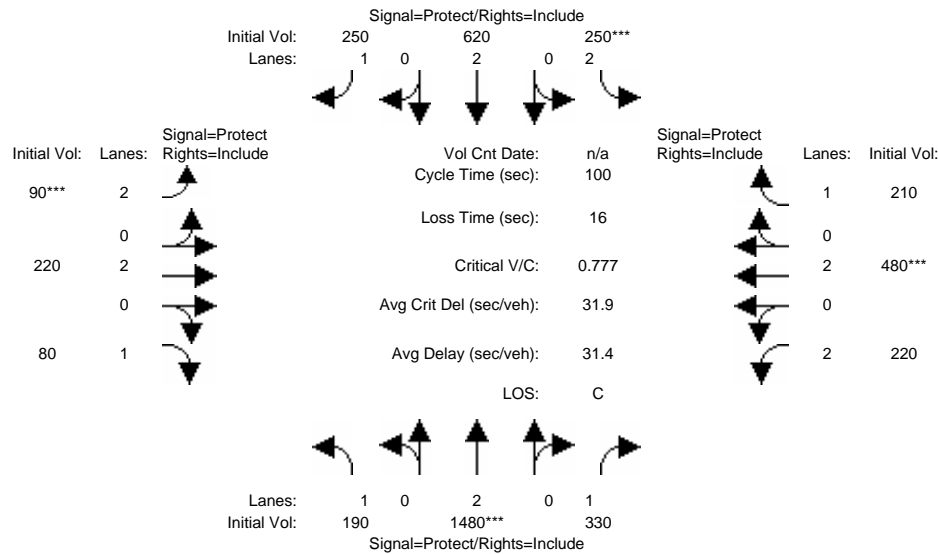


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.24	0.53	0.53	0.09	0.38	0.38	0.05	0.12	0.12	0.11	0.17	0.17
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.5	17.6	5.5	3.5	6.8	5.1	1.3	3.0	2.1	3.0	6.7	5.6
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.8	3.4	0.6	2.4	0.8	0.7	0.9	1.1	0.7	1.3	2.9	2.6
HCM2KQueue:	5.3	21.0	6.1	5.9	7.6	5.8	2.2	4.1	2.8	4.4	9.6	8.2
70thFactor:	1.19	1.16	1.19	1.19	1.18	1.19	1.19	1.19	1.19	1.19	1.18	1.18
70thHCM2kQ:	6.3	24.3	7.3	7.0	9.0	6.8	2.6	4.9	3.4	5.2	11.3	9.6
85thFactor:	1.55	1.45	1.54	1.55	1.53	1.55	1.58	1.56	1.57	1.56	1.52	1.53
85thHCM2kQ:	8.3	30.4	9.5	9.2	11.6	8.9	3.5	6.4	4.4	6.8	14.5	12.5
90thFactor:	1.71	1.54	1.69	1.70	1.67	1.70	1.76	1.73	1.75	1.72	1.65	1.67
90thHCM2kQ:	9.1	32.3	10.4	10.1	12.7	9.8	3.9	7.1	4.9	7.5	15.8	13.6
95thFactor:	1.95	1.69	1.93	1.93	1.89	1.94	2.03	1.98	2.01	1.97	1.85	1.88
95thHCM2kQ:	10.4	35.4	11.8	11.5	14.4	11.2	4.5	8.1	5.7	8.6	17.8	15.4
98thFactor:	2.36	1.90	2.32	2.33	2.26	2.34	2.54	2.43	2.51	2.42	2.18	2.23
98thHCM2kQ:	12.6	39.8	14.2	13.9	17.2	13.5	5.6	10.0	7.0	10.5	20.9	18.2

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	40.7	301	49.3	61.2	116	45.7	22.0	51.9	18.6	52.6	115	50.2

Name: year 1995 composite fleet
Fuel Consumption: 151.818 pounds
24.595 gallons
Carbon Dioxide: 473.672 pounds
Carbon Monoxide: 37.939 pounds
Hydrocarbons: 7.159 pounds
Nitrogen Oxides: 1.298 pounds

Name: year 2000 composite fleet
Fuel Consumption: 143.662 pounds
23.273 gallons
Carbon Dioxide: 448.224 pounds
Carbon Monoxide: 37.297 pounds
Hydrocarbons: 7.030 pounds
Nitrogen Oxides: 1.168 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP PM

Intersection #8: Cosumnes River Boulevard/Franklin Boulevard

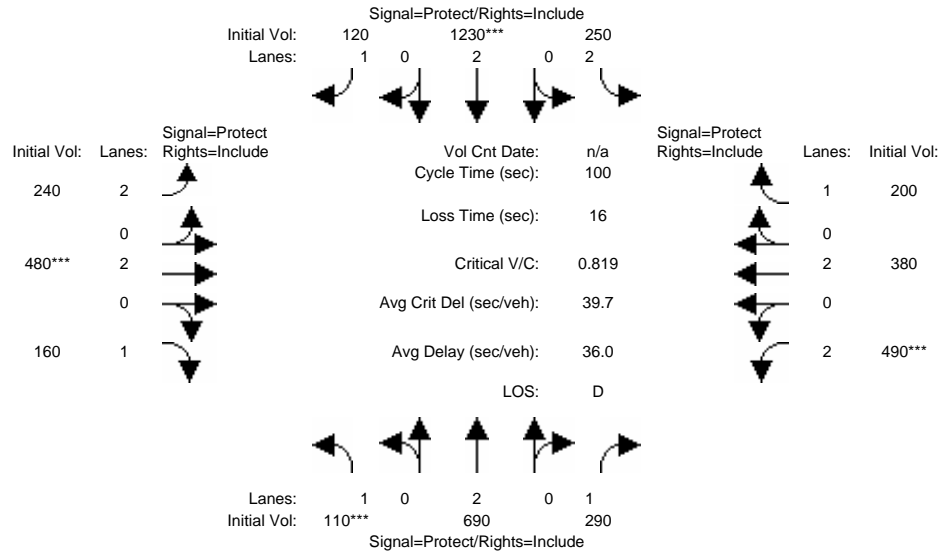


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module (Base Vol, Growth Adj, etc.) and Saturation Flow Module (Sat/Lane, Adjustment, etc.).

Capacity Analysis Module table showing Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

HCM Ops Adjusted Lane Utilization Module table showing Lane Group, #LnsInGrps, and lane utilization percentages.

HCM Ops Input Saturation Adj Module table showing Lane Width, CrosswalkWid, % Hev Veh, Grade, Parking/Hr, Bus Stp/Hr, Area Type, Chft Ped/Hr, ExclusiveRT, and % RT Prtct.

HCM Ops f(lt) Adj Case Module table showing f(lt) Case values for each approach.

HCM Ops Saturation Adj Module table showing Ln Wid Adj, Hev Veh Adj, Grade Adj, Parking Adj, Bus Stp Adj, Area Adj, RT Adj, LT Adj, PedBike Adj, HCM Sat Adj, User Sat Adj, MLF Sat Adj, and Fnl Sat Adj.

Delay Adjustment Factor Module table showing Coordinated, Signal Type, and DelAdjPctr values.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.08	0.36	0.36	0.14	0.42	0.42	0.12	0.17	0.17	0.17	0.22	0.22
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.0	8.0	6.3	3.3	15.9	2.1	3.2	6.8	4.1	6.8	4.9	5.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	2.5	1.1	1.0	1.1	3.7	0.2	1.2	3.1	1.4	3.1	0.9	1.3
HCM2KQueue:	5.5	9.1	7.3	4.4	19.6	2.3	4.5	9.9	5.5	9.9	5.8	6.2
70thFactor:	1.19	1.18	1.18	1.19	1.16	1.19	1.19	1.18	1.19	1.18	1.19	1.19
70thHCM2kQ:	6.5	10.7	8.6	5.2	22.8	2.7	5.3	11.7	6.6	11.6	6.9	7.4
85thFactor:	1.55	1.52	1.54	1.56	1.46	1.58	1.56	1.52	1.55	1.52	1.55	1.54
85thHCM2kQ:	8.5	13.8	11.1	6.9	28.6	3.6	7.0	15.0	8.6	15.0	9.0	9.6
90thFactor:	1.70	1.65	1.68	1.72	1.55	1.76	1.72	1.64	1.70	1.64	1.70	1.69
90thHCM2kQ:	9.4	15.0	12.2	7.6	30.4	4.0	7.7	16.3	9.4	16.3	9.8	10.6
95thFactor:	1.94	1.86	1.90	1.97	1.70	2.03	1.97	1.85	1.94	1.85	1.93	1.92
95thHCM2kQ:	10.7	16.9	13.8	8.7	33.4	4.7	8.8	18.3	10.7	18.3	11.2	12.0
98thFactor:	2.35	2.20	2.27	2.41	1.92	2.54	2.41	2.17	2.35	2.17	2.34	2.32
98thHCM2kQ:	13.0	20.0	16.5	10.6	37.7	5.8	10.8	21.5	13.0	21.4	13.6	14.5

Fuel Consumption and Emissions

2000 HCM Operations Method

Base Volume Alternative

 Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	27.1	136	56.4	58.2	271	18.7	56.7	116	37.1	118.0	83.1	44.7

Name: year 1995 composite fleet

Fuel Consumption: 174.905 pounds
 28.335 gallons
 Carbon Dioxide: 545.702 pounds
 Carbon Monoxide: 44.214 pounds
 Hydrocarbons: 8.501 pounds
 Nitrogen Oxides: 1.458 pounds

Name: year 2000 composite fleet

Fuel Consumption: 166.241 pounds
 26.931 gallons
 Carbon Dioxide: 518.672 pounds
 Carbon Monoxide: 43.530 pounds
 Hydrocarbons: 8.363 pounds
 Nitrogen Oxides: 1.318 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsWalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsWalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #15: Cosumnes River Boulevard/Street B

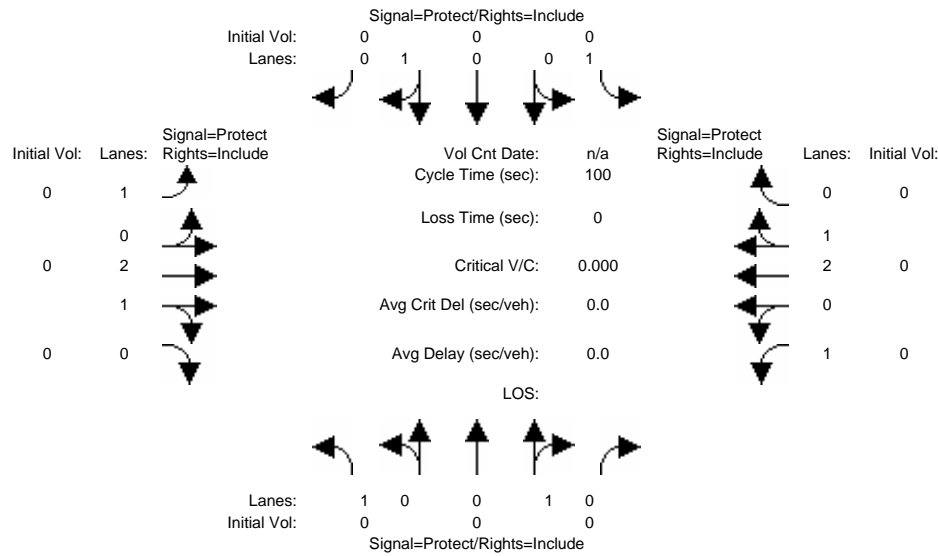


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsWalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsWalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFPIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #15: Cosumnes River Boulevard/Street B

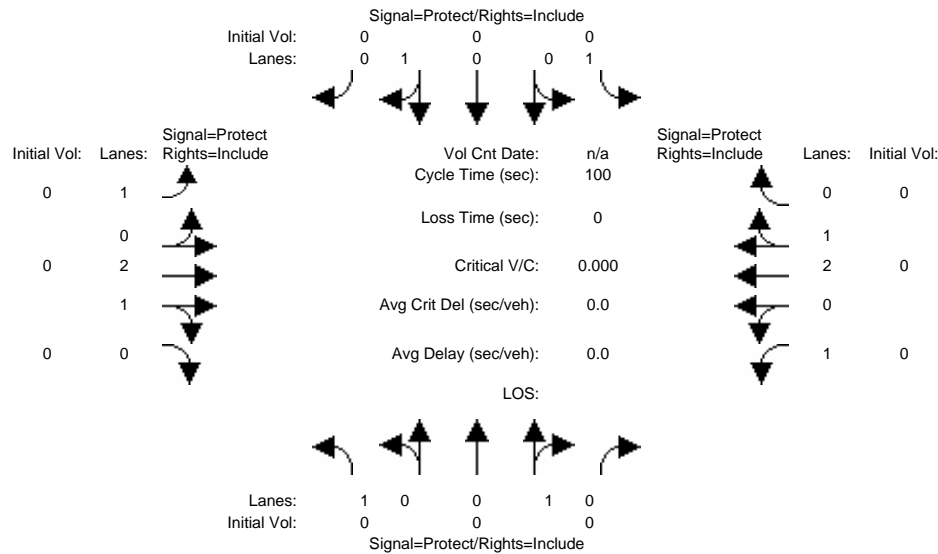


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsSwalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsSwalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #16: Cosumnes River Boulevard/24th Street

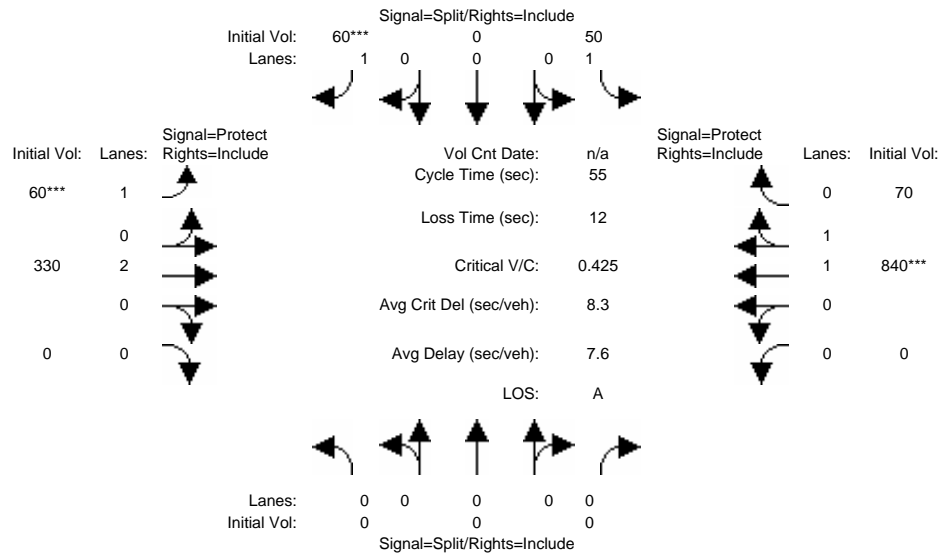


Table with columns for Approach, Movement, and various performance metrics. Includes sections for Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.09	0.00	0.09	0.09	0.69	0.00	0.00	0.60	0.60
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.7	0.0	0.9	0.9	0.9	0.0	0.0	3.9	3.9
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	0.5	0.0	0.7	0.6	0.2	0.0	0.0	0.8	0.8
HCM2KQueue:	0.0	0.0	0.0	1.2	0.0	1.6	1.4	1.1	0.0	0.0	4.7	4.7
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.19	1.19
70thHCM2kQ:	0.0	0.0	0.0	1.4	0.0	1.9	1.7	1.3	0.0	0.0	5.6	5.6
85thFactor:	1.60	1.60	1.60	1.59	1.60	1.58	1.59	1.59	1.60	1.60	1.56	1.56
85thHCM2kQ:	0.0	0.0	0.0	1.9	0.0	2.5	2.3	1.7	0.0	0.0	7.3	7.3
90thFactor:	1.80	1.80	1.80	1.78	1.80	1.77	1.77	1.78	1.80	1.80	1.72	1.72
90thHCM2kQ:	0.0	0.0	0.0	2.1	0.0	2.8	2.5	1.9	0.0	0.0	8.0	8.0
95thFactor:	2.10	2.10	2.10	2.06	2.10	2.05	2.05	2.07	2.10	2.10	1.96	1.96
95thHCM2kQ:	0.0	0.0	0.0	2.4	0.0	3.2	2.9	2.2	0.0	0.0	9.2	9.2
98thFactor:	2.70	2.70	2.70	2.61	2.70	2.59	2.60	2.62	2.70	2.70	2.40	2.40
98thHCM2kQ:	0.0	0.0	0.0	3.1	0.0	4.1	3.7	2.8	0.0	0.0	11.2	11.2

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #16 Cosumnes River Boulevard/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	11.7	0.0	14.2	14.1	27.8	0.0	0.0	11.3	9.4

Name: year 1995 composite fleet
 Fuel Consumption: 24.007 pounds
 3.889 gallons
 Carbon Dioxide: 74.903 pounds
 Carbon Monoxide: 5.217 pounds
 Hydrocarbons: 0.748 pounds
 Nitrogen Oxides: 0.256 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 21.602 pounds
 3.499 gallons
 Carbon Dioxide: 67.398 pounds
 Carbon Monoxide: 5.032 pounds
 Hydrocarbons: 0.710 pounds
 Nitrogen Oxides: 0.222 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.10	0.65	0.00	0.00	0.54	0.54
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	1.0	0.0	1.0	0.8	3.0	0.0	0.0	2.7	2.7
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	0.4	0.0	0.5	0.5	0.5	0.0	0.0	0.5	0.5
HCM2KQueue:	0.0	0.0	0.0	1.4	0.0	1.4	1.3	3.5	0.0	0.0	3.2	3.2
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.19	1.20	1.20	1.19	1.19
70thHCM2kQ:	0.0	0.0	0.0	1.6	0.0	1.7	1.6	4.2	0.0	0.0	3.8	3.8
85thFactor:	1.60	1.60	1.60	1.59	1.60	1.59	1.59	1.57	1.60	1.60	1.57	1.57
85thHCM2kQ:	0.0	0.0	0.0	2.2	0.0	2.3	2.1	5.5	0.0	0.0	5.0	5.0
90thFactor:	1.80	1.80	1.80	1.77	1.80	1.77	1.77	1.74	1.80	1.80	1.74	1.74
90thHCM2kQ:	0.0	0.0	0.0	2.4	0.0	2.5	2.3	6.1	0.0	0.0	5.6	5.6
95thFactor:	2.10	2.10	2.10	2.06	2.10	2.05	2.06	1.99	2.10	2.10	2.00	2.00
95thHCM2kQ:	0.0	0.0	0.0	2.8	0.0	2.9	2.7	7.1	0.0	0.0	6.4	6.4
98thFactor:	2.70	2.70	2.70	2.60	2.70	2.60	2.60	2.46	2.70	2.70	2.48	2.48
98thHCM2kQ:	0.0	0.0	0.0	3.5	0.0	3.7	3.4	8.7	0.0	0.0	8.0	8.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #16 Cosumnes River Boulevard/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	15.7	0.0	15.8	13.9	93.1	0.0	0.0	75.2	9.7

Name: year 1995 composite fleet
 Fuel Consumption: 27.859 pounds
 4.513 gallons
 Carbon Dioxide: 86.921 pounds
 Carbon Monoxide: 6.070 pounds
 Hydrocarbons: 0.874 pounds
 Nitrogen Oxides: 0.297 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 25.086 pounds
 4.064 gallons
 Carbon Dioxide: 78.267 pounds
 Carbon Monoxide: 5.856 pounds
 Hydrocarbons: 0.830 pounds
 Nitrogen Oxides: 0.258 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP PM

Intersection #17: Cosumnes River Boulevard/Stone-Boswell Access West

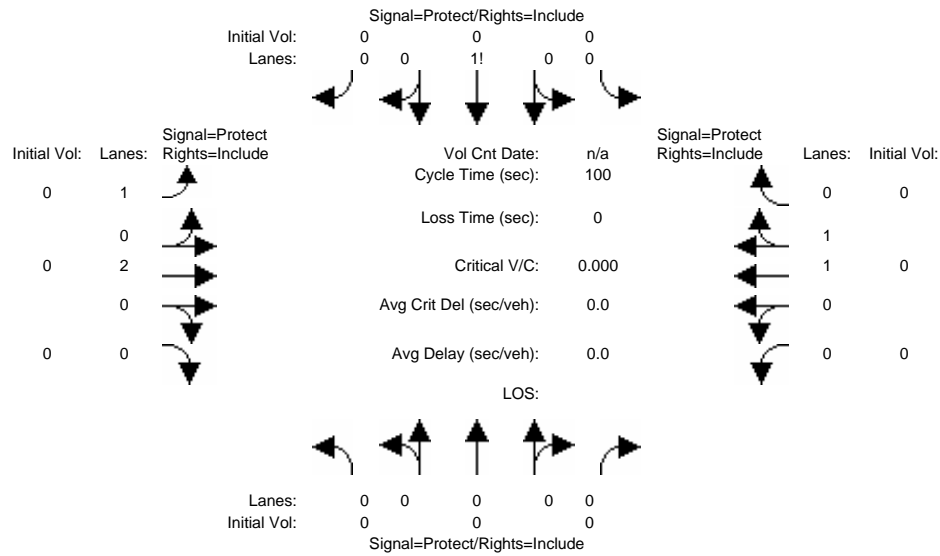


Table with columns: Approach, Movement, Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsWalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsWalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

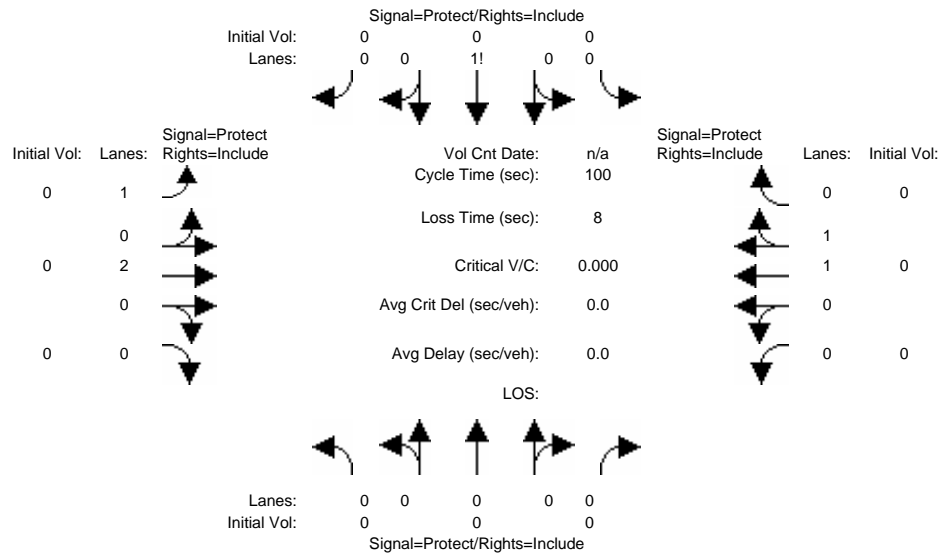


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsWalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsWalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

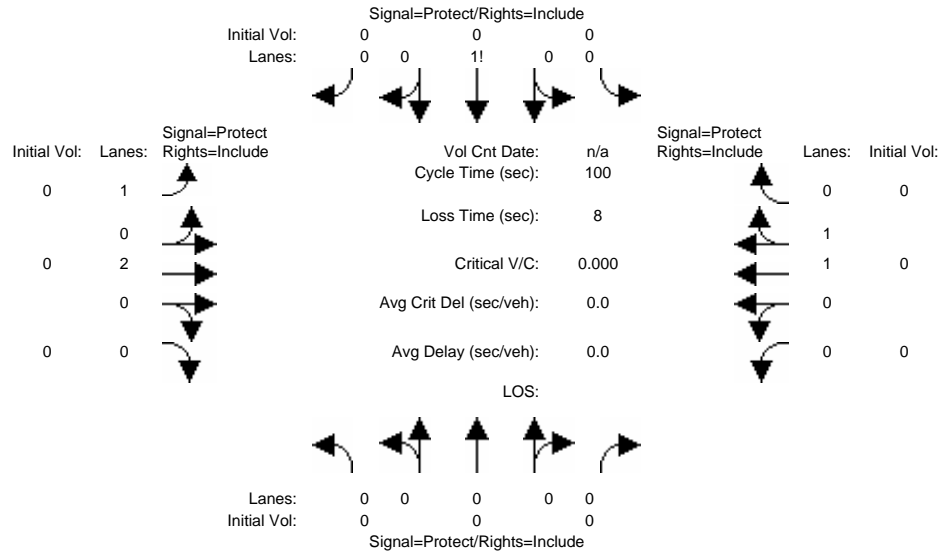


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
POcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFPIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsWalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsWalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

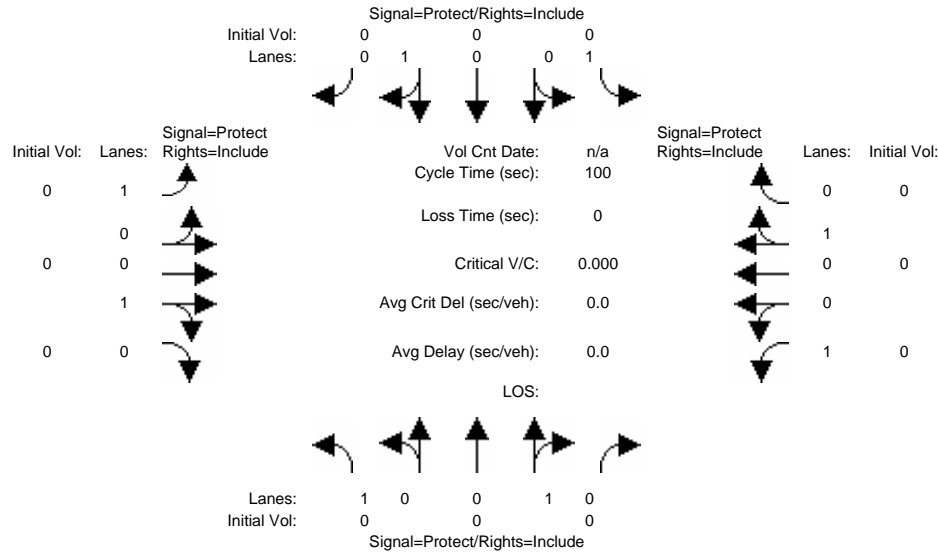
Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP PM

Intersection #20: Delta Shores Circle/Street C



Approach:	North Bound			South Bound			East Bound			West Bound														
Movement:	L	T	R	L	T	R	L	T	R	L	T	R												
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0												
Volume Module:																								
Base Vol:	0	0	0	0	0	0	0	0	0	0	0	0												
Growth Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Initial Bse:	0	0	0	0	0	0	0	0	0	0	0	0												
User Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
PHF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
PHF Volume:	0	0	0	0	0	0	0	0	0	0	0	0												
Reduced Vol:	0	0	0	0	0	0	0	0	0	0	0	0												
FCE Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
MLF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Final Vol.:	0	0	0	0	0	0	0	0	0	0	0	0												
Saturation Flow Module:																								
Sat/Lane:	0	0	0	0	0	0	0	0	0	0	0	0												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Final Sat.:	0	0	0	0	0	0	0	0	0	0	0	0												
Capacity Analysis Module:																								
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Crit Moves:																								
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
LOS by Move:																								
HCM2kAvgQ:	0	0	0	0	0	0	0	0	0	0	0	0												
HCM Ops Adjusted Lane Utilization Module:																								
Lanes:	0	0	1	0	0	0	1	0	0	0	0	1	0	0										
Lane Group:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx											
#LnsInGrps:	0	0	0	0	0	0	0	0	0	0	0	0	0											
HCM Ops Input Saturation Adj Module:																								
Lane Width:	12	12	12	12	12	12	12	12	12	12	12	12												
CrosswalkWid:	8	8	8	8	8	8	8	8	8	8	8	8												
% Hev Veh:	0	0	0	0	0	0	0	0	0	0	0	0												
Grade:	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%												
Parking/Hr:	No	No	No	No	No	No	No	No	No	No	No	No												
Bus Stp/Hr:	0	0	0	0	0	0	0	0	0	0	0	0												
Area Type:	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<
Cnft Ped/Hr:	0	0	0	0	0	0	0	0	0	0	0	0												
ExclusiveRT:	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include	Include												
% RT Prtct:	0	0	0	0	0	0	0	0	0	0	0	0												
HCM Ops f(lt) Adj Case Module:																								
f(lt) Case:	0	0	0	0	0	0	0	0	0	0	0	0												
HCM Ops Saturation Adj Module:																								
Ln Wid Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Hev Veh Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Grade Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Parking Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Bus Stp Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Area Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
RT Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
LT Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
PedBike Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
HCM Sat Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
User Sat Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
MLF Sat Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
FnL Sat Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												
Delay Adjustment Factor Module:																								
Coordinated:	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<		
Signal Type:	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<	<		
DelAdjPctr:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00												

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsSwalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsSwalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #21: Delta Shores Circle/Street A

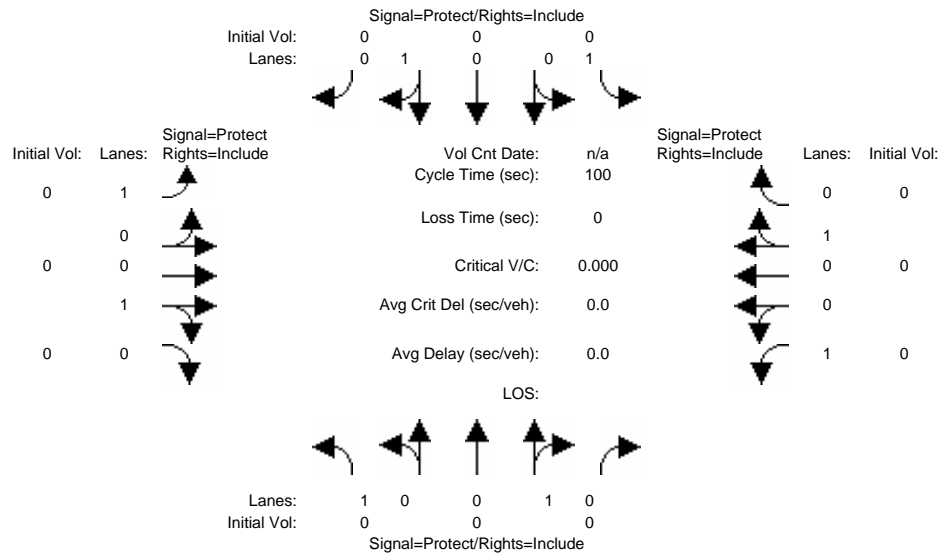


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #22: Delta Shores Circle North/24th Street

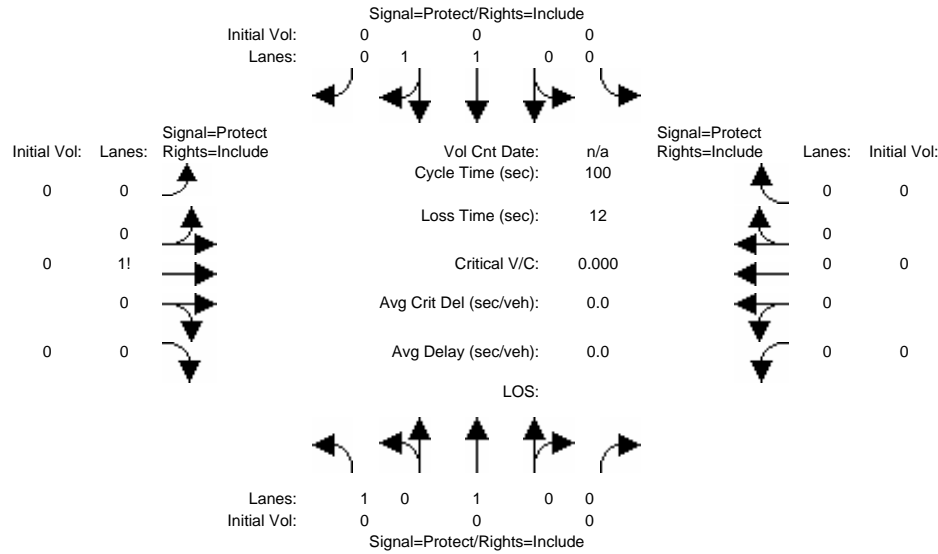


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFPIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #22: Delta Shores Circle North/24th Street

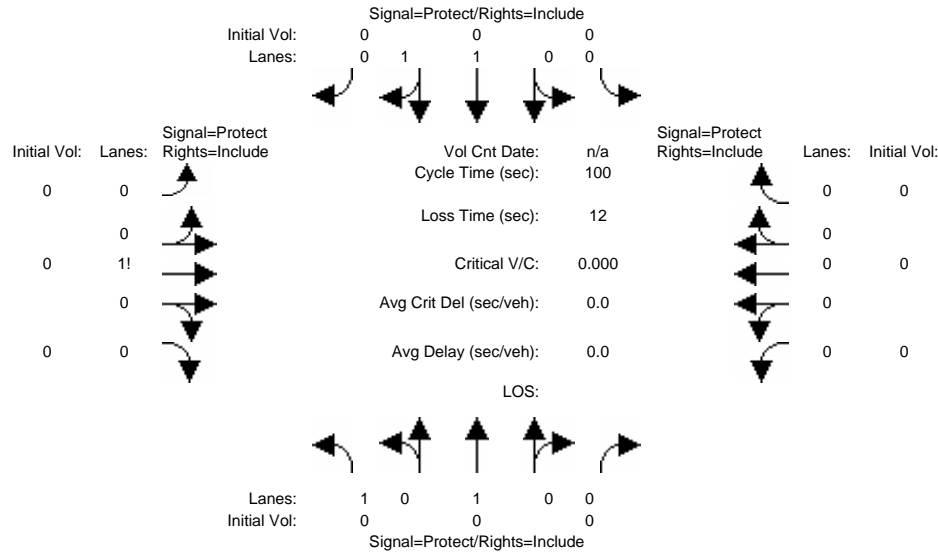


Table containing detailed traffic engineering data including Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsSwalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsSwalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

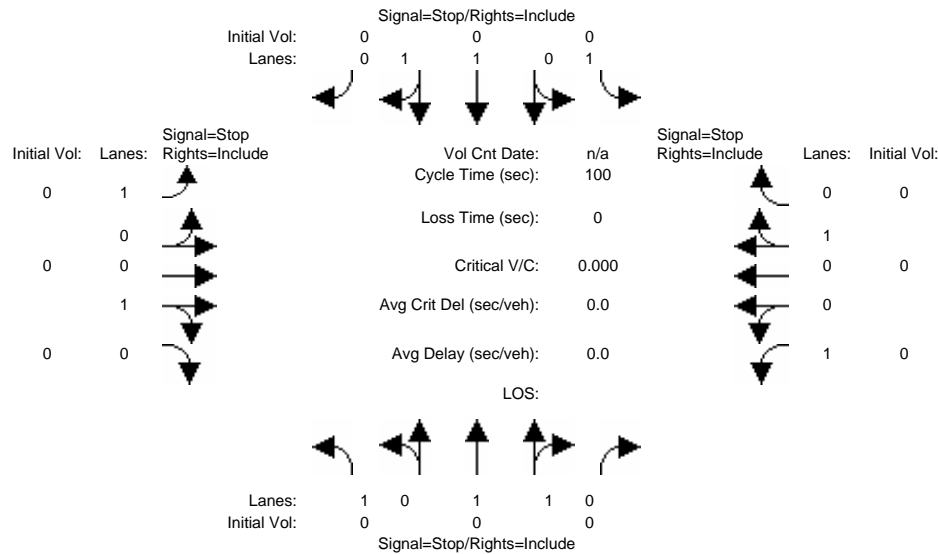
Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (alternative)
Baseline NP AM

Intersection #23: Street C/ 24th Street



Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	
Volume Module:													
Base Vol:	0	0	0	0	0	0	0	0	0	0	0	0	
Growth Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Bse:	0	0	0	0	0	0	0	0	0	0	0	0	
User Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
PHF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
PHF Volume:	0	0	0	0	0	0	0	0	0	0	0	0	
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	
Reduced Vol:	0	0	0	0	0	0	0	0	0	0	0	0	
PCE Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
MLF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Final Vol.:	0	0	0	0	0	0	0	0	0	0	0	0	
Saturation Flow Module:													
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Final Sat.:	0	0	0	0	0	0	0	0	0	0	0	0	
Capacity Analysis Module:													
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Crit Moves:													
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
LOS by Move:													
ApproachDel:	0.0	0.0			0.0			0.0			0.0		
Delay Adj:	0.00	0.00			0.00			0.00			0.00		
ApprAdjDel:	0.0	0.0			0.0			0.0			0.0		
LOS by Appr:													
AllWayAvgQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #23 Street C/ 24th Street

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1
Initial Vol:	0	0	0	0	0	0	0	0	0	0	0	0

Major Street Volume: 0
Minor Approach Volume: 0
Minor Approach Volume Threshold: +Inf

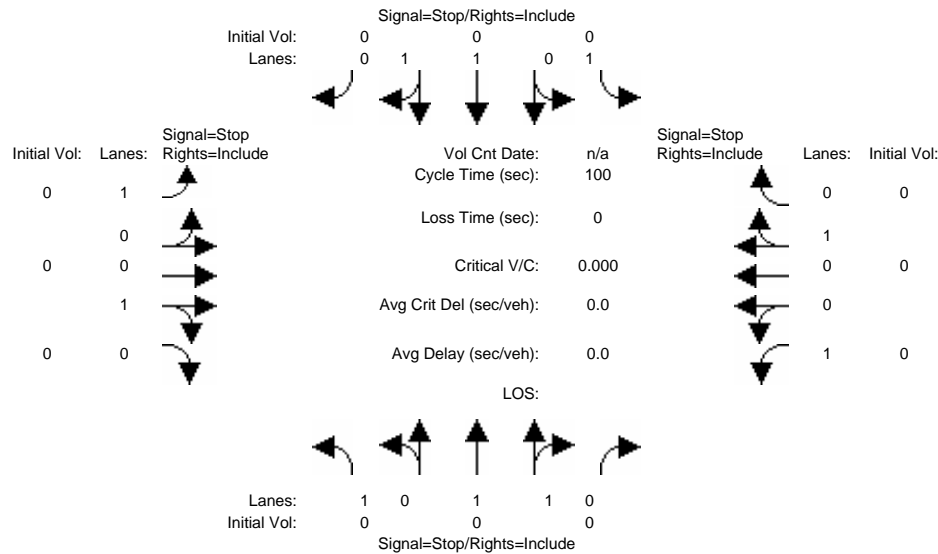
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (alternative)
Baseline NP PM

Intersection #23: Street C/ 24th Street



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Growth Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Initial Bse:	0	0	0	0	0	0	0	0	0	0	0	0
User Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PHF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PHF Volume:	0	0	0	0	0	0	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PCE Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MLF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Vol.:	0	0	0	0	0	0	0	0	0	0	0	0
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	0	0	0	0	0	0	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:												
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:												
ApproachDel:	0.0	0.0					0.0	0.0				
Delay Adj:	0.00	0.00					0.00	0.00				
ApprAdjDel:	0.0	0.0					0.0	0.0				
LOS by Appr:												
AllWayAvgQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #23 Street C/ 24th Street

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1
Initial Vol:	0	0	0	0	0	0	0	0	0	0	0	0

Major Street Volume: 0
Minor Approach Volume: 0
Minor Approach Volume Threshold: +Inf

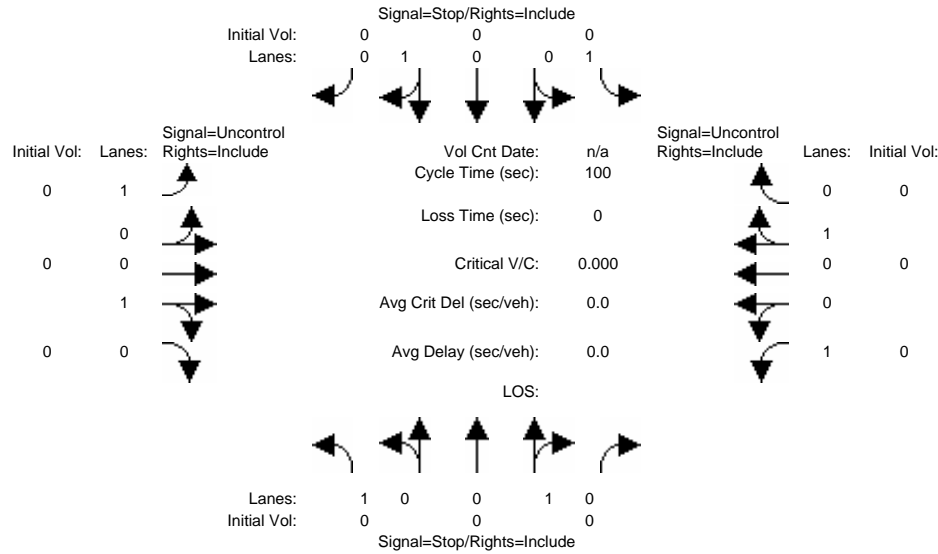
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline NP AM

Intersection #24: Street A/Street C



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Cnflct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
SharedQueue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	0 0 0	0 0 0	0 0 0	0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an

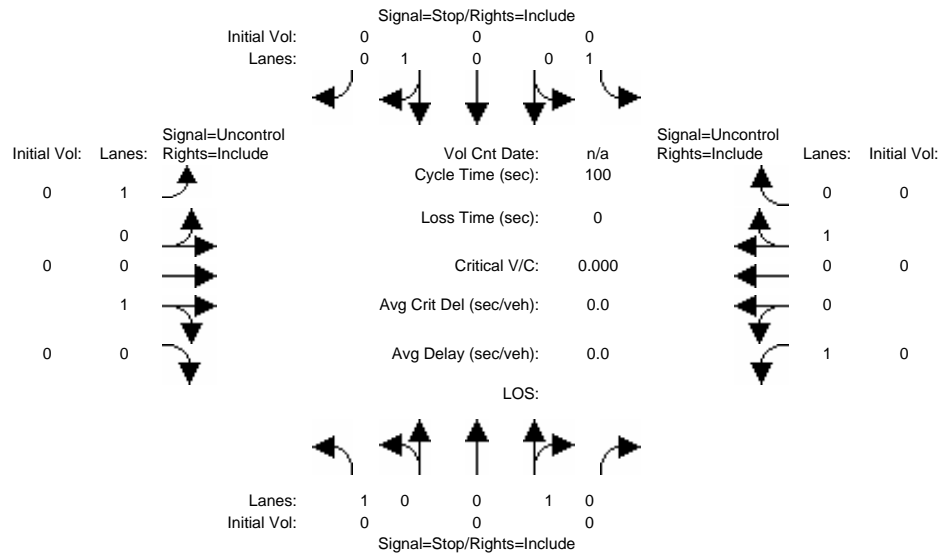
"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP PM

Intersection #24: Street A/Street C



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Shared Queue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP AM

Intersection #25: Street C/Street B

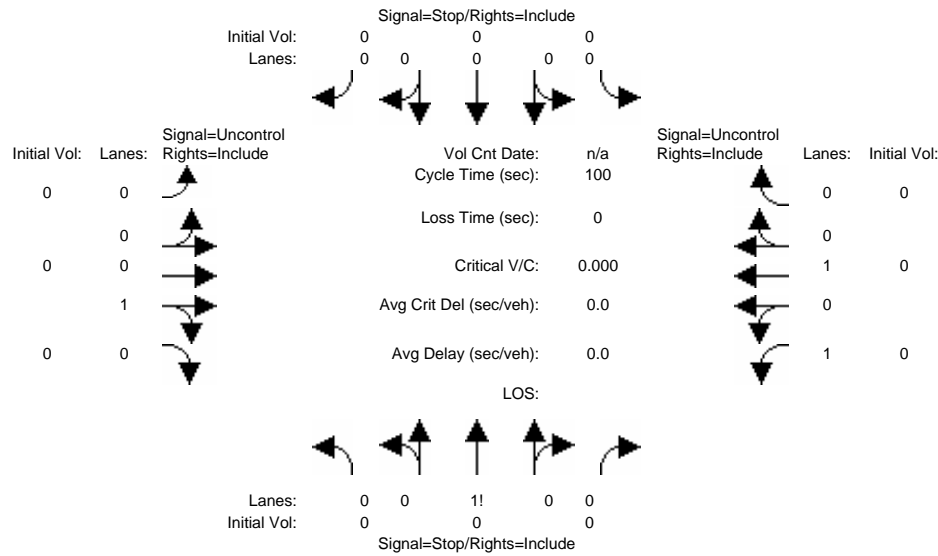


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Table titled 'Peak Hour Delay Signal Warrant Report' for Intersection #25 Street C/Street B. It shows Base Volume Alternative: Peak Hour Warrant NOT Met and includes a table for Control, Lanes, Initial Vol, and ApproachDel.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Table titled 'Peak Hour Volume Signal Warrant Report [Urban]' for Intersection #25 Street C/Street B. It shows Base Volume Alternative: Peak Hour Warrant NOT Met and includes a table for Control, Lanes, Initial Vol, and ApproachDel, along with Major Street Volume, Minor Approach Volume, and Minor Approach Volume Threshold.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

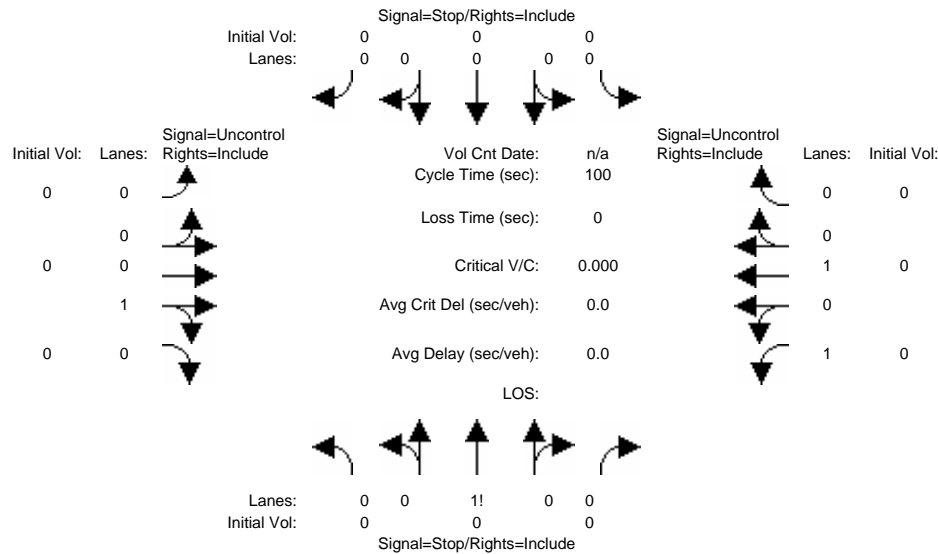
"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP PM

Intersection #25: Street C/Street B



Vol Cnt Date: n/a
Cycle Time (sec): 100
Loss Time (sec): 0
Critical V/C: 0.000
Avg Crit Del (sec/veh): 0.0
Avg Delay (sec/veh): 0.0

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Shared Queue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline NP AM

Intersection #26: Street D/Street E

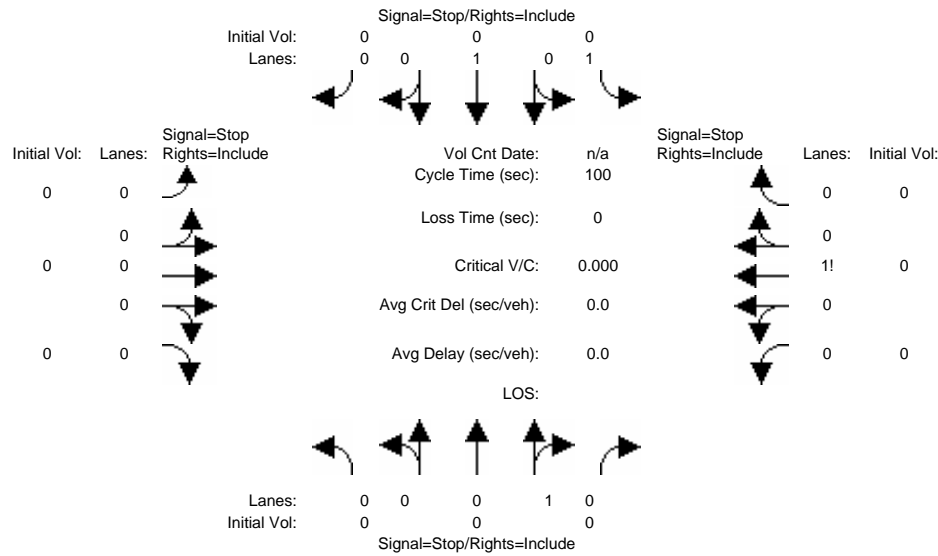


Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module. It contains numerical data for various traffic metrics.

Table titled 'Peak Hour Volume Signal Warrant Report [Urban]' for Intersection #26 Street D/Street E. It shows 'Base Volume Alternative: Peak Hour Warrant NOT Met' and provides lane control details (Stop Sign) and approach volumes.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline NP PM

Intersection #26: Street D/Street E

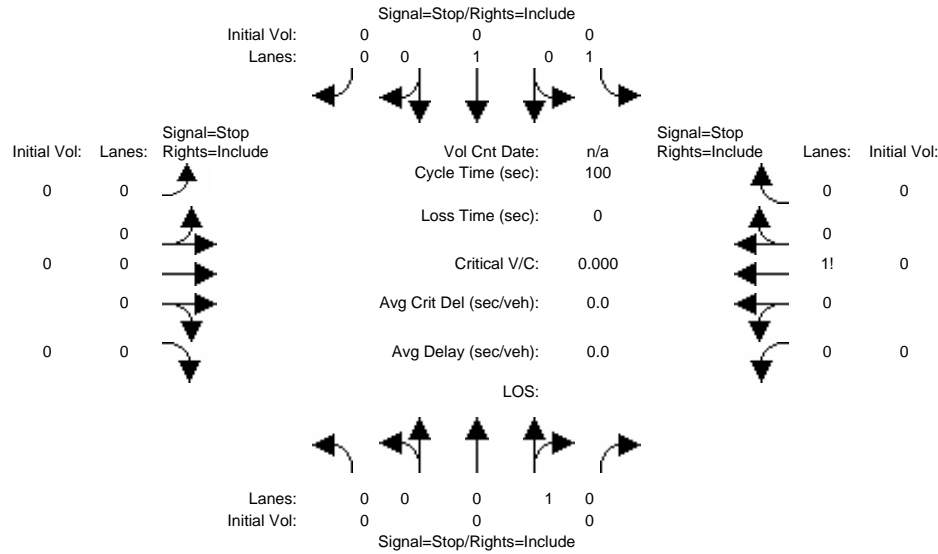


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Table for Peak Hour Volume Signal Warrant Report [Urban] showing Control, Lanes, and Initial Vol for each approach.

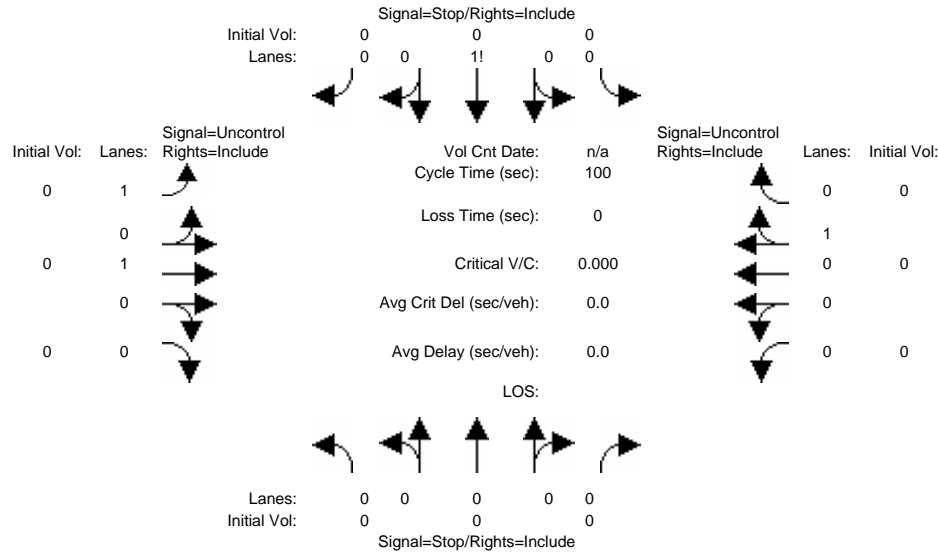
SIGNAL WARRANT DISCLAIMER
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The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP AM

Intersection #27: Street E/Street G



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Shared Queue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #27 Street E/Street G

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 0	0 0 1 0 0	1 0 1 0 0	0 0 0 1 0
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #27 Street E/Street G

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 0	0 0 1 0 0	1 0 1 0 0	0 0 0 1 0
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP PM

Intersection #27: Street E/Street G

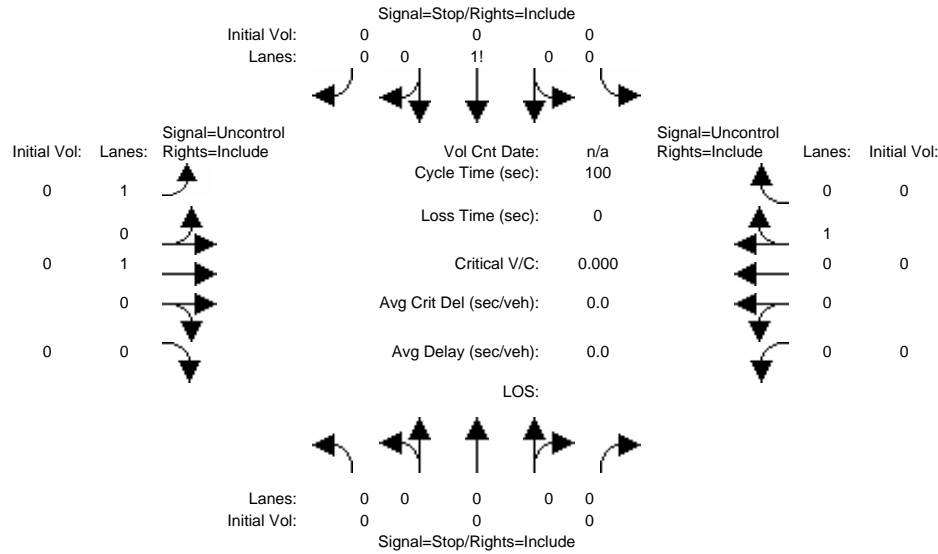


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Table titled 'Peak Hour Delay Signal Warrant Report' for Intersection #27 Street E/Street G. It shows Base Volume Alternative: Peak Hour Warrant NOT Met and includes a table for Control, Lanes, Initial Vol, and ApproachDel.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Table titled 'Peak Hour Volume Signal Warrant Report [Urban]' for Intersection #27 Street E/Street G. It shows Base Volume Alternative: Peak Hour Warrant NOT Met and includes a table for Control, Lanes, Initial Vol, and ApproachDel.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #28: Delta Shores Circle South/Street D South

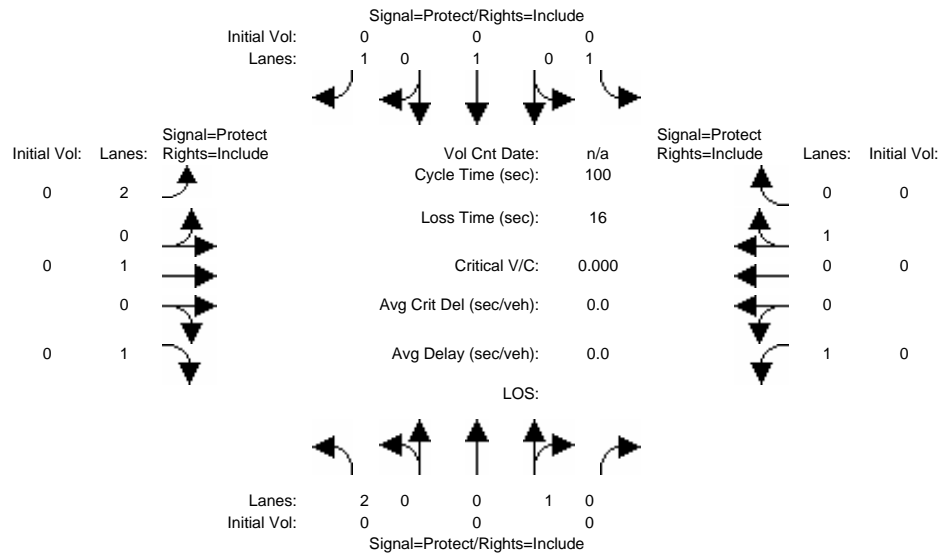


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrosswalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrosswalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFPIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #28: Delta Shores Circle South/Street D South

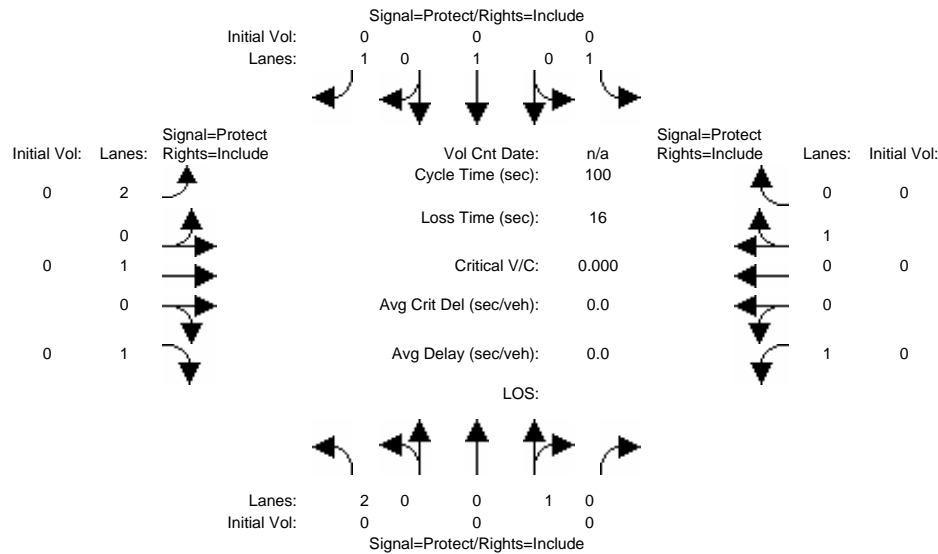


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
CrsSwalkWid:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CrsSwalkLen:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MinPedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedGrn:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PedVol:	0	0	0	0	0	0	0	0	0	0	0	0
PedFlowRate:	0	0	0	0	0	0	0	0	0	0	0	0
BikeVol:	0	0	0	0	0	0	0	0	0	0	0	0
BikeFlwRate:	0	0	0	0	0	0	0	0	0	0	0	0
PedOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
BikeOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedAfterOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
rOcc:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
TurnVehAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Prt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PrtA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Plt:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PltA:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PedBikeAdj:	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Level of Service Detailed Computation Report (HCM2000 Queue Method)
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q1:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
HCM2kQueue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
70thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
85thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
90thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
95thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
98thFactor:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30	MPH		30	MPH		30	MPH		30	MPH	
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

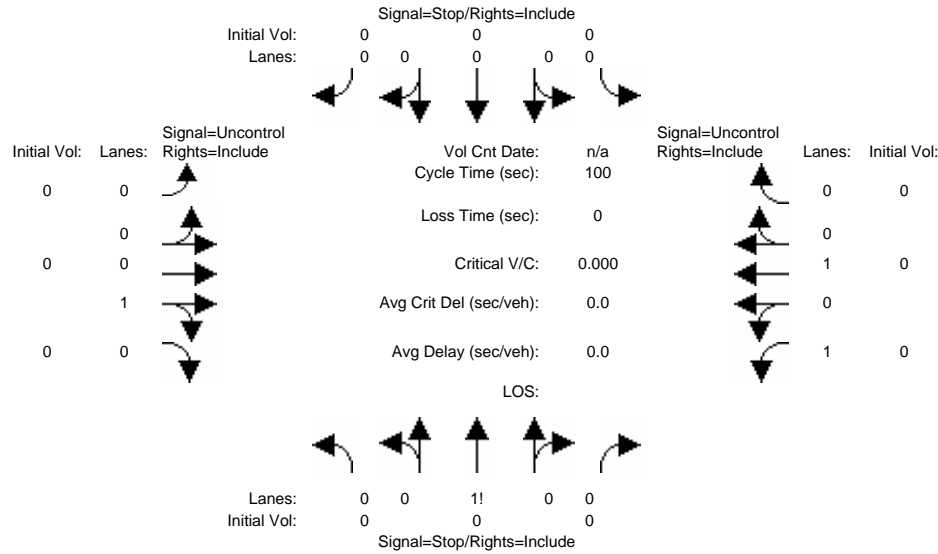
Name: year 2000 composite fleet
 Fuel Consumption: 0.000 pounds
 0.000 gallons
 Carbon Dioxide: 0.000 pounds
 Carbon Monoxide: 0.000 pounds
 Hydrocarbons: 0.000 pounds
 Nitrogen Oxides: 0.000 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP AM

Intersection #29: Street C/Stone-Boswell Access West



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Growth Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Initial Bse:	0	0	0	0	0	0	0	0	0	0	0	0
User Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PHF Adj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PHF Volume:	0	0	0	0	0	0	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	0	0	0	0	0	0	0	0	0	0	0	0
Critical Gap Module:												
Critical Gap:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
FollowUpTrim:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Capacity Module:												
Conflict Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Potent Cap.:	0	0	0	0	0	0	0	0	0	0	0	0
Move Cap.:	1	1	1	1	1	1	1	1	1	1	1	1
Volume/Cap.:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Level of Service Module:												
2Way95thQ:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Del:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:												
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	0	0	0	0	0	0	0	0	0	0	0	0
Shared Queue:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Shrd ConDel:	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Shared LOS:												
ApproachDel:	0.0	0.0					0.0	0.0				
ApproachLOS:												

Peak Hour Delay Signal Warrant Report
 Intersection #29 Street C/Stone-Boswell Access West
 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	0
Initial Vol:	0	0	0	0	0	0	0	0	0	0	0	0
ApproachDel:	0.0	0.0					0.0	0.0				

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]
 Intersection #29 Street C/Stone-Boswell Access West
 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	0
Initial Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Major Street Volume:	0											
Minor Approach Volume:	0											
Minor Approach Volume Threshold:	+Inf											

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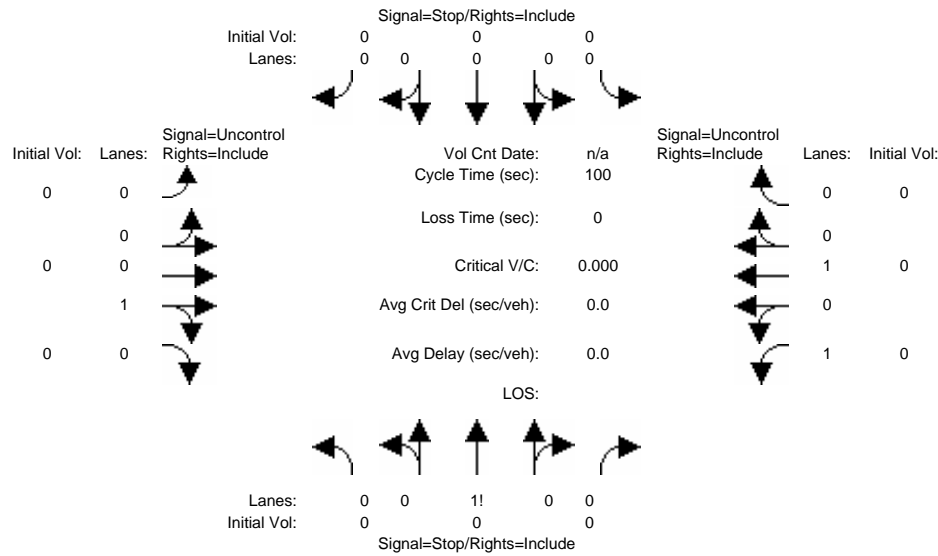
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline NP PM

Intersection #29: Street C/Stone-Boswell Access West



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Shared Queue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0	0 0 0	0 0 0	0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

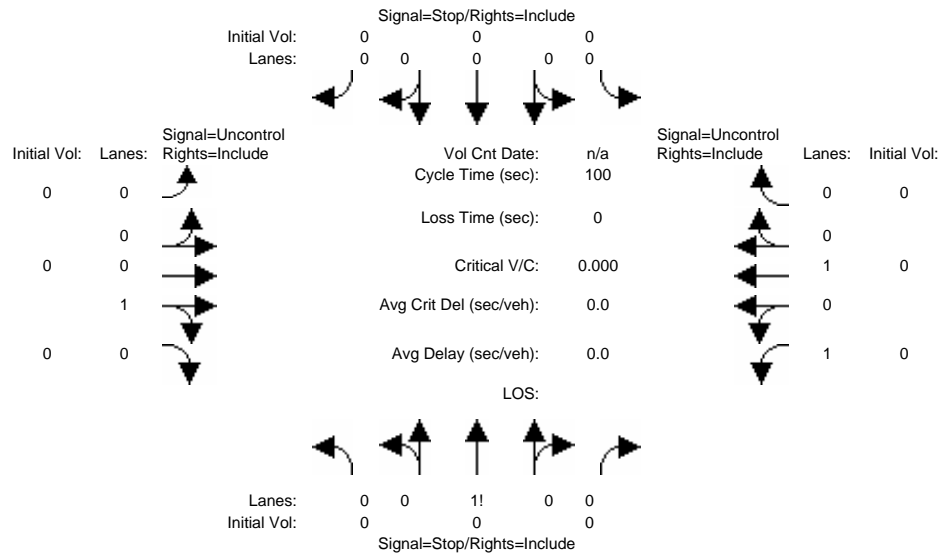
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline NP AM

Intersection #30: Street C/Stone-Boswell Access East



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Shared Queue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

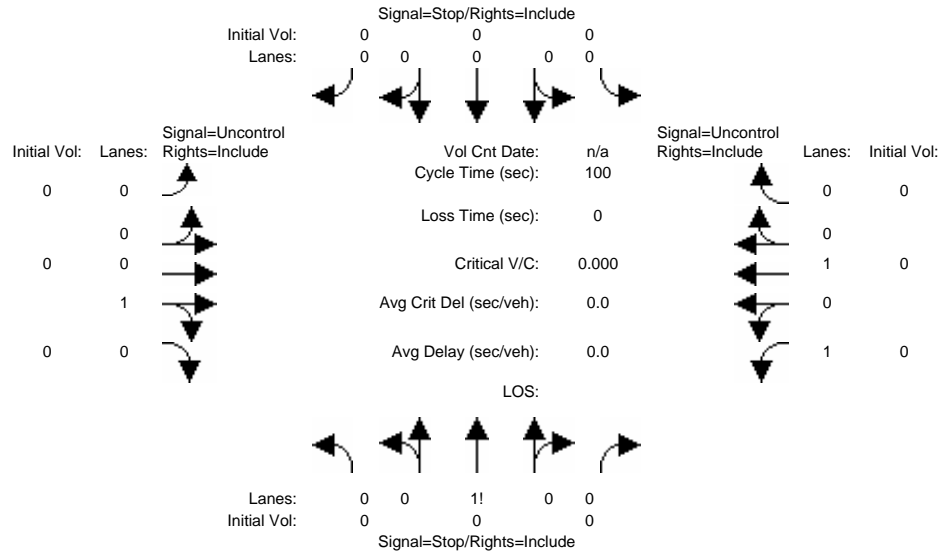
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline NP PM

Intersection #30: Street C/Stone-Boswell Access East



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Growth Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Initial Bse:	0 0 0	0 0 0	0 0 0	0 0 0
User Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Adj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
PHF Volume:	0 0 0	0 0 0	0 0 0	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Vol.:	0 0 0	0 0 0	0 0 0	0 0 0
Critical Gap Module:				
Critical Gap:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
FollowUpTrim:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Capacity Module:				
Conflict Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Potent Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
Move Cap.:	1 1 1	1 1 1	1 1 1	1 1 1
Volume/Cap.:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
Level of Service Module:				
2Way95thQ:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Control Del:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
LOS by Move:				
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	0 0 0	0 0 0	0 0 0	0 0 0
SharedQueue:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
Shrd ConDel:	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0	1.0 1.0 1.0
Shared LOS:				
ApproachDel:	0.0	0.0	0.0	0.0
ApproachLOS:				

Peak Hour Delay Signal Warrant Report

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
Major Street Volume:	0			
Minor Approach Volume:	0			
Minor Approach Volume Threshold:	+Inf			

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline NP AM

Intersection #31: Street C/Detroit Boulevard

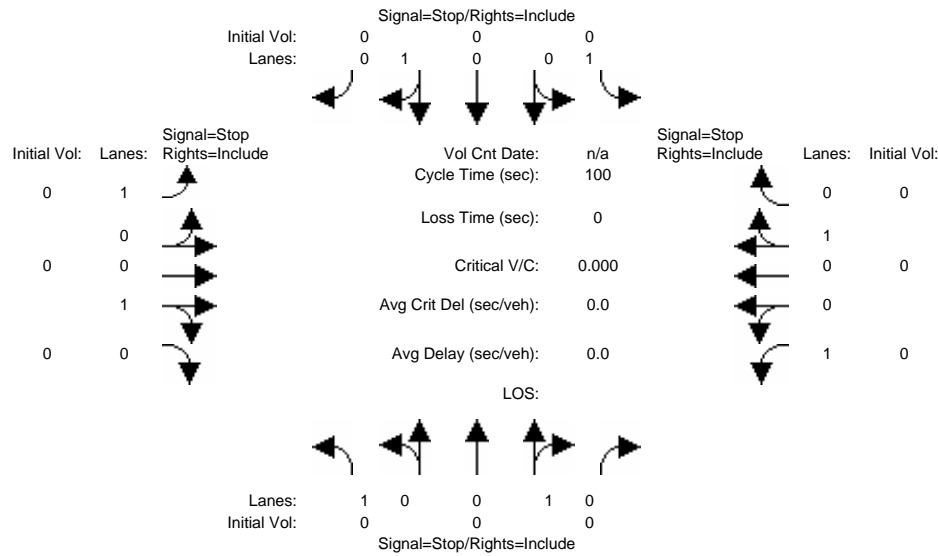


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Table for Peak Hour Volume Signal Warrant Report [Urban] showing Control, Lanes, and Initial Vol for each approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline NP PM

Intersection #31: Street C/Detroit Boulevard

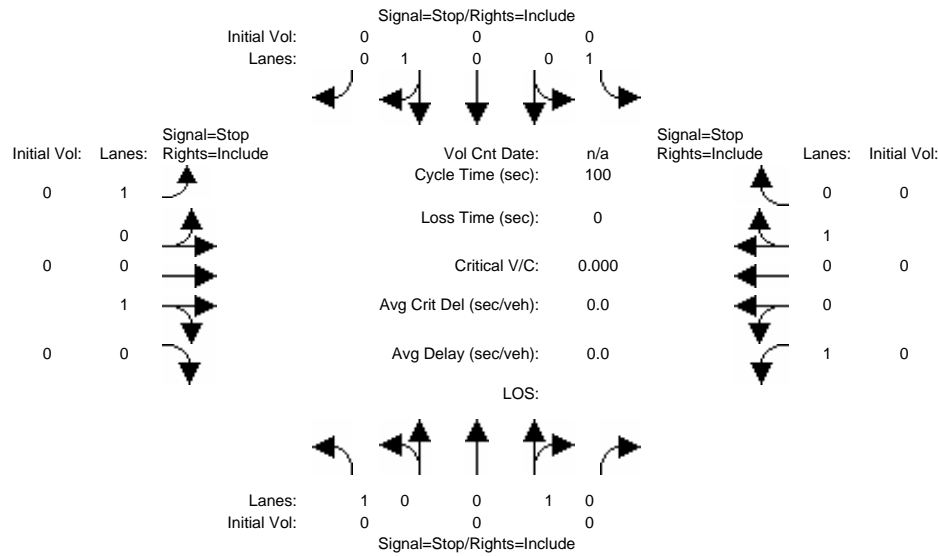


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Table for Peak Hour Volume Signal Warrant Report [Urban]. Includes columns for Approach, Movement, Control, Lanes, and Initial Vol. Rows show North, South, East, and West Bound movements.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP AM

Intersection #32:

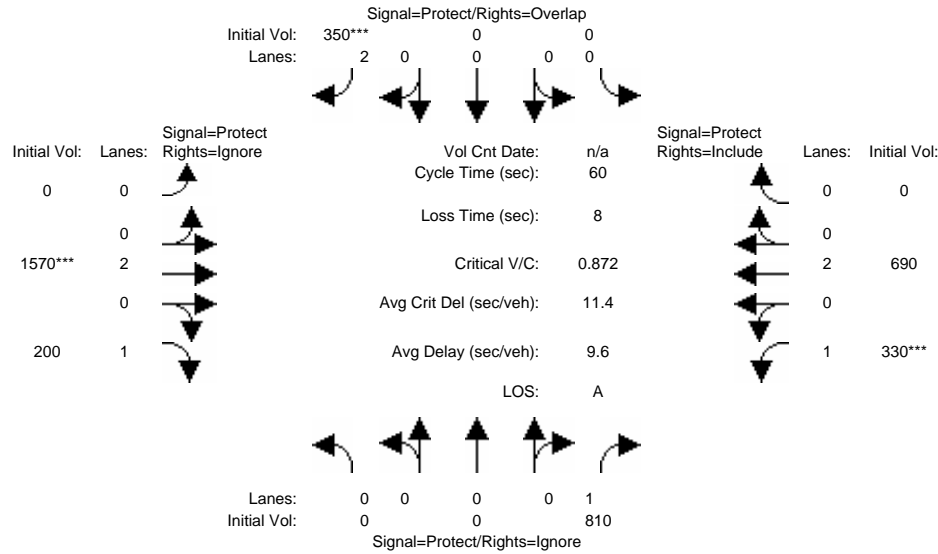


Table containing traffic engineering data: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	3.1	0.0	5.9	0.0	5.1	2.3	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	2.3	0.0	1.4	0.0	2.8	0.4	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	5.4	0.0	7.3	0.0	8.0	2.6	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.19	1.20	1.18	1.20	1.18	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	6.4	0.0	8.7	0.0	9.4	3.2	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.55	1.60	1.54	1.60	1.53	1.57	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	8.4	0.0	11.2	0.0	12.2	4.2	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.71	1.80	1.68	1.80	1.67	1.75	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	9.2	0.0	12.3	0.0	13.3	4.6	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.94	2.10	1.90	2.10	1.89	2.02	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	10.5	0.0	13.9	0.0	15.0	5.3	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.36	2.70	2.27	2.70	2.24	2.52	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	12.7	0.0	16.6	0.0	17.8	6.6	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	83.1	0.0	169	0.0	77.1	64.3	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 54.013 pounds
 8.750 gallons
 Carbon Dioxide: 168.520 pounds
 Carbon Monoxide: 12.005 pounds
 Hydrocarbons: 1.821 pounds
 Nitrogen Oxides: 0.552 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 49.002 pounds
 7.938 gallons
 Carbon Dioxide: 152.885 pounds
 Carbon Monoxide: 11.620 pounds
 Hydrocarbons: 1.741 pounds
 Nitrogen Oxides: 0.481 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #32:

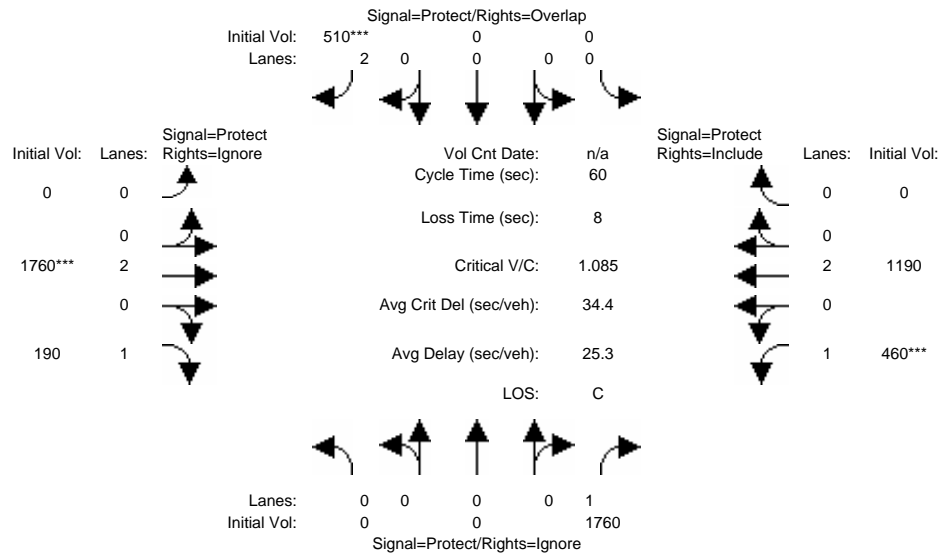


Table containing detailed traffic engineering data including: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	4.8	0.0	7.4	0.0	7.7	4.7	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	7.5	0.0	1.8	0.0	10.1	0.9	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	12.3	0.0	9.2	0.0	17.8	5.6	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.17	1.20	1.18	1.20	1.16	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	14.4	0.0	10.9	0.0	20.7	6.7	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.50	1.60	1.52	1.60	1.47	1.55	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	18.4	0.0	14.0	0.0	26.1	8.7	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.62	1.80	1.65	1.80	1.56	1.70	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	19.9	0.0	15.2	0.0	27.8	9.6	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.80	2.10	1.86	2.10	1.72	1.94	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	22.1	0.0	17.1	0.0	30.6	10.9	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.09	2.70	2.19	2.70	1.95	2.35	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	25.7	0.0	20.2	0.0	34.8	13.2	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	129.5	0.0	210	0.0	118.1	134	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 115.217 pounds
 18.665 gallons
 Carbon Dioxide: 359.476 pounds
 Carbon Monoxide: 28.260 pounds
 Hydrocarbons: 5.222 pounds
 Nitrogen Oxides: 0.969 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 108.409 pounds
 17.562 gallons
 Carbon Dioxide: 338.236 pounds
 Carbon Monoxide: 27.734 pounds
 Hydrocarbons: 5.114 pounds
 Nitrogen Oxides: 0.871 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP AM

Intersection #33:

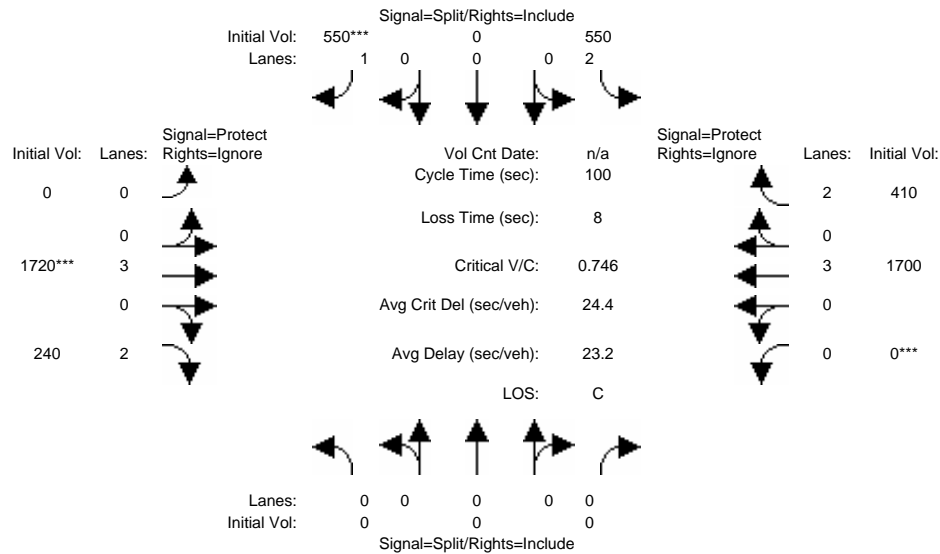


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.47	0.00	0.47	0.00	0.45	0.00	0.00	0.45	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	5.0	0.0	12.5	0.0	14.4	0.0	0.0	14.2	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.5	0.0	2.6	0.0	2.7	0.0	0.0	2.6	0.0
HCM2KQueue:	0.0	0.0	0.0	5.5	0.0	15.1	0.0	17.1	0.0	0.0	16.8	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.17	1.20	1.17	1.20	1.20	1.17	1.20
70thHCM2kQ:	0.0	0.0	0.0	6.6	0.0	17.7	0.0	19.9	0.0	0.0	19.5	0.0
85thFactor:	1.60	1.60	1.60	1.55	1.60	1.48	1.60	1.47	1.60	1.60	1.47	1.60
85thHCM2kQ:	0.0	0.0	0.0	8.6	0.0	22.4	0.0	25.1	0.0	0.0	24.7	0.0
90thFactor:	1.80	1.80	1.80	1.70	1.80	1.59	1.80	1.57	1.80	1.80	1.57	1.80
90thHCM2kQ:	0.0	0.0	0.0	9.4	0.0	24.0	0.0	26.9	0.0	0.0	26.4	0.0
95thFactor:	2.10	2.10	2.10	1.94	2.10	1.76	2.10	1.73	2.10	2.10	1.74	2.10
95thHCM2kQ:	0.0	0.0	0.0	10.7	0.0	26.6	0.0	29.6	0.0	0.0	29.1	0.0
98thFactor:	2.70	2.70	2.70	2.35	2.70	2.01	2.70	1.97	2.70	2.70	1.98	2.70
98thHCM2kQ:	0.0	0.0	0.0	13.0	0.0	30.5	0.0	33.7	0.0	0.0	33.1	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	87.4	0.0	112.5	0.0	355	0.0	0.0	349	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 129.444 pounds
 20.970 gallons
 Carbon Dioxide: 403.864 pounds
 Carbon Monoxide: 31.478 pounds
 Hydrocarbons: 5.652 pounds
 Nitrogen Oxides: 1.187 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 121.180 pounds
 19.631 gallons
 Carbon Dioxide: 378.080 pounds
 Carbon Monoxide: 30.829 pounds
 Hydrocarbons: 5.521 pounds
 Nitrogen Oxides: 1.057 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline NP PM

Intersection #33:

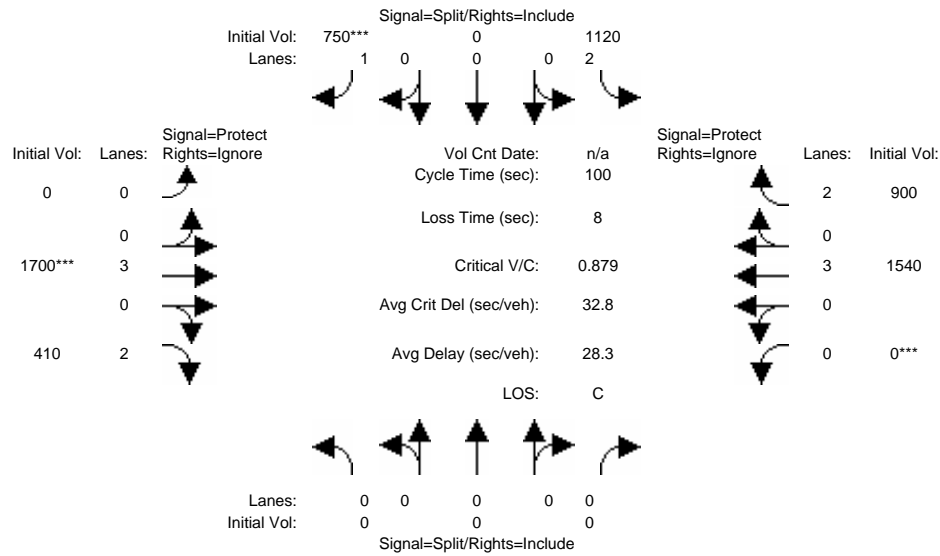


Table with columns for Approach (North, South, East, West) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.54	0.00	0.54	0.00	0.38	0.00	0.00	0.38	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	11.0	0.0	18.2	0.0	16.1	0.0	0.0	13.9	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.5	0.0	5.2	0.0	5.0	0.0	0.0	3.3	0.0
HCM2KQueue:	0.0	0.0	0.0	12.5	0.0	23.4	0.0	21.1	0.0	0.0	17.2	0.0
70thFactor:	1.20	1.20	1.20	1.17	1.20	1.16	1.20	1.16	1.20	1.20	1.17	1.20
70thHCM2kQ:	0.0	0.0	0.0	14.6	0.0	27.1	0.0	24.4	0.0	0.0	20.1	0.0
85thFactor:	1.60	1.60	1.60	1.50	1.60	1.44	1.60	1.45	1.60	1.60	1.47	1.60
85thHCM2kQ:	0.0	0.0	0.0	18.7	0.0	33.7	0.0	30.5	0.0	0.0	25.3	0.0
90thFactor:	1.80	1.80	1.80	1.61	1.80	1.52	1.80	1.54	1.80	1.80	1.57	1.80
90thHCM2kQ:	0.0	0.0	0.0	20.1	0.0	35.7	0.0	32.4	0.0	0.0	27.0	0.0
95thFactor:	2.10	2.10	2.10	1.80	2.10	1.66	2.10	1.69	2.10	2.10	1.73	2.10
95thHCM2kQ:	0.0	0.0	0.0	22.4	0.0	38.9	0.0	35.5	0.0	0.0	29.8	0.0
98thFactor:	2.70	2.70	2.70	2.08	2.70	1.87	2.70	1.90	2.70	2.70	1.97	2.70
98thHCM2kQ:	0.0	0.0	0.0	25.9	0.0	43.7	0.0	40.0	0.0	0.0	33.9	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	191.5	0.0	164.2	0.0	395	0.0	0.0	342	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 165.043 pounds
 26.737 gallons
 Carbon Dioxide: 514.936 pounds
 Carbon Monoxide: 40.865 pounds
 Hydrocarbons: 7.572 pounds
 Nitrogen Oxides: 1.460 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 155.566 pounds
 25.202 gallons
 Carbon Dioxide: 485.367 pounds
 Carbon Monoxide: 40.118 pounds
 Hydrocarbons: 7.422 pounds
 Nitrogen Oxides: 1.308 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline NP AM

Intersection #34:

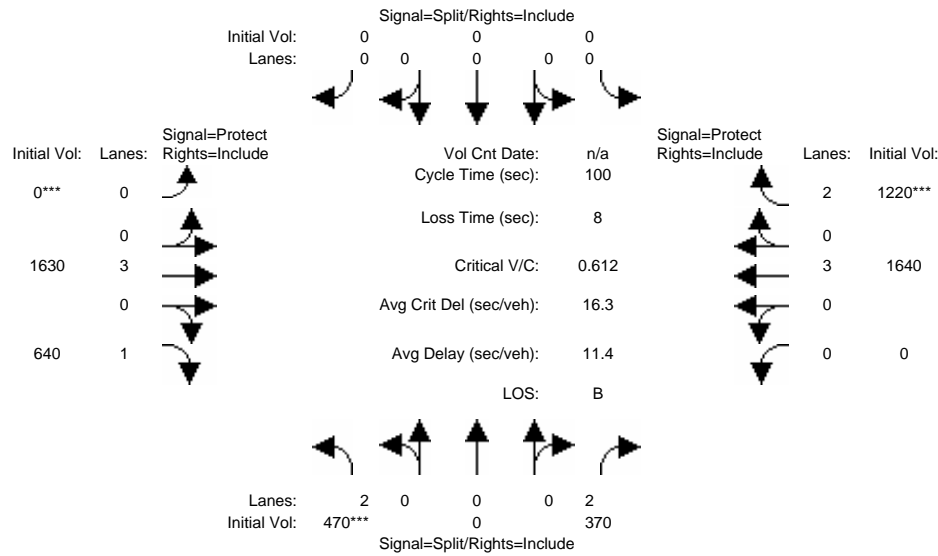


Table containing traffic engineering data: Approach: North Bound, South Bound, East Bound, West Bound; Movement: L, T, R; Volume Module; Saturation Flow Module; Capacity Analysis Module; HCM Ops Adjusted Lane Utilization Module; HCM Ops Input Saturation Adj Module; HCM Ops f(lt) Adj Case Module; HCM Ops Saturation Adj Module; Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.22	0.00	0.22	0.00	0.00	0.00	0.00	0.70	0.70	0.00	0.70	0.70
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.1	0.0	5.2	0.0	0.0	0.0	0.0	7.2	8.8	0.0	7.3	10.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00
Q2:	1.5	0.0	1.4	0.0	0.0	0.0	0.0	0.8	1.3	0.0	0.8	1.5
HCM2KQueue:	7.5	0.0	6.6	0.0	0.0	0.0	0.0	8.0	10.1	0.0	8.1	11.6
70thFactor:	1.18	1.20	1.18	1.20	1.20	1.20	1.20	1.18	1.18	1.20	1.18	1.17
70thHCM2kQ:	8.9	0.0	7.8	0.0	0.0	0.0	0.0	9.5	11.9	0.0	9.6	13.7
85thFactor:	1.53	1.60	1.54	1.60	1.60	1.60	1.60	1.53	1.51	1.60	1.53	1.50
85thHCM2kQ:	11.6	0.0	10.2	0.0	0.0	0.0	0.0	12.3	15.3	0.0	12.4	17.5
90thFactor:	1.67	1.80	1.69	1.80	1.80	1.80	1.80	1.67	1.64	1.80	1.67	1.62
90thHCM2kQ:	12.6	0.0	11.1	0.0	0.0	0.0	0.0	13.4	16.6	0.0	13.5	18.9
95thFactor:	1.89	2.10	1.92	2.10	2.10	2.10	2.10	1.88	1.84	2.10	1.88	1.81
95thHCM2kQ:	14.3	0.0	12.6	0.0	0.0	0.0	0.0	15.1	18.6	0.0	15.3	21.1
98thFactor:	2.26	2.70	2.30	2.70	2.70	2.70	2.70	2.24	2.16	2.70	2.24	2.11
98thHCM2kQ:	17.0	0.0	15.2	0.0	0.0	0.0	0.0	18.0	21.8	0.0	18.1	24.5

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	106.0	0.0	83.0	0.0	0.0	0.0	0.0	178	79.3	0.0	179	159.9

Name: year 1995 composite fleet
 Fuel Consumption: 116.932 pounds
 18.943 gallons
 Carbon Dioxide: 364.829 pounds
 Carbon Monoxide: 26.452 pounds
 Hydrocarbons: 4.184 pounds
 Nitrogen Oxides: 1.150 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 106.783 pounds
 17.299 gallons
 Carbon Dioxide: 333.164 pounds
 Carbon Monoxide: 25.671 pounds
 Hydrocarbons: 4.022 pounds
 Nitrogen Oxides: 1.007 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.17	0.00	0.17	0.00	0.00	0.00	0.00	0.75	0.75	0.00	0.75	0.75
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.1	0.0	6.3	0.0	0.0	0.0	0.0	8.4	18.9	0.0	8.4	2.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00
Q2:	2.4	0.0	3.3	0.0	0.0	0.0	0.0	1.1	4.4	0.0	1.1	0.3
HCM2KQueue:	8.5	0.0	9.5	0.0	0.0	0.0	0.0	9.5	23.3	0.0	9.5	2.8
70thFactor:	1.18	1.20	1.18	1.20	1.20	1.20	1.20	1.18	1.16	1.20	1.18	1.19
70thHCM2kQ:	10.0	0.0	11.2	0.0	0.0	0.0	0.0	11.2	26.9	0.0	11.2	3.3
85thFactor:	1.53	1.60	1.52	1.60	1.60	1.60	1.60	1.52	1.44	1.60	1.52	1.57
85thHCM2kQ:	12.9	0.0	14.4	0.0	0.0	0.0	0.0	14.4	33.4	0.0	14.4	4.4
90thFactor:	1.66	1.80	1.65	1.80	1.80	1.80	1.80	1.65	1.53	1.80	1.65	1.75
90thHCM2kQ:	14.1	0.0	15.7	0.0	0.0	0.0	0.0	15.7	35.5	0.0	15.7	4.9
95thFactor:	1.87	2.10	1.85	2.10	2.10	2.10	2.10	1.85	1.66	2.10	1.85	2.01
95thHCM2kQ:	15.9	0.0	17.6	0.0	0.0	0.0	0.0	17.6	38.7	0.0	17.6	5.6
98thFactor:	2.22	2.70	2.18	2.70	2.70	2.70	2.70	2.18	1.87	2.70	2.18	2.51
98thHCM2kQ:	18.8	0.0	20.7	0.0	0.0	0.0	0.0	20.7	43.4	0.0	20.7	7.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	106.8	0.0	99.1	0.0	0.0	0.0	0.0	207	170.1	0.0	207	39.2

Name: year 1995 composite fleet
 Fuel Consumption: 129.196 pounds
 20.930 gallons
 Carbon Dioxide: 403.091 pounds
 Carbon Monoxide: 29.458 pounds
 Hydrocarbons: 4.746 pounds
 Nitrogen Oxides: 1.246 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 118.339 pounds
 19.171 gallons
 Carbon Dioxide: 369.217 pounds
 Carbon Monoxide: 28.624 pounds
 Hydrocarbons: 4.572 pounds
 Nitrogen Oxides: 1.095 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #1: Pocket Road/I-5 SB Ramps

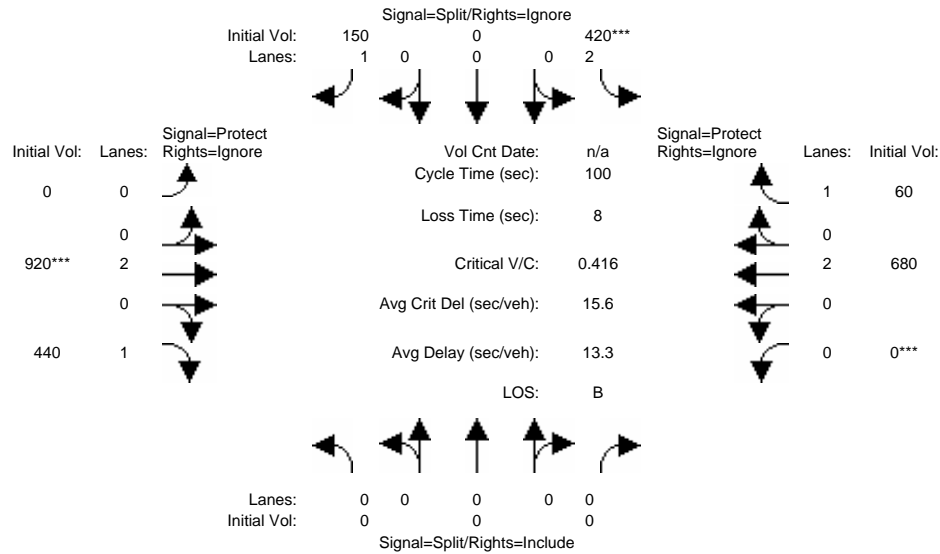


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.29	0.00	0.00	0.00	0.63	0.00	0.00	0.63	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	4.8	0.0	0.0	0.0	6.8	0.0	0.0	4.6	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.7	0.0	0.0	0.0	0.7	0.0	0.0	0.4	0.0
HCM2KQueue:	0.0	0.0	0.0	5.5	0.0	0.0	0.0	7.5	0.0	0.0	5.0	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.20	1.20	1.18	1.20	1.20	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	6.6	0.0	0.0	0.0	8.9	0.0	0.0	6.0	0.0
85thFactor:	1.60	1.60	1.60	1.55	1.60	1.60	1.60	1.53	1.60	1.60	1.55	1.60
85thHCM2kQ:	0.0	0.0	0.0	8.6	0.0	0.0	0.0	11.5	0.0	0.0	7.8	0.0
90thFactor:	1.80	1.80	1.80	1.70	1.80	1.80	1.80	1.67	1.80	1.80	1.71	1.80
90thHCM2kQ:	0.0	0.0	0.0	9.4	0.0	0.0	0.0	12.6	0.0	0.0	8.6	0.0
95thFactor:	2.10	2.10	2.10	1.94	2.10	2.10	2.10	1.90	2.10	2.10	1.95	2.10
95thHCM2kQ:	0.0	0.0	0.0	10.7	0.0	0.0	0.0	14.2	0.0	0.0	9.9	0.0
98thFactor:	2.70	2.70	2.70	2.35	2.70	2.70	2.70	2.26	2.70	2.70	2.38	2.70
98thHCM2kQ:	0.0	0.0	0.0	13.0	0.0	0.0	0.0	17.0	0.0	0.0	12.0	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	84.4	0.0	0.0	0.0	116	0.0	0.0	78.8	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 42.393 pounds
 6.868 gallons
 Carbon Dioxide: 132.265 pounds
 Carbon Monoxide: 9.746 pounds
 Hydrocarbons: 1.592 pounds
 Nitrogen Oxides: 0.408 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 38.933 pounds
 6.307 gallons
 Carbon Dioxide: 121.470 pounds
 Carbon Monoxide: 9.480 pounds
 Hydrocarbons: 1.537 pounds
 Nitrogen Oxides: 0.359 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #1: Pocket Road/I-5 SB Ramps

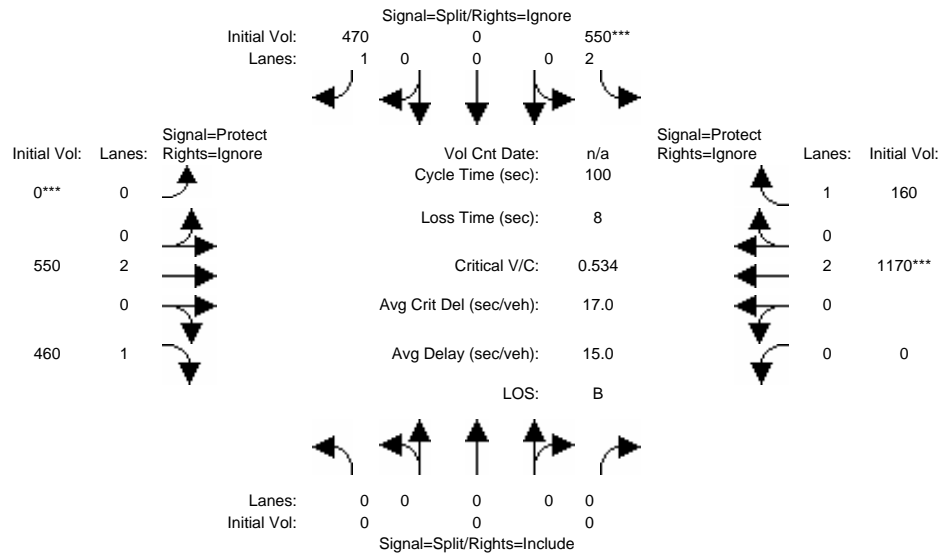


Table with columns for Approach, Movement, and various traffic engineering metrics such as Volume Module, Saturation Flow Module, Capacity Analysis Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.30	0.00	0.00	0.00	0.62	0.00	0.00	0.62	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	6.6	0.0	0.0	0.0	3.6	0.0	0.0	9.7	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.1	0.0	0.0	0.0	0.3	0.0	0.0	1.1	0.0
HCM2KQueue:	0.0	0.0	0.0	7.7	0.0	0.0	0.0	4.0	0.0	0.0	10.8	0.0
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.19	1.20	1.20	1.18	1.20
70thHCM2kQ:	0.0	0.0	0.0	9.1	0.0	0.0	0.0	4.7	0.0	0.0	12.8	0.0
85thFactor:	1.60	1.60	1.60	1.53	1.60	1.60	1.60	1.56	1.60	1.60	1.51	1.60
85thHCM2kQ:	0.0	0.0	0.0	11.7	0.0	0.0	0.0	6.2	0.0	0.0	16.4	0.0
90thFactor:	1.80	1.80	1.80	1.67	1.80	1.80	1.80	1.73	1.80	1.80	1.63	1.80
90thHCM2kQ:	0.0	0.0	0.0	12.8	0.0	0.0	0.0	6.8	0.0	0.0	17.7	0.0
95thFactor:	2.10	2.10	2.10	1.89	2.10	2.10	2.10	1.98	2.10	2.10	1.83	2.10
95thHCM2kQ:	0.0	0.0	0.0	14.5	0.0	0.0	0.0	7.8	0.0	0.0	19.8	0.0
98thFactor:	2.70	2.70	2.70	2.25	2.70	2.70	2.70	2.44	2.70	2.70	2.13	2.70
98thHCM2kQ:	0.0	0.0	0.0	17.3	0.0	0.0	0.0	9.6	0.0	0.0	23.1	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	114.6	0.0	0.0	0.0	61.9	0.0	0.0	166	0.0

Name: year 1995 composite fleet
Fuel Consumption: 50.677 pounds
8.210 gallons
Carbon Dioxide: 158.113 pounds
Carbon Monoxide: 11.799 pounds
Hydrocarbons: 1.970 pounds
Nitrogen Oxides: 0.484 pounds

Name: year 2000 composite fleet
Fuel Consumption: 46.736 pounds
7.571 gallons
Carbon Dioxide: 145.815 pounds
Carbon Monoxide: 11.494 pounds
Hydrocarbons: 1.907 pounds
Nitrogen Oxides: 0.427 pounds

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #2: Meadowview Road/I-5 NB Ramps

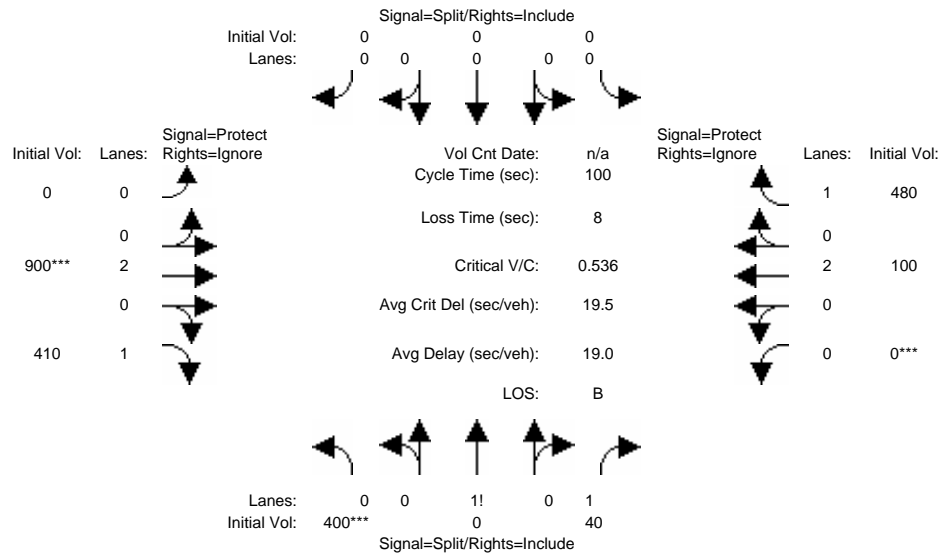


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.45	0.00	0.45	0.00	0.00	0.00	0.00	0.47	0.00	0.00	0.47	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	8.5	0.0	0.6	0.0	0.0	0.0	0.0	9.3	0.0	0.0	0.8	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	1.1	0.0	0.1	0.0	0.0	0.0	0.0	1.1	0.0	0.0	0.1	0.0
HCM2KQueue:	9.6	0.0	0.7	0.0	0.0	0.0	0.0	10.4	0.0	0.0	0.9	0.0
70thFactor:	1.18	1.20	1.20	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.20
70thHCM2kQ:	11.3	0.0	0.8	0.0	0.0	0.0	0.0	12.3	0.0	0.0	1.0	0.0
85thFactor:	1.52	1.60	1.59	1.60	1.60	1.60	1.60	1.51	1.60	1.60	1.59	1.60
85thHCM2kQ:	14.6	0.0	1.0	0.0	0.0	0.0	0.0	15.7	0.0	0.0	1.4	0.0
90thFactor:	1.65	1.80	1.79	1.80	1.80	1.80	1.80	1.64	1.80	1.80	1.78	1.80
90thHCM2kQ:	15.9	0.0	1.2	0.0	0.0	0.0	0.0	17.0	0.0	0.0	1.5	0.0
95thFactor:	1.85	2.10	2.08	2.10	2.10	2.10	2.10	1.84	2.10	2.10	2.07	2.10
95thHCM2kQ:	17.8	0.0	1.4	0.0	0.0	0.0	0.0	19.1	0.0	0.0	1.8	0.0
98thFactor:	2.18	2.70	2.65	2.70	2.70	2.70	2.70	2.15	2.70	2.70	2.64	2.70
98thHCM2kQ:	21.0	0.0	1.7	0.0	0.0	0.0	0.0	22.4	0.0	0.0	2.3	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	72.8	0.0	5.7	0.0	0.0	0.0	0.0	159	0.0	0.0	13.5	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 36.577 pounds
 5.926 gallons
 Carbon Dioxide: 114.121 pounds
 Carbon Monoxide: 8.725 pounds
 Hydrocarbons: 1.519 pounds
 Nitrogen Oxides: 0.341 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 34.014 pounds
 5.510 gallons
 Carbon Dioxide: 106.125 pounds
 Carbon Monoxide: 8.525 pounds
 Hydrocarbons: 1.478 pounds
 Nitrogen Oxides: 0.303 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP PM

Intersection #2: Meadowview Road/I-5 NB Ramps

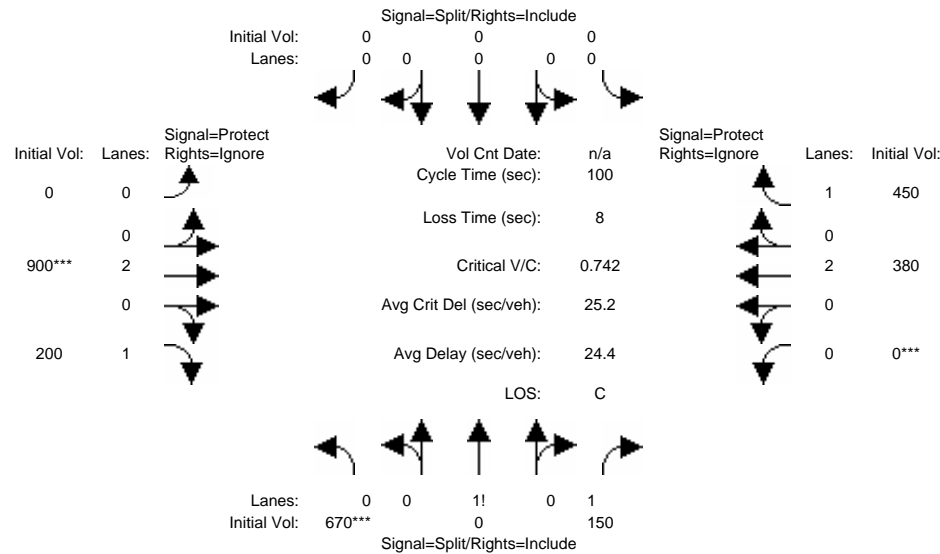


Table with columns for Approach, Movement, and Volume Module. It contains detailed traffic engineering data including saturation flow, capacity analysis, and HCM operations adjusted lane utilization.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.58	0.00	0.58	0.00	0.00	0.00	0.00	0.34	0.00	0.00	0.34	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	15.3	0.0	1.7	0.0	0.0	0.0	0.0	11.6	0.0	0.0	4.1	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	2.7	0.0	0.2	0.0	0.0	0.0	0.0	2.6	0.0	0.0	0.5	0.0
HCM2KQueue:	18.0	0.0	1.9	0.0	0.0	0.0	0.0	14.1	0.0	0.0	4.5	0.0
70thFactor:	1.16	1.20	1.20	1.20	1.20	1.20	1.20	1.17	1.20	1.20	1.19	1.20
70thHCM2kQ:	20.9	0.0	2.3	0.0	0.0	0.0	0.0	16.6	0.0	0.0	5.4	0.0
85thFactor:	1.46	1.60	1.58	1.60	1.60	1.60	1.60	1.49	1.60	1.60	1.56	1.60
85thHCM2kQ:	26.3	0.0	3.0	0.0	0.0	0.0	0.0	21.0	0.0	0.0	7.1	0.0
90thFactor:	1.56	1.80	1.76	1.80	1.80	1.80	1.80	1.60	1.80	1.80	1.72	1.80
90thHCM2kQ:	28.1	0.0	3.3	0.0	0.0	0.0	0.0	22.6	0.0	0.0	7.8	0.0
95thFactor:	1.72	2.10	2.04	2.10	2.10	2.10	2.10	1.77	2.10	2.10	1.97	2.10
95thHCM2kQ:	30.9	0.0	3.9	0.0	0.0	0.0	0.0	25.1	0.0	0.0	8.9	0.0
98thFactor:	1.95	2.70	2.56	2.70	2.70	2.70	2.70	2.04	2.70	2.70	2.41	2.70
98thHCM2kQ:	35.0	0.0	4.9	0.0	0.0	0.0	0.0	28.8	0.0	0.0	10.9	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #2 Meadowview Road/I-5 NB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	123.9	0.0	17.2	0.0	0.0	0.0	0.0	198	0.0	0.0	69.9	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 61.711 pounds
 9.997 gallons
 Carbon Dioxide: 192.537 pounds
 Carbon Monoxide: 15.078 pounds
 Hydrocarbons: 2.736 pounds
 Nitrogen Oxides: 0.555 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 57.891 pounds
 9.378 gallons
 Carbon Dioxide: 180.620 pounds
 Carbon Monoxide: 14.779 pounds
 Hydrocarbons: 2.676 pounds
 Nitrogen Oxides: 0.495 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.07	0.24	0.24	0.09	0.26	0.26	0.27	0.42	0.42	0.09	0.24	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.9	5.8	5.8	1.8	6.3	6.3	6.2	8.8	8.8	1.0	6.1	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	1.1	1.1	1.1	0.7	1.2	1.2	1.2	1.2	1.2	0.3	1.2	0.0
HCM2KQueue:	3.0	6.9	6.9	2.5	7.5	7.5	7.4	10.0	10.0	1.4	7.3	0.0
70thFactor:	1.19	1.18	1.18	1.19	1.18	1.18	1.18	1.18	1.18	1.20	1.18	1.20
70thHCM2kQ:	3.5	8.2	8.2	3.0	8.9	8.9	8.8	11.8	11.8	1.6	8.7	0.0
85thFactor:	1.57	1.54	1.54	1.58	1.53	1.53	1.53	1.52	1.52	1.59	1.54	1.60
85thHCM2kQ:	4.7	10.6	10.6	4.0	11.5	11.5	11.4	15.1	15.1	2.1	11.2	0.0
90thFactor:	1.74	1.68	1.68	1.75	1.67	1.67	1.68	1.64	1.64	1.77	1.68	1.80
90thHCM2kQ:	5.2	11.6	11.6	4.5	12.6	12.6	12.4	16.4	16.4	2.4	12.3	0.0
95thFactor:	2.01	1.91	1.91	2.02	1.90	1.90	1.90	1.84	1.84	2.06	1.90	2.10
95thHCM2kQ:	5.9	13.1	13.1	5.1	14.2	14.2	14.1	18.4	18.4	2.8	13.9	0.0
98thFactor:	2.50	2.29	2.29	2.52	2.26	2.26	2.27	2.16	2.16	2.60	2.27	2.70
98thHCM2kQ:	7.4	15.8	15.8	6.4	17.0	17.0	16.8	21.6	21.6	3.5	16.6	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	16.9	89.7	8.7	16.5	97.4	10.8	56.0	140	11.3	9.3	105	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 91.092 pounds
 14.757 gallons
 Carbon Dioxide: 284.208 pounds
 Carbon Monoxide: 22.741 pounds
 Hydrocarbons: 4.282 pounds
 Nitrogen Oxides: 0.782 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 86.160 pounds
 13.958 gallons
 Carbon Dioxide: 268.818 pounds
 Carbon Monoxide: 22.352 pounds
 Hydrocarbons: 4.204 pounds
 Nitrogen Oxides: 0.703 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP PM

Intersection #3: Meadowview Road/Freeport Boulevard

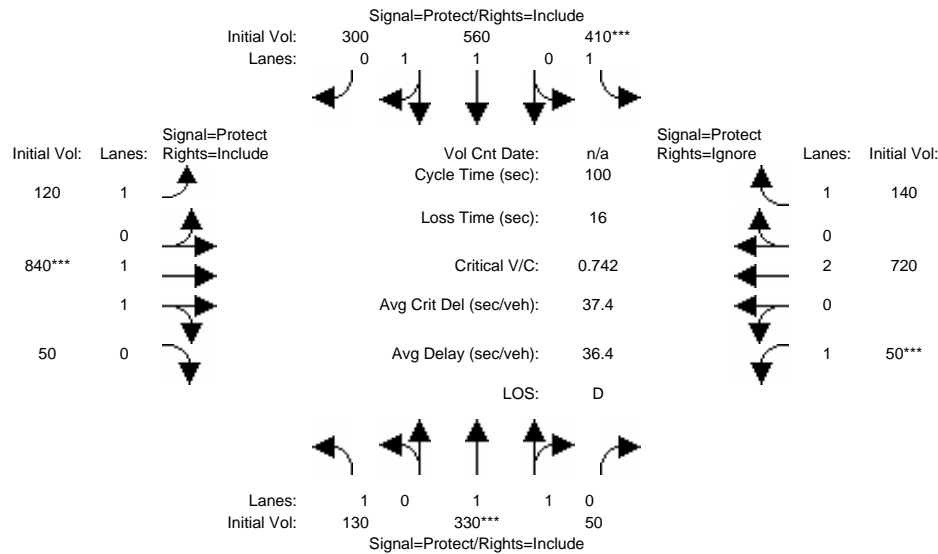


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.10	0.15	0.15	0.31	0.35	0.35	0.10	0.34	0.34	0.05	0.29	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.5	5.3	5.3	10.3	11.0	11.0	3.2	11.6	11.6	1.4	9.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Q2:	2.0	2.4	2.4	2.6	2.4	2.4	1.8	2.7	2.7	1.1	2.1	0.0
HCM2KQueue:	5.5	7.7	7.7	12.9	13.4	13.4	5.1	14.2	14.2	2.4	11.5	0.0
70thFactor:	1.19	1.18	1.18	1.17	1.17	1.17	1.19	1.17	1.17	1.19	1.17	1.20
70thHCM2kQ:	6.5	9.1	9.1	15.1	15.6	15.6	6.0	16.7	16.7	2.9	13.5	0.0
85thFactor:	1.55	1.53	1.53	1.50	1.49	1.49	1.55	1.49	1.49	1.58	1.50	1.60
85thHCM2kQ:	8.5	11.8	11.8	19.3	19.9	19.9	7.9	21.2	21.2	3.8	17.3	0.0
90thFactor:	1.70	1.67	1.67	1.61	1.61	1.61	1.71	1.60	1.60	1.75	1.62	1.80
90thHCM2kQ:	9.4	12.9	12.9	20.8	21.4	21.4	8.7	22.7	22.7	4.2	18.7	0.0
95thFactor:	1.94	1.89	1.89	1.79	1.79	1.79	1.95	1.77	1.77	2.02	1.82	2.10
95thHCM2kQ:	10.7	14.6	14.6	23.1	23.8	23.8	9.9	25.2	25.2	4.9	20.9	0.0
98thFactor:	2.35	2.25	2.25	2.07	2.06	2.06	2.38	2.03	2.03	2.53	2.11	2.70
98thHCM2kQ:	13.0	17.3	17.3	26.7	27.5	27.5	12.0	29.0	29.0	6.1	24.3	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #3 Meadowview Road/Freeport Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	31.5	79.2	12.0	92.4	122	65.3	29.1	187	11.1	12.2	160	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 135.231 pounds
 21.907 gallons
 Carbon Dioxide: 421.922 pounds
 Carbon Monoxide: 34.208 pounds
 Hydrocarbons: 6.579 pounds
 Nitrogen Oxides: 1.130 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 128.554 pounds
 20.826 gallons
 Carbon Dioxide: 401.087 pounds
 Carbon Monoxide: 33.680 pounds
 Hydrocarbons: 6.474 pounds
 Nitrogen Oxides: 1.021 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP AM

Intersection #4: Meadowview Road/Manorside Drive

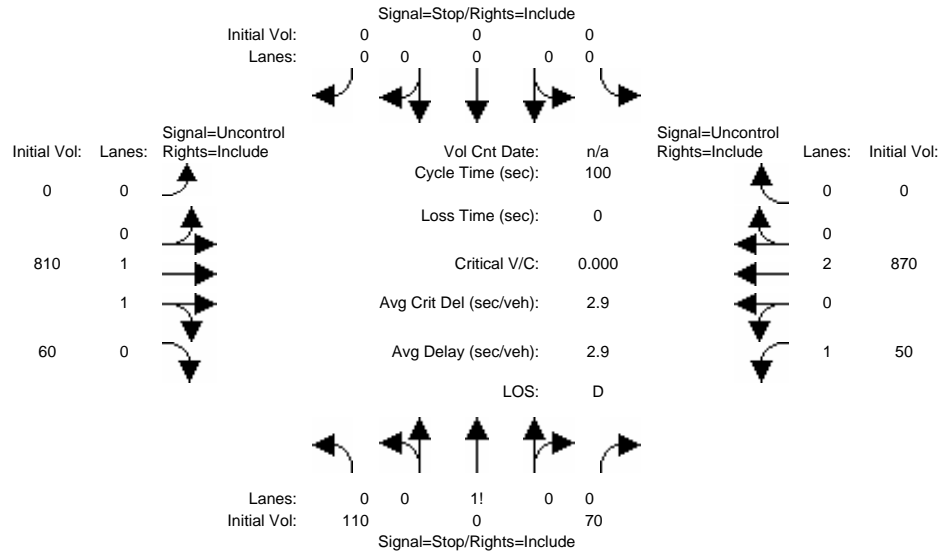


Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Shared Queue, Shared LOS, and Approach Del/Los.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Pedestrian Walk Speed, Lane Width, and Time Period.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Two-Stage Gap Acceptance and Stage One Module.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Two-Stage Gap Acceptance and Stage Two Module.

Intersection #4 Meadowview Road/Manorside Drive
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Control, Lanes, Initial Vol, and Approach Del.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.4]
FAIL - Vehicle-hours less than 4 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	0	0	0	1
Initial Vol:	110	0	70	0	0	0	0	0	810	60	50	870

Major Street Volume: 1790
 Minor Approach Volume: 180
 Minor Approach Volume Threshold: 84 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP PM

Intersection #4: Meadowview Road/Manorside Drive

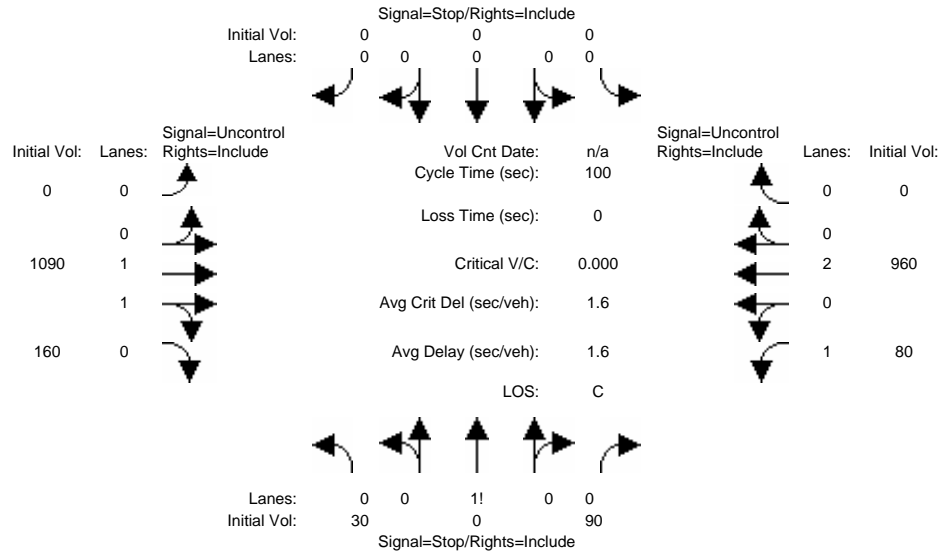


Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Vol.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Critical Gap Module, Capacity Module, and Level of Service Module.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Shared Queue, Shared LOS, ApproachDel, ApproachLOS, HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Two-Stage Gap Acceptance - Stage One Module and Two-Stage Gap Acceptance - Stage Two Module.

Peak Hour Delay Signal Warrant Report

Intersection #4 Meadowview Road/Manorside Drive
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.8]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=120]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=2410]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled											
Lanes:	0	0	1	0	0	0	0	0	0	0	0	1	1	0	1	0	1	0	2	0	0
Initial Vol:	30	0	90	0	0	0	0	0	0	0	1090	160	80	960	0	0					

Major Street Volume: 2290
Minor Approach Volume: 120
Minor Approach Volume Threshold: -1 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #5: Meadowview Road/24th Street

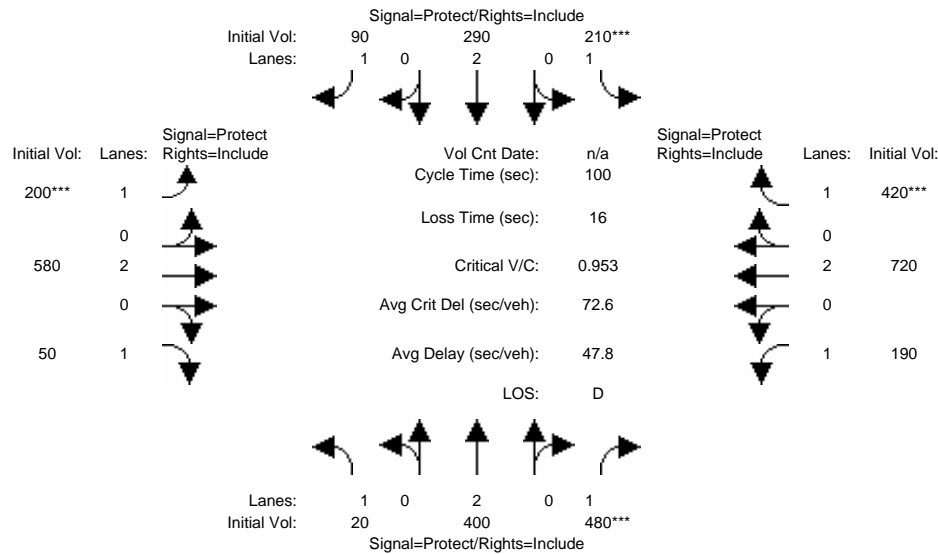


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.17	0.32	0.32	0.12	0.28	0.28	0.12	0.24	0.24	0.16	0.28	0.28
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.5	4.5	13.0	5.8	3.3	1.9	5.5	7.7	1.1	5.0	9.5	11.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.1	0.5	6.4	4.5	0.4	0.3	4.4	1.9	0.2	1.8	2.4	6.1
HCM2KQueue:	0.5	5.0	19.5	10.3	3.8	2.2	9.9	9.7	1.2	6.8	11.9	17.5
70thFactor:	1.20	1.19	1.16	1.18	1.19	1.19	1.18	1.18	1.20	1.18	1.17	1.16
70thHCM2kQ:	0.6	6.0	22.6	12.1	4.5	2.6	11.7	11.4	1.5	8.1	14.0	20.4
85thFactor:	1.59	1.55	1.46	1.51	1.56	1.58	1.52	1.52	1.59	1.54	1.50	1.47
85thHCM2kQ:	0.9	7.8	28.3	15.6	5.9	3.4	15.1	14.6	2.0	10.5	17.9	25.7
90thFactor:	1.79	1.71	1.55	1.64	1.73	1.76	1.64	1.65	1.78	1.68	1.62	1.57
90thHCM2kQ:	1.0	8.6	30.2	16.9	6.5	3.8	16.3	15.9	2.2	11.5	19.3	27.4
95thFactor:	2.08	1.95	1.70	1.84	1.99	2.03	1.85	1.85	2.06	1.91	1.81	1.73
95thHCM2kQ:	1.1	9.8	33.1	19.0	7.5	4.4	18.3	17.9	2.6	13.1	21.6	30.3
98thFactor:	2.66	2.38	1.92	2.15	2.45	2.55	2.17	2.18	2.61	2.29	2.10	1.96
98thHCM2kQ:	1.4	12.0	37.4	22.2	9.2	5.6	21.5	21.0	3.2	15.7	25.0	34.3

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	4.2	76.9	117.4	52.2	57.3	17.3	49.7	132	9.8	44.9	163	103.1

Name: year 1995 composite fleet
 Fuel Consumption: 167.509 pounds
 27.136 gallons
 Carbon Dioxide: 522.629 pounds
 Carbon Monoxide: 43.283 pounds
 Hydrocarbons: 8.638 pounds
 Nitrogen Oxides: 1.299 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 160.652 pounds
 26.026 gallons
 Carbon Dioxide: 501.235 pounds
 Carbon Monoxide: 42.741 pounds
 Hydrocarbons: 8.530 pounds
 Nitrogen Oxides: 1.187 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.14	0.29	0.29	0.29	0.43	0.43	0.08	0.21	0.21	0.06	0.18	0.18
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.5	3.0	14.4	16.1	3.5	4.4	4.7	12.1	0.4	3.1	10.5	8.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.3	0.4	12.9	14.0	0.3	0.5	6.1	11.1	0.1	4.6	10.3	6.5
HCM2KQueue:	1.8	3.4	27.4	30.1	3.9	5.0	10.8	23.2	0.5	7.6	20.9	14.5
70thFactor:	1.20	1.19	1.15	1.15	1.19	1.19	1.18	1.16	1.20	1.18	1.16	1.17
70thHCM2kQ:	2.1	4.0	31.5	34.5	4.6	5.9	12.7	26.9	0.6	9.0	24.2	17.0
85thFactor:	1.58	1.57	1.42	1.41	1.56	1.55	1.51	1.44	1.59	1.53	1.45	1.48
85thHCM2kQ:	2.8	5.3	38.9	42.4	6.0	7.7	16.3	33.4	0.8	11.7	30.2	21.6
90thFactor:	1.77	1.74	1.50	1.49	1.73	1.71	1.63	1.53	1.79	1.67	1.54	1.59
90thHCM2kQ:	3.2	5.9	41.1	44.8	6.7	8.5	17.6	35.4	0.9	12.8	32.1	23.2
95thFactor:	2.04	2.00	1.63	1.61	1.98	1.95	1.83	1.67	2.08	1.89	1.69	1.77
95thHCM2kQ:	3.6	6.8	44.6	48.5	7.6	9.7	19.8	38.7	1.1	14.4	35.2	25.7
98thFactor:	2.57	2.47	1.82	1.80	2.44	2.38	2.14	1.87	2.66	2.26	1.90	2.03
98thHCM2kQ:	4.6	8.4	49.9	54.1	9.4	11.9	23.1	43.4	1.4	17.2	39.7	29.5

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #5 Meadowview Road/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	13.3	52.0	137.7	153.6	60.3	40.0	43.0	215	4.0	27.7	186	72.9

Name: year 1995 composite fleet
 Fuel Consumption: 338.591 pounds
 54.852 gallons
 Carbon Dioxide: 1056.403 pounds
 Carbon Monoxide: 91.291 pounds
 Hydrocarbons: 19.493 pounds
 Nitrogen Oxides: 2.216 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 330.616 pounds
 53.560 gallons
 Carbon Dioxide: 1031.523 pounds
 Carbon Monoxide: 90.658 pounds
 Hydrocarbons: 19.367 pounds
 Nitrogen Oxides: 2.084 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP AM

Intersection #6: Meadowview Road/Detroit Boulevard

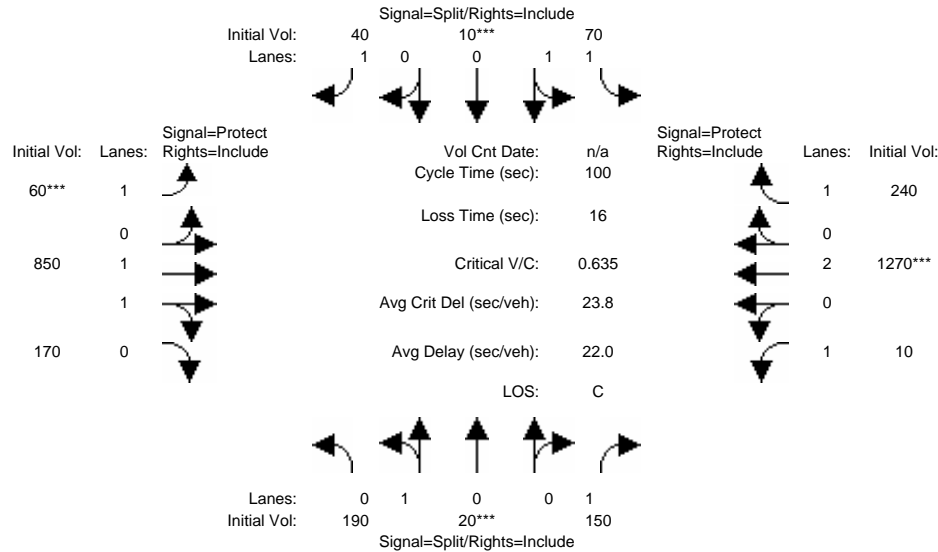


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound), Movement, and various performance metrics including Volume Module, Saturation Flow Module, Capacity Analysis Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.18	0.18	0.18	0.07	0.07	0.07	0.05	0.51	0.51	0.09	0.54	0.54
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	5.4	5.4	3.8	1.1	1.1	1.1	1.6	10.4	10.4	0.3	13.3	3.6
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.7	1.7	1.1	0.5	0.5	0.5	1.4	1.4	1.4	0.1	1.9	0.4
HCM2KQueue:	7.2	7.2	4.9	1.5	1.5	1.6	3.1	11.8	11.8	0.3	15.2	4.0
70thFactor:	1.18	1.18	1.19	1.20	1.20	1.20	1.19	1.17	1.17	1.20	1.17	1.19
70thHCM2kQ:	8.5	8.5	5.8	1.8	1.8	1.9	3.7	13.9	13.9	0.4	17.7	4.8
85thFactor:	1.54	1.54	1.56	1.59	1.59	1.58	1.57	1.50	1.50	1.60	1.48	1.56
85thHCM2kQ:	11.0	11.0	7.6	2.4	2.4	2.5	4.8	17.7	17.7	0.5	22.5	6.2
90thFactor:	1.68	1.68	1.71	1.77	1.77	1.77	1.74	1.62	1.62	1.79	1.59	1.73
90thHCM2kQ:	12.1	12.1	8.3	2.7	2.7	2.8	5.3	19.1	19.1	0.6	24.1	6.9
95thFactor:	1.90	1.90	1.96	2.05	2.05	2.05	2.01	1.81	1.81	2.09	1.76	1.98
95thHCM2kQ:	13.7	13.7	9.5	3.1	3.1	3.3	6.2	21.4	21.4	0.7	26.7	7.9
98thFactor:	2.28	2.28	2.39	2.59	2.59	2.58	2.49	2.10	2.10	2.68	2.01	2.44
98thHCM2kQ:	16.3	16.3	11.6	3.9	3.9	4.1	7.6	24.8	24.8	0.9	30.5	9.7

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	44.3	4.7	34.1	16.6	2.4	9.5	14.7	149	29.8	2.3	227	32.4

Name: year 1995 composite fleet

Fuel Consumption: 85.011 pounds
 13.772 gallons
 Carbon Dioxide: 265.233 pounds
 Carbon Monoxide: 20.571 pounds
 Hydrocarbons: 3.674 pounds
 Nitrogen Oxides: 0.775 pounds

Name: year 2000 composite fleet

Fuel Consumption: 79.471 pounds
 12.874 gallons
 Carbon Dioxide: 247.949 pounds
 Carbon Monoxide: 20.138 pounds
 Hydrocarbons: 3.586 pounds
 Nitrogen Oxides: 0.690 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.12	0.12	0.10	0.10	0.10	0.09	0.52	0.52	0.10	0.53	0.53
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	5.2	5.2	1.8	4.1	4.1	1.6	0.8	19.8	19.8	4.1	10.8	1.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	3.3	3.3	0.5	3.1	3.1	0.6	0.2	4.8	4.8	3.1	1.3	0.1
HCM2KQueue:	8.5	8.5	2.3	7.2	7.2	2.2	1.0	24.6	24.6	7.2	12.1	1.2
70thFactor:	1.18	1.18	1.19	1.18	1.18	1.19	1.20	1.15	1.15	1.18	1.17	1.20
70thHCM2kQ:	10.0	10.0	2.8	8.5	8.5	2.6	1.2	28.4	28.4	8.5	14.2	1.4
85thFactor:	1.53	1.53	1.58	1.54	1.54	1.58	1.59	1.43	1.43	1.54	1.50	1.59
85thHCM2kQ:	12.9	12.9	3.7	11.0	11.0	3.4	1.6	35.2	35.2	11.0	18.2	1.9
90thFactor:	1.66	1.66	1.76	1.68	1.68	1.76	1.78	1.52	1.52	1.68	1.62	1.78
90thHCM2kQ:	14.1	14.1	4.1	12.0	12.0	3.8	1.8	37.3	37.3	12.0	19.6	2.1
95thFactor:	1.87	1.87	2.03	1.90	1.90	2.03	2.07	1.65	1.65	1.90	1.81	2.06
95thHCM2kQ:	15.9	15.9	4.7	13.6	13.6	4.4	2.1	40.6	40.6	13.6	21.9	2.5
98thFactor:	2.22	2.22	2.54	2.28	2.28	2.55	2.62	1.85	1.85	2.28	2.09	2.61
98thHCM2kQ:	18.8	18.8	5.9	16.3	16.3	5.5	2.7	45.5	45.5	16.3	25.4	3.1

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #6 Meadowview Road/Detroit Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	41.7	4.9	16.0	68.9	4.9	14.1	7.0	31.3	26.1	36.9	18.4	9.9

Name: year 1995 composite fleet
 Fuel Consumption: 119.732 pounds
 19.397 gallons
 Carbon Dioxide: 373.563 pounds
 Carbon Monoxide: 29.875 pounds
 Hydrocarbons: 5.625 pounds
 Nitrogen Oxides: 1.025 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 113.237 pounds
 18.344 gallons
 Carbon Dioxide: 353.299 pounds
 Carbon Monoxide: 29.364 pounds
 Hydrocarbons: 5.522 pounds
 Nitrogen Oxides: 0.922 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #7: Mack Road/Franklin Boulevard

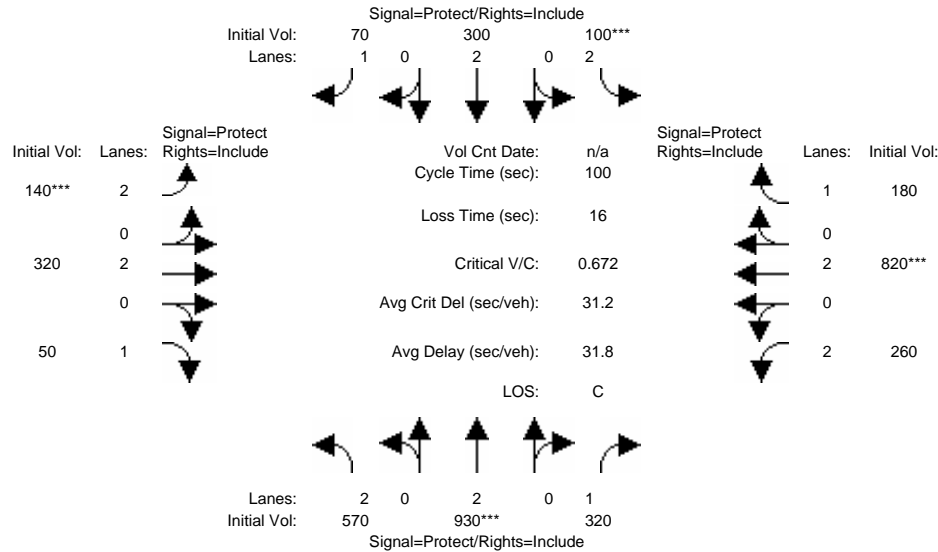


Table with columns for Approach (North, South, East, West) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.29	0.39	0.39	0.05	0.15	0.15	0.06	0.22	0.22	0.18	0.34	0.34
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	7.0	11.3	6.8	1.4	4.1	1.7	2.0	4.0	1.1	3.3	10.3	3.7
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.3	2.0	1.1	1.1	1.2	0.4	1.5	0.7	0.2	0.7	2.0	0.5
HCM2KQueue:	8.2	13.3	7.9	2.5	5.3	2.2	3.5	4.7	1.3	4.0	12.2	4.2
70thFactor:	1.18	1.17	1.18	1.19	1.19	1.19	1.19	1.19	1.20	1.19	1.17	1.19
70thHCM2kQ:	9.7	15.5	9.3	3.0	6.3	2.6	4.2	5.6	1.5	4.7	14.3	5.0
85thFactor:	1.53	1.49	1.53	1.58	1.55	1.58	1.57	1.56	1.59	1.56	1.50	1.56
85thHCM2kQ:	12.6	19.8	12.1	4.0	8.3	3.4	5.5	7.3	2.0	6.2	18.3	6.6
90thFactor:	1.67	1.61	1.67	1.75	1.71	1.76	1.74	1.72	1.78	1.73	1.62	1.72
90thHCM2kQ:	13.7	21.3	13.2	4.4	9.1	3.8	6.1	8.1	2.3	6.9	19.8	7.3
95thFactor:	1.88	1.79	1.89	2.02	1.95	2.03	1.99	1.96	2.06	1.98	1.80	1.97
95thHCM2kQ:	15.5	23.7	14.9	5.1	10.4	4.4	7.0	9.2	2.7	7.9	22.1	8.3
98thFactor:	2.23	2.06	2.25	2.52	2.36	2.55	2.46	2.40	2.61	2.44	2.09	2.42
98thHCM2kQ:	18.4	27.3	17.7	6.4	12.6	5.5	8.6	11.3	3.4	9.7	25.6	10.2

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	121.4	193	61.4	24.5	69.8	15.6	34.3	68.7	10.1	57.4	176	33.4

Name: year 1995 composite fleet
 Fuel Consumption: 140.811 pounds
 22.811 gallons
 Carbon Dioxide: 439.332 pounds
 Carbon Monoxide: 35.225 pounds
 Hydrocarbons: 6.654 pounds
 Nitrogen Oxides: 1.205 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 133.288 pounds
 21.593 gallons
 Carbon Dioxide: 415.859 pounds
 Carbon Monoxide: 34.633 pounds
 Hydrocarbons: 6.535 pounds
 Nitrogen Oxides: 1.085 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #7: Mack Road/Franklin Boulevard

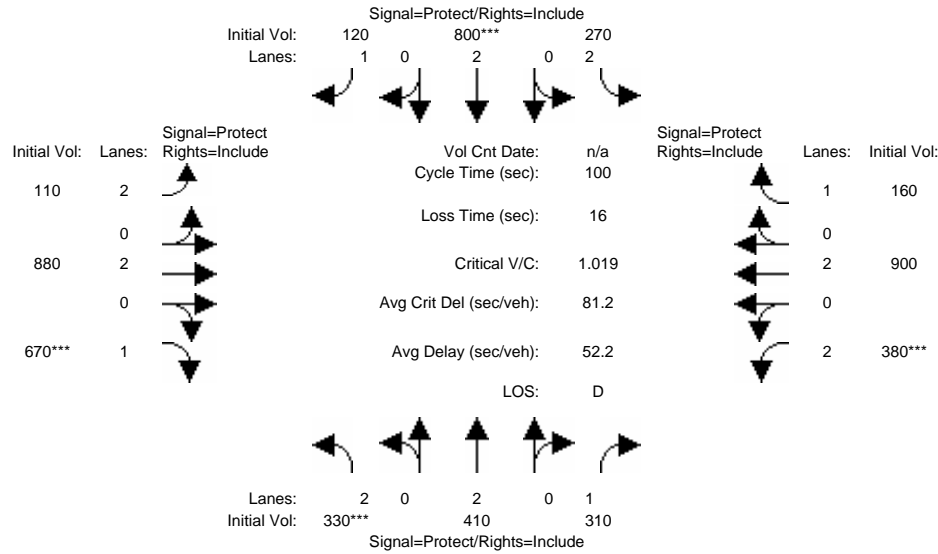


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.09	0.23	0.23	0.09	0.22	0.22	0.09	0.42	0.42	0.11	0.44	0.44
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.7	5.3	8.3	3.8	11.7	2.8	1.5	10.0	18.6	5.4	9.9	2.8
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	4.8	1.0	3.9	3.1	7.8	0.5	0.6	1.4	10.0	5.2	1.3	0.3
HCM2KQueue:	9.5	6.3	12.2	6.9	19.5	3.3	2.1	11.5	28.6	10.6	11.3	3.1
70thFactor:	1.18	1.19	1.17	1.18	1.16	1.19	1.19	1.18	1.15	1.18	1.18	1.19
70thHCM2kQ:	11.2	7.4	14.4	8.1	22.6	4.0	2.5	13.5	32.9	12.5	13.2	3.7
85thFactor:	1.52	1.54	1.50	1.54	1.46	1.57	1.58	1.50	1.42	1.51	1.51	1.57
85thHCM2kQ:	14.5	9.7	18.4	10.6	28.4	5.2	3.2	17.2	40.5	16.1	17.0	4.8
90thFactor:	1.65	1.69	1.62	1.68	1.55	1.74	1.76	1.63	1.50	1.64	1.63	1.74
90thHCM2kQ:	15.7	10.6	19.8	11.6	30.2	5.8	3.6	18.6	42.8	17.4	18.3	5.4
95thFactor:	1.85	1.92	1.80	1.91	1.70	2.00	2.04	1.82	1.62	1.83	1.82	2.01
95thHCM2kQ:	17.7	12.1	22.1	13.1	33.2	6.6	4.2	20.8	46.4	19.5	20.5	6.2
98thFactor:	2.18	2.32	2.09	2.29	1.92	2.47	2.55	2.11	1.81	2.14	2.12	2.49
98thHCM2kQ:	20.8	14.5	25.6	15.7	37.5	8.2	5.2	24.2	51.8	22.8	23.9	7.7

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #7 Mack Road/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	82.7	89.8	74.6	66.6	201	25.3	26.0	171	169.8	95.2	170	25.0

Name: year 1995 composite fleet
 Fuel Consumption: 260.914 pounds
 42.268 gallons
 Carbon Dioxide: 814.052 pounds
 Carbon Monoxide: 67.842 pounds
 Hydrocarbons: 13.688 pounds
 Nitrogen Oxides: 1.972 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 250.908 pounds
 40.647 gallons
 Carbon Dioxide: 782.833 pounds
 Carbon Monoxide: 67.051 pounds
 Hydrocarbons: 13.530 pounds
 Nitrogen Oxides: 1.809 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #8: Cosumnes River Boulevard/Franklin Boulevard

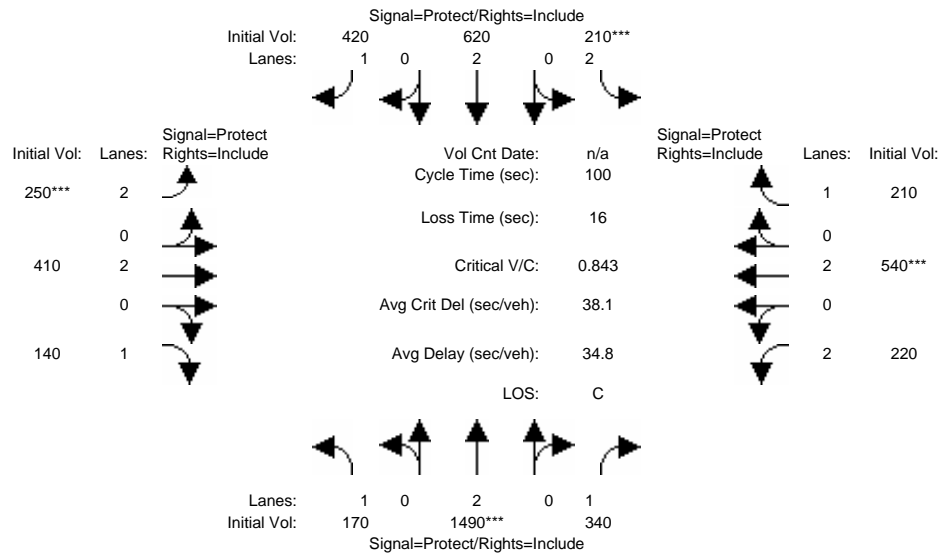


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.15	0.50	0.50	0.07	0.42	0.42	0.09	0.17	0.17	0.10	0.18	0.18
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.4	18.8	6.0	3.0	6.4	9.2	3.5	5.6	3.5	3.0	7.6	5.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.5	4.3	0.7	2.6	0.7	1.6	2.8	1.8	1.0	1.7	3.5	2.2
HCM2KQueue:	6.0	23.2	6.8	5.6	7.1	10.8	6.3	7.4	4.5	4.7	11.1	7.7
70thFactor:	1.19	1.16	1.18	1.19	1.18	1.18	1.19	1.18	1.19	1.19	1.18	1.18
70thHCM2kQ:	7.1	26.8	8.0	6.6	8.4	12.7	7.5	8.8	5.4	5.6	13.1	9.1
85thFactor:	1.55	1.44	1.54	1.55	1.54	1.51	1.54	1.53	1.56	1.56	1.51	1.53
85thHCM2kQ:	9.2	33.3	10.4	8.7	10.9	16.4	9.7	11.4	7.1	7.3	16.8	11.8
90thFactor:	1.70	1.53	1.69	1.70	1.68	1.63	1.69	1.68	1.72	1.72	1.63	1.67
90thHCM2kQ:	10.1	35.3	11.4	9.5	11.9	17.7	10.7	12.4	7.8	8.1	18.1	12.9
95thFactor:	1.93	1.67	1.91	1.94	1.90	1.83	1.92	1.90	1.97	1.96	1.82	1.89
95thHCM2kQ:	11.5	38.6	12.9	10.9	13.5	19.8	12.1	14.1	8.9	9.2	20.3	14.6
98thFactor:	2.33	1.87	2.29	2.35	2.28	2.13	2.32	2.27	2.41	2.40	2.12	2.25
98thHCM2kQ:	13.9	43.3	15.5	13.2	16.1	23.1	14.6	16.8	10.9	11.3	23.7	17.4

Fuel Consumption and Emissions

2000 HCM Operations Method

Base Volume Alternative

 Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	39.9	322	54.2	51.9	109	82.9	61.6	96.0	31.8	53.2	130	49.6

Name: year 1995 composite fleet
 Fuel Consumption: 184.697 pounds
 29.921 gallons
 Carbon Dioxide: 576.255 pounds
 Carbon Monoxide: 46.561 pounds
 Hydrocarbons: 8.916 pounds
 Nitrogen Oxides: 1.546 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 175.372 pounds
 28.410 gallons
 Carbon Dioxide: 547.159 pounds
 Carbon Monoxide: 45.826 pounds
 Hydrocarbons: 8.768 pounds
 Nitrogen Oxides: 1.396 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.09	0.36	0.36	0.13	0.40	0.40	0.15	0.17	0.17	0.17	0.19	0.19
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.8	8.0	6.6	3.3	15.9	6.4	6.0	7.2	6.2	6.9	7.9	5.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	2.9	1.1	1.1	1.1	4.2	0.9	3.1	3.4	3.3	3.4	3.3	1.7
HCM2KQueue:	6.7	9.2	7.6	4.4	20.1	7.3	9.0	10.6	9.5	10.4	11.1	6.8
70thFactor:	1.18	1.18	1.18	1.19	1.16	1.18	1.18	1.18	1.18	1.18	1.18	1.18
70thHCM2kQ:	7.9	10.8	9.0	5.3	23.3	8.7	10.7	12.5	11.2	12.2	13.1	8.1
85thFactor:	1.54	1.52	1.53	1.56	1.45	1.53	1.52	1.51	1.52	1.51	1.51	1.54
85thHCM2kQ:	10.3	13.9	11.7	6.9	29.2	11.3	13.8	16.0	14.5	15.7	16.8	10.5
90thFactor:	1.69	1.65	1.67	1.72	1.55	1.68	1.65	1.64	1.65	1.64	1.63	1.68
90thHCM2kQ:	11.3	15.1	12.8	7.6	31.1	12.3	14.9	17.3	15.7	17.0	18.2	11.5
95thFactor:	1.91	1.86	1.89	1.97	1.70	1.90	1.86	1.83	1.85	1.84	1.82	1.91
95thHCM2kQ:	12.8	17.0	14.5	8.7	34.1	13.9	16.8	19.4	17.6	19.1	20.3	13.1
98thFactor:	2.30	2.19	2.26	2.41	1.91	2.27	2.20	2.14	2.18	2.15	2.12	2.29
98thHCM2kQ:	15.4	20.1	17.2	10.7	38.4	16.6	19.9	22.7	20.8	22.3	23.7	15.7

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #8 Cosummes River Boulevard/Franklin Boulevard

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	34.4	137	59.2	58.3	272	57.7	104.2	123	55.7	121.1	134	46.2

Name: year 1995 composite fleet
 Fuel Consumption: 216.047 pounds
 35.000 gallons
 Carbon Dioxide: 674.068 pounds
 Carbon Monoxide: 55.090 pounds
 Hydrocarbons: 10.746 pounds
 Nitrogen Oxides: 1.758 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 206.059 pounds
 33.382 gallons
 Carbon Dioxide: 642.904 pounds
 Carbon Monoxide: 54.300 pounds
 Hydrocarbons: 10.588 pounds
 Nitrogen Oxides: 1.595 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.14	0.22	0.22	0.16	0.24	0.24	0.09	0.31	0.31	0.15	0.37	0.37
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.6	2.3	2.3	1.0	5.9	5.9	2.4	4.1	4.1	0.2	8.8	8.8
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.2	0.3	0.3	0.2	1.3	1.3	1.2	0.5	0.5	0.0	1.3	1.3
HCM2kQueue:	4.9	2.6	2.6	1.1	7.1	7.1	3.6	4.6	4.6	0.3	10.0	10.0
70thFactor:	1.19	1.19	1.19	1.20	1.18	1.18	1.19	1.19	1.19	1.20	1.18	1.18
70thHCM2kQ:	5.8	3.1	3.1	1.3	8.4	8.4	4.3	5.4	5.4	0.3	11.8	11.8
85thFactor:	1.56	1.57	1.57	1.59	1.54	1.54	1.57	1.56	1.56	1.60	1.51	1.51
85thHCM2kQ:	7.6	4.1	4.1	1.8	11.0	11.0	5.6	7.1	7.1	0.4	15.2	15.2
90thFactor:	1.71	1.75	1.75	1.78	1.68	1.68	1.73	1.72	1.72	1.79	1.64	1.64
90thHCM2kQ:	8.3	4.6	4.6	2.0	12.0	12.0	6.2	7.8	7.8	0.5	16.5	16.5
95thFactor:	1.96	2.02	2.02	2.06	1.90	1.90	1.99	1.97	1.97	2.09	1.84	1.84
95thHCM2kQ:	9.5	5.3	5.3	2.3	13.6	13.6	7.1	9.0	9.0	0.6	18.5	18.5
98thFactor:	2.39	2.52	2.52	2.62	2.28	2.28	2.46	2.40	2.40	2.68	2.16	2.16
98thHCM2kQ:	11.6	6.6	6.6	2.9	16.3	16.3	8.8	11.0	11.0	0.7	21.7	21.7

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	32.7	18.6	2.1	8.6	33.0	19.8	21.6	78.6	21.1	2.1	209	6.0

Name: year 1995 composite fleet
Fuel Consumption: 73.052 pounds
11.834 gallons
Carbon Dioxide: 227.922 pounds
Carbon Monoxide: 18.151 pounds
Hydrocarbons: 3.392 pounds
Nitrogen Oxides: 0.632 pounds

Name: year 2000 composite fleet
Fuel Consumption: 68.975 pounds
11.174 gallons
Carbon Dioxide: 215.202 pounds
Carbon Monoxide: 17.831 pounds
Hydrocarbons: 3.328 pounds
Nitrogen Oxides: 0.568 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.17	0.32	0.32	0.10	0.25	0.25	0.14	0.32	0.32	0.10	0.28	0.28
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.1	6.3	6.3	0.8	5.8	5.8	3.4	1.3	5.9	0.3	6.5	6.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	1.1	0.9	0.9	0.2	1.1	1.1	1.1	0.1	1.0	0.1	1.1	1.1
HCM2KQueue:	5.1	7.2	7.2	1.0	6.9	6.9	4.4	1.4	6.9	0.3	7.7	7.7
70thFactor:	1.19	1.18	1.18	1.20	1.18	1.18	1.19	1.20	1.18	1.20	1.18	1.18
70thHCM2kQ:	6.1	8.6	8.6	1.2	8.1	8.1	5.3	1.7	8.2	0.4	9.0	9.0
85thFactor:	1.55	1.54	1.54	1.59	1.54	1.54	1.56	1.59	1.54	1.60	1.53	1.53
85thHCM2kQ:	8.0	11.1	11.1	1.5	10.6	10.6	6.9	2.3	10.6	0.5	11.7	11.7
90thFactor:	1.71	1.68	1.68	1.78	1.68	1.68	1.72	1.77	1.68	1.79	1.67	1.67
90thHCM2kQ:	8.8	12.1	12.1	1.7	11.6	11.6	7.6	2.6	11.6	0.6	12.8	12.8
95thFactor:	1.95	1.90	1.90	2.07	1.91	1.91	1.97	2.05	1.91	2.09	1.89	1.89
95thHCM2kQ:	10.0	13.7	13.7	2.0	13.1	13.1	8.7	3.0	13.1	0.6	14.5	14.5
98thFactor:	2.37	2.27	2.27	2.63	2.29	2.29	2.41	2.60	2.29	2.68	2.26	2.26
98thHCM2kQ:	12.2	16.4	16.4	2.5	15.7	15.7	10.7	3.7	15.8	0.8	17.3	17.3

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #14 Cosumnes River Boulevard/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	36.5	38.4	18.2	6.9	36.7	15.1	30.3	21.3	48.6	2.3	15.4	6.3

Name: year 1995 composite fleet
Fuel Consumption: 68.218 pounds
11.051 gallons
Carbon Dioxide: 212.839 pounds
Carbon Monoxide: 17.065 pounds
Hydrocarbons: 3.225 pounds
Nitrogen Oxides: 0.582 pounds

Name: year 2000 composite fleet
Fuel Consumption: 64.576 pounds
10.461 gallons
Carbon Dioxide: 201.478 pounds
Carbon Monoxide: 16.778 pounds
Hydrocarbons: 3.167 pounds
Nitrogen Oxides: 0.524 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #15: Cosumnes River Boulevard/Street B

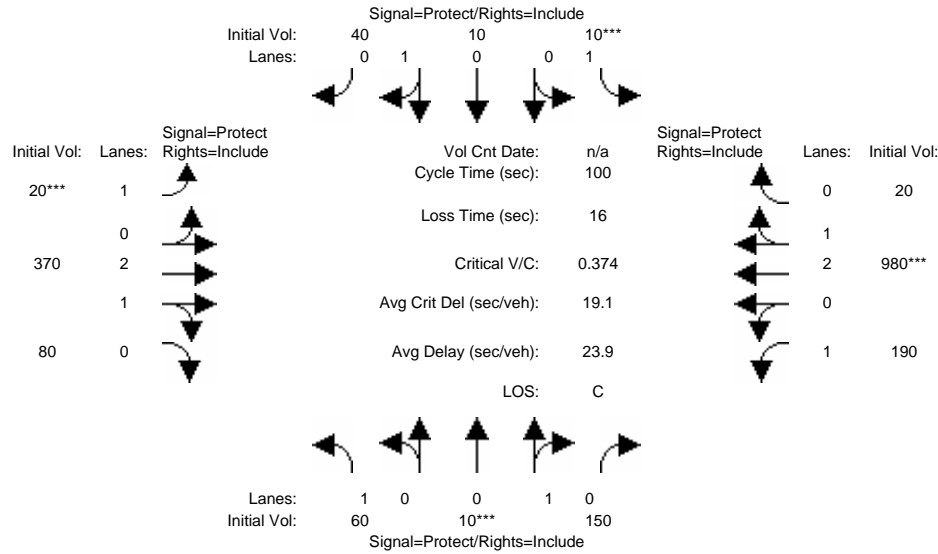


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.25	0.25	0.05	0.17	0.17	0.05	0.25	0.25	0.29	0.49	0.49
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.5	3.7	3.7	0.3	1.2	1.2	0.5	3.8	3.8	4.2	6.5	6.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.4	0.7	0.7	0.1	0.2	0.2	0.3	0.6	0.6	0.6	0.7	0.7
HCM2KQueue:	1.9	4.4	4.4	0.4	1.4	1.4	0.8	4.4	4.4	4.8	7.1	7.1
70thFactor:	1.20	1.19	1.19	1.20	1.20	1.20	1.20	1.19	1.19	1.19	1.18	1.18
70thHCM2kQ:	2.2	5.2	5.2	0.5	1.7	1.7	1.0	5.2	5.2	5.6	8.4	8.4
85thFactor:	1.58	1.56	1.56	1.60	1.59	1.59	1.59	1.56	1.56	1.56	1.54	1.54
85thHCM2kQ:	3.0	6.8	6.8	0.6	2.2	2.2	1.3	6.8	6.8	7.4	10.9	10.9
90thFactor:	1.76	1.72	1.72	1.79	1.77	1.77	1.78	1.72	1.72	1.72	1.68	1.68
90thHCM2kQ:	3.3	7.5	7.5	0.7	2.5	2.5	1.5	7.5	7.5	8.2	12.0	12.0
95thFactor:	2.04	1.97	1.97	2.09	2.06	2.06	2.07	1.97	1.97	1.96	1.90	1.90
95thHCM2kQ:	3.8	8.6	8.6	0.8	2.9	2.9	1.7	8.6	8.6	9.3	13.6	13.6
98thFactor:	2.57	2.41	2.41	2.67	2.60	2.60	2.64	2.42	2.42	2.39	2.28	2.28
98thHCM2kQ:	4.8	10.5	10.5	1.0	3.6	3.6	2.2	10.5	10.5	11.4	16.2	16.2

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	13.6	2.1	31.3	2.4	2.1	8.5	4.8	76.5	16.5	37.6	155	3.2

Name: year 1995 composite fleet
 Fuel Consumption: 56.039 pounds
 9.078 gallons
 Carbon Dioxide: 174.842 pounds
 Carbon Monoxide: 13.669 pounds
 Hydrocarbons: 2.480 pounds
 Nitrogen Oxides: 0.499 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 52.556 pounds
 8.514 gallons
 Carbon Dioxide: 163.974 pounds
 Carbon Monoxide: 13.396 pounds
 Hydrocarbons: 2.425 pounds
 Nitrogen Oxides: 0.445 pounds

DISCLAIMER
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #15: Cosumnes River Boulevard/Street B

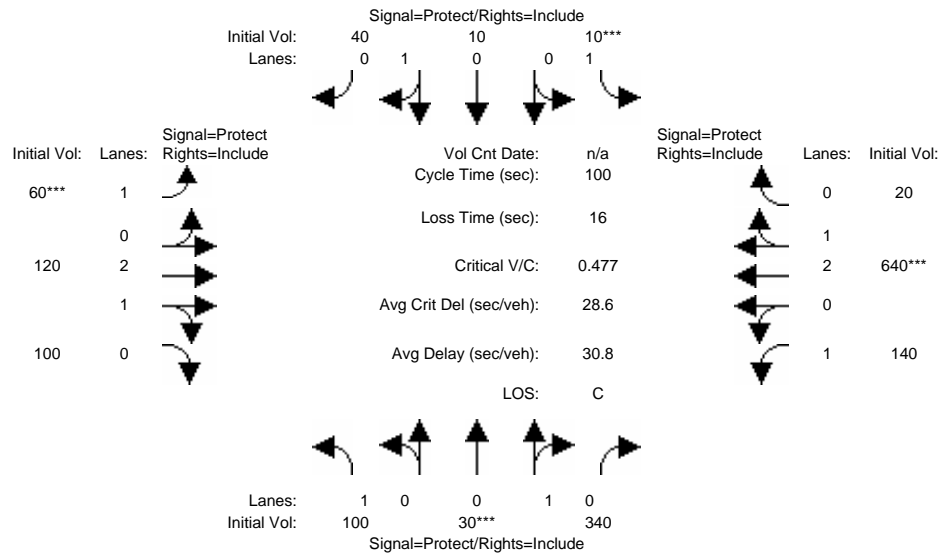


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.23	0.46	0.46	0.05	0.28	0.28	0.07	0.15	0.15	0.17	0.26	0.26
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	2.3	7.2	7.2	0.3	1.0	1.0	1.6	1.6	2.8	3.5	5.7	5.7
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.3	1.0	1.0	0.1	0.1	0.1	0.9	0.3	0.7	0.8	1.0	1.0
HCM2KQueue:	2.6	8.2	8.2	0.4	1.1	1.1	2.5	1.9	3.4	4.3	6.7	6.7
70thFactor:	1.19	1.18	1.18	1.20	1.20	1.20	1.19	1.20	1.19	1.19	1.18	1.18
70thHCM2kQ:	3.1	9.7	9.7	0.5	1.4	1.4	3.0	2.3	4.1	5.1	7.9	7.9
85thFactor:	1.58	1.53	1.53	1.60	1.59	1.59	1.58	1.58	1.57	1.56	1.54	1.54
85thHCM2kQ:	4.1	12.5	12.5	0.6	1.8	1.8	3.9	3.1	5.4	6.7	10.3	10.3
90thFactor:	1.75	1.67	1.67	1.79	1.78	1.78	1.75	1.76	1.74	1.72	1.69	1.69
90thHCM2kQ:	4.5	13.6	13.6	0.7	2.0	2.0	4.4	3.4	6.0	7.4	11.3	11.3
95thFactor:	2.02	1.88	1.88	2.09	2.06	2.06	2.02	2.04	2.00	1.97	1.91	1.91
95thHCM2kQ:	5.2	15.4	15.4	0.8	2.4	2.4	5.1	3.9	6.8	8.5	12.8	12.8
98thFactor:	2.52	2.23	2.23	2.67	2.62	2.62	2.52	2.56	2.47	2.42	2.30	2.30
98thHCM2kQ:	6.5	18.3	18.3	1.0	3.0	3.0	6.3	5.0	8.5	10.4	15.3	15.3

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #15 Cosumnes River Boulevard/Street B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	20.4	5.3	59.5	2.4	1.8	7.4	14.5	26.4	22.6	31.4	136	4.2

Name: year 1995 composite fleet
Fuel Consumption: 54.643 pounds
8.852 gallons
Carbon Dioxide: 170.485 pounds
Carbon Monoxide: 13.635 pounds
Hydrocarbons: 2.568 pounds
Nitrogen Oxides: 0.468 pounds

Name: year 2000 composite fleet
Fuel Consumption: 51.680 pounds
8.372 gallons
Carbon Dioxide: 161.242 pounds
Carbon Monoxide: 13.402 pounds
Hydrocarbons: 2.521 pounds
Nitrogen Oxides: 0.421 pounds

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #16: Cosumnes River Boulevard/24th Street

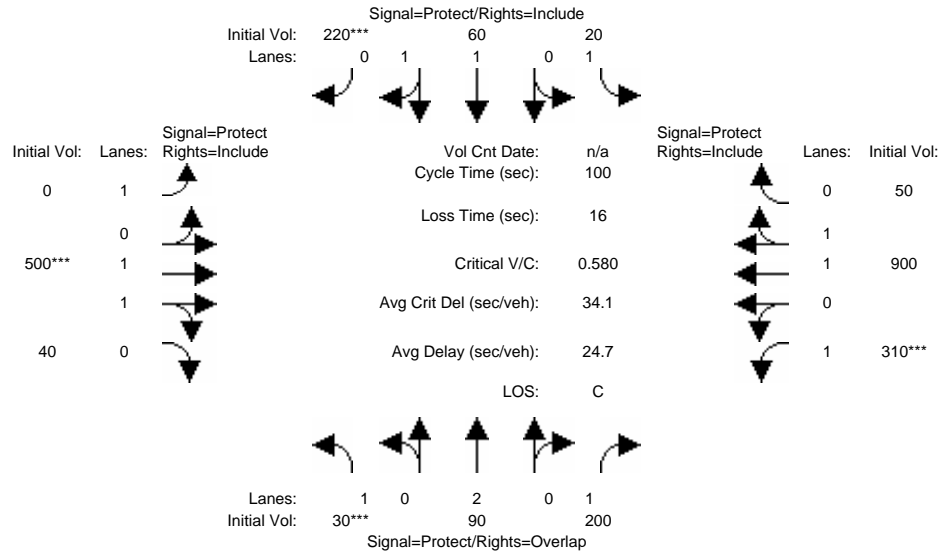


Table with 16 columns representing different approaches and movements (L, T, R). Rows include Volume Module (Base Vol, Growth Adj, etc.), Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.05	0.17	0.46	0.12	0.24	0.24	0.00	0.26	0.26	0.29	0.55	0.55
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.8	1.1	3.4	0.5	1.4	5.7	0.0	6.9	6.9	7.4	8.5	8.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.5	0.2	0.4	0.1	0.2	1.4	0.0	1.4	1.4	1.4	0.9	0.9
HCM2KQueue:	1.3	1.3	3.8	0.6	1.6	7.1	0.0	8.3	8.3	8.8	9.5	9.5
70thFactor:	1.20	1.20	1.19	1.20	1.20	1.18	1.20	1.18	1.18	1.18	1.18	1.18
70thHCM2kQ:	1.5	1.6	4.5	0.7	1.9	8.4	0.0	9.8	9.8	10.3	11.1	11.1
85thFactor:	1.59	1.59	1.56	1.59	1.58	1.54	1.60	1.53	1.53	1.52	1.52	1.52
85thHCM2kQ:	2.0	2.1	5.9	1.0	2.5	10.9	0.0	12.7	12.7	13.4	14.4	14.4
90thFactor:	1.78	1.77	1.73	1.79	1.77	1.68	1.80	1.66	1.66	1.66	1.65	1.65
90thHCM2kQ:	2.3	2.3	6.6	1.1	2.8	11.9	0.0	13.8	13.8	14.5	15.6	15.6
95thFactor:	2.06	2.06	1.99	2.08	2.05	1.90	2.10	1.88	1.88	1.87	1.85	1.85
95thHCM2kQ:	2.6	2.7	7.5	1.2	3.2	13.5	0.0	15.6	15.6	16.4	17.5	17.5
98thFactor:	2.61	2.60	2.45	2.65	2.59	2.28	2.70	2.23	2.23	2.21	2.18	2.18
98thHCM2kQ:	3.4	3.4	9.3	1.6	4.1	16.2	0.0	18.5	18.5	19.4	20.6	20.6

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #16 Cosumnes River Boulevard/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	7.2	19.2	30.8	4.5	11.9	48.9	0.0	11.0	8.8	66.3	138	7.7

Name: year 1995 composite fleet
Fuel Consumption: 71.327 pounds
11.555 gallons
Carbon Dioxide: 222.540 pounds
Carbon Monoxide: 17.448 pounds
Hydrocarbons: 3.180 pounds
Nitrogen Oxides: 0.633 pounds

Name: year 2000 composite fleet
Fuel Consumption: 66.961 pounds
10.848 gallons
Carbon Dioxide: 208.918 pounds
Carbon Monoxide: 17.107 pounds
Hydrocarbons: 3.110 pounds
Nitrogen Oxides: 0.566 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.09	0.17	0.43	0.05	0.13	0.13	0.18	0.36	0.36	0.26	0.44	0.44
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.8	1.0	9.6	0.8	1.9	1.9	4.3	9.9	9.9	7.4	8.6	8.6
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.2	0.2	1.7	0.5	0.5	0.5	1.1	1.7	1.7	1.7	1.1	1.1
HCM2KQueue:	1.0	1.1	11.3	1.3	2.3	2.3	5.3	11.6	11.6	9.1	9.7	9.7
70thFactor:	1.20	1.20	1.18	1.20	1.19	1.19	1.19	1.17	1.17	1.18	1.18	1.18
70thHCM2kQ:	1.2	1.4	13.3	1.5	2.8	2.8	6.3	13.6	13.6	10.7	11.5	11.5
85thFactor:	1.59	1.59	1.51	1.59	1.58	1.58	1.55	1.50	1.50	1.52	1.52	1.52
85thHCM2kQ:	1.6	1.8	17.0	2.0	3.7	3.7	8.3	17.4	17.4	13.8	14.8	14.8
90thFactor:	1.78	1.78	1.63	1.78	1.76	1.76	1.71	1.62	1.62	1.65	1.65	1.65
90thHCM2kQ:	1.8	2.0	18.4	2.3	4.1	4.1	9.1	18.8	18.8	15.0	16.0	16.0
95thFactor:	2.07	2.06	1.82	2.06	2.03	2.03	1.95	1.82	1.82	1.86	1.85	1.85
95thHCM2kQ:	2.1	2.4	20.6	2.6	4.7	4.7	10.4	21.0	21.0	16.9	18.0	18.0
98thFactor:	2.63	2.62	2.12	2.61	2.54	2.54	2.36	2.11	2.11	2.20	2.17	2.17
98thHCM2kQ:	2.6	3.0	24.0	3.4	5.9	5.9	12.6	24.4	24.4	19.9	21.1	21.1

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #16 Cosumnes River Boulevard/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	6.9	17.0	86.5	7.2	27.3	4.5	38.4	163	6.3	66.5	139	9.1

Name: year 1995 composite fleet
Fuel Consumption: 89.781 pounds
14.544 gallons
Carbon Dioxide: 280.115 pounds
Carbon Monoxide: 22.212 pounds
Hydrocarbons: 4.119 pounds
Nitrogen Oxides: 0.786 pounds

Name: year 2000 composite fleet
Fuel Consumption: 84.625 pounds
13.709 gallons
Carbon Dioxide: 264.031 pounds
Carbon Monoxide: 21.807 pounds
Hydrocarbons: 4.038 pounds
Nitrogen Oxides: 0.705 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP AM

Intersection #17: Cosumnes River Boulevard/Stone-Boswell Access West

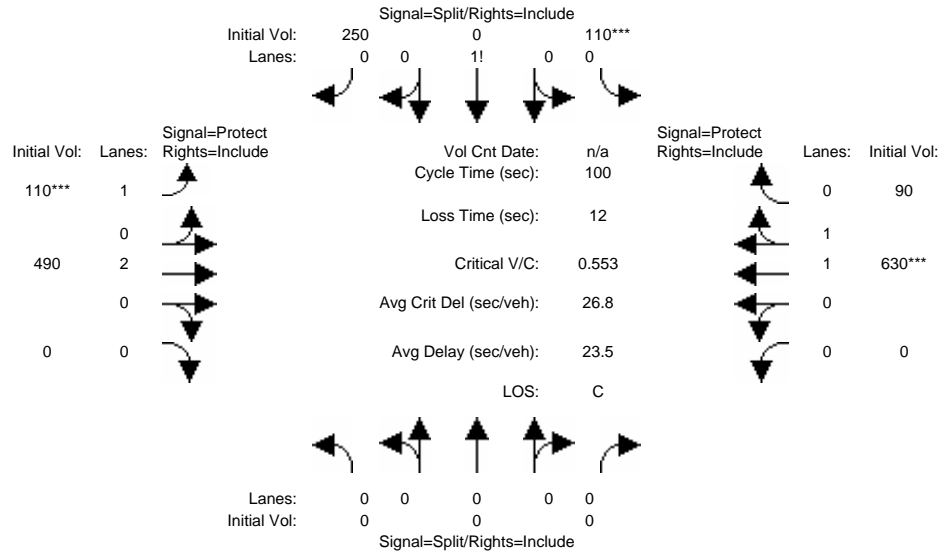


Table with multiple sections: Approach: (North, South, East, West Bound), Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.39	0.00	0.39	0.11	0.49	0.00	0.00	0.38	0.38
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	7.8	0.0	7.8	2.9	4.3	0.0	0.0	8.3	8.3
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	1.2	0.0	1.2	1.1	0.4	0.0	0.0	1.2	1.2
HCM2KQueue:	0.0	0.0	0.0	9.0	0.0	9.0	4.0	4.7	0.0	0.0	9.5	9.5
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.18	1.19	1.19	1.20	1.20	1.18	1.18
70thHCM2kQ:	0.0	0.0	0.0	10.6	0.0	10.6	4.8	5.5	0.0	0.0	11.2	11.2
85thFactor:	1.60	1.60	1.60	1.52	1.60	1.52	1.56	1.56	1.60	1.60	1.52	1.52
85thHCM2kQ:	0.0	0.0	0.0	13.6	0.0	13.6	6.3	7.2	0.0	0.0	14.4	14.4
90thFactor:	1.80	1.80	1.80	1.66	1.80	1.66	1.73	1.72	1.80	1.80	1.65	1.65
90thHCM2kQ:	0.0	0.0	0.0	14.8	0.0	14.8	6.9	8.0	0.0	0.0	15.7	15.7
95thFactor:	2.10	2.10	2.10	1.86	2.10	1.86	1.98	1.96	2.10	2.10	1.85	1.85
95thHCM2kQ:	0.0	0.0	0.0	16.7	0.0	16.7	7.9	9.1	0.0	0.0	17.6	17.6
98thFactor:	2.70	2.70	2.70	2.20	2.70	2.20	2.43	2.40	2.70	2.70	2.18	2.18
98thHCM2kQ:	0.0	0.0	0.0	19.7	0.0	19.7	9.8	11.2	0.0	0.0	20.7	20.7

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	21.3	0.0	48.5	26.0	72.8	0.0	0.0	124	17.7

Name: year 1995 composite fleet
 Fuel Consumption: 48.052 pounds
 7.784 gallons
 Carbon Dioxide: 149.922 pounds
 Carbon Monoxide: 11.698 pounds
 Hydrocarbons: 2.114 pounds
 Nitrogen Oxides: 0.431 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 45.028 pounds
 7.295 gallons
 Carbon Dioxide: 140.487 pounds
 Carbon Monoxide: 11.462 pounds
 Hydrocarbons: 2.066 pounds
 Nitrogen Oxides: 0.385 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP PM

Intersection #17: Cosumnes River Boulevard/Stone-Boswell Access West

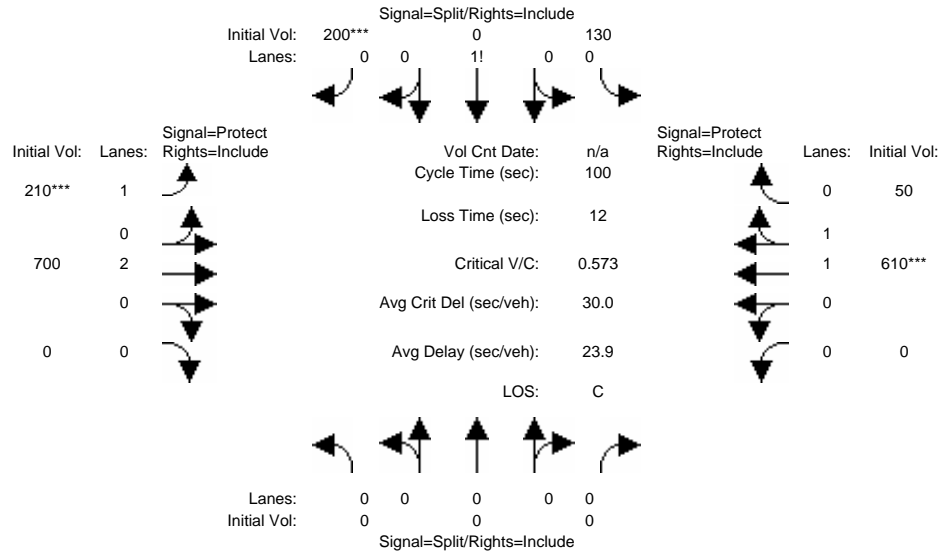


Table with columns for Approach (North, South, East, West), Movement (L, T, R), and various performance metrics including Volume Module, Saturation Flow Module, Capacity Analysis Module, and HCM Ops Adjusted Lane Utilization Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.34	0.00	0.34	0.21	0.54	0.00	0.00	0.33	0.33
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	7.5	0.0	7.5	5.2	5.9	0.0	0.0	8.0	8.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	1.3	0.0	1.3	1.3	0.6	0.0	0.0	1.3	1.3
HCM2KQueue:	0.0	0.0	0.0	8.8	0.0	8.8	6.5	6.5	0.0	0.0	9.3	9.3
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.18	1.18	1.19	1.20	1.20	1.18	1.18
70thHCM2kQ:	0.0	0.0	0.0	10.4	0.0	10.4	7.7	7.7	0.0	0.0	10.9	10.9
85thFactor:	1.60	1.60	1.60	1.52	1.60	1.52	1.54	1.54	1.60	1.60	1.52	1.52
85thHCM2kQ:	0.0	0.0	0.0	13.4	0.0	13.4	10.0	10.0	0.0	0.0	14.1	14.1
90thFactor:	1.80	1.80	1.80	1.66	1.80	1.66	1.69	1.69	1.80	1.80	1.65	1.65
90thHCM2kQ:	0.0	0.0	0.0	14.6	0.0	14.6	11.0	11.0	0.0	0.0	15.3	15.3
95thFactor:	2.10	2.10	2.10	1.87	2.10	1.87	1.92	1.92	2.10	2.10	1.86	1.86
95thHCM2kQ:	0.0	0.0	0.0	16.4	0.0	16.4	12.5	12.5	0.0	0.0	17.2	17.2
98thFactor:	2.70	2.70	2.70	2.21	2.70	2.21	2.31	2.31	2.70	2.70	2.19	2.19
98thHCM2kQ:	0.0	0.0	0.0	19.4	0.0	19.4	15.0	15.0	0.0	0.0	20.3	20.3

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	26.6	0.0	40.9	47.2	101	0.0	0.0	126	10.3

Name: year 1995 composite fleet
 Fuel Consumption: 54.960 pounds
 8.903 gallons
 Carbon Dioxide: 171.474 pounds
 Carbon Monoxide: 13.405 pounds
 Hydrocarbons: 2.430 pounds
 Nitrogen Oxides: 0.491 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 51.538 pounds
 8.349 gallons
 Carbon Dioxide: 160.797 pounds
 Carbon Monoxide: 13.137 pounds
 Hydrocarbons: 2.376 pounds
 Nitrogen Oxides: 0.438 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

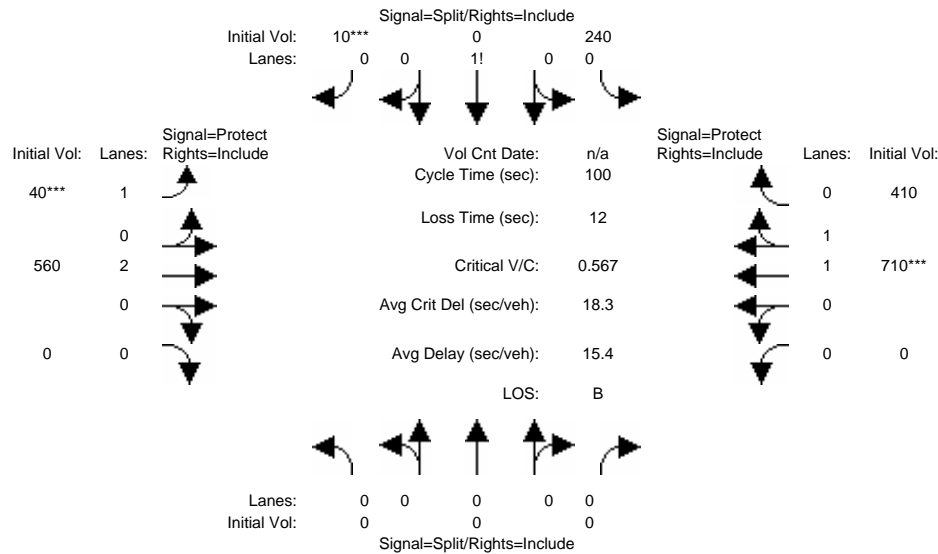


Table with columns for Approach (North, South, East, West) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.25	0.00	0.25	0.05	0.63	0.00	0.00	0.58	0.58
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	6.1	0.0	6.1	1.1	3.6	0.0	0.0	10.3	10.3
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	1.3	0.0	1.3	0.7	0.3	0.0	0.0	1.3	1.3
HCM2KQueue:	0.0	0.0	0.0	7.4	0.0	7.4	1.8	3.9	0.0	0.0	11.6	11.6
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.18	1.20	1.19	1.20	1.20	1.17	1.17
70thHCM2kQ:	0.0	0.0	0.0	8.7	0.0	8.7	2.2	4.6	0.0	0.0	13.6	13.6
85thFactor:	1.60	1.60	1.60	1.53	1.60	1.53	1.58	1.56	1.60	1.60	1.50	1.50
85thHCM2kQ:	0.0	0.0	0.0	11.3	0.0	11.3	2.9	6.1	0.0	0.0	17.4	17.4
90thFactor:	1.80	1.80	1.80	1.68	1.80	1.68	1.77	1.73	1.80	1.80	1.62	1.62
90thHCM2kQ:	0.0	0.0	0.0	12.4	0.0	12.4	3.2	6.7	0.0	0.0	18.8	18.8
95thFactor:	2.10	2.10	2.10	1.90	2.10	1.90	2.04	1.98	2.10	2.10	1.82	1.82
95thHCM2kQ:	0.0	0.0	0.0	14.0	0.0	14.0	3.7	7.7	0.0	0.0	21.0	21.0
98thFactor:	2.70	2.70	2.70	2.27	2.70	2.27	2.57	2.44	2.70	2.70	2.11	2.11
98thHCM2kQ:	0.0	0.0	0.0	16.7	0.0	16.7	4.7	9.5	0.0	0.0	24.4	24.4

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	52.7	0.0	2.2	9.7	60.9	0.0	0.0	111	64.2

Name: year 1995 composite fleet
 Fuel Consumption: 44.558 pounds
 7.218 gallons
 Carbon Dioxide: 139.019 pounds
 Carbon Monoxide: 10.402 pounds
 Hydrocarbons: 1.746 pounds
 Nitrogen Oxides: 0.424 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 41.130 pounds
 6.663 gallons
 Carbon Dioxide: 128.326 pounds
 Carbon Monoxide: 10.137 pounds
 Hydrocarbons: 1.691 pounds
 Nitrogen Oxides: 0.374 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

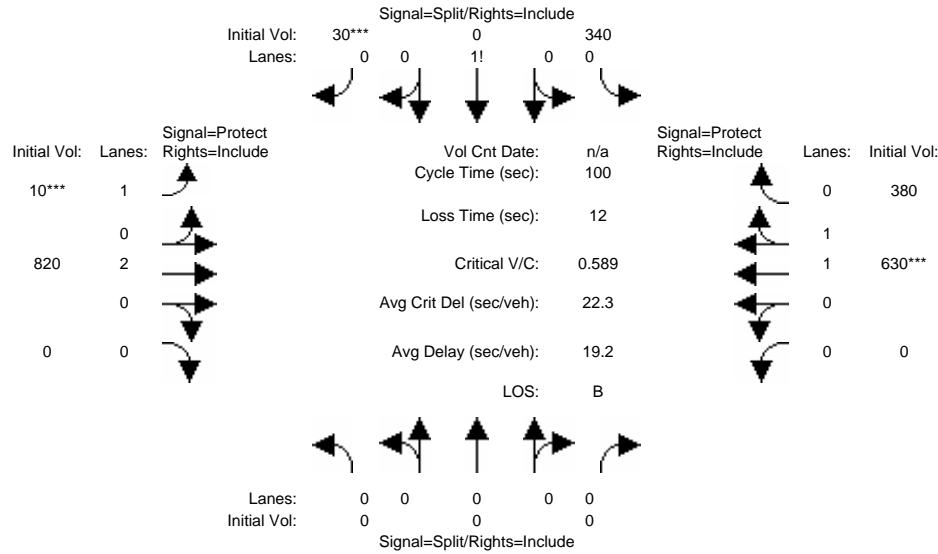


Table with columns for Approach, Movement, and various performance metrics. Includes sections for Volume Module, Saturation Flow Module, Capacity Analysis Module, and HCM Ops Adjusted Lane Utilization Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.34	0.00	0.34	0.05	0.54	0.00	0.00	0.49	0.49
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	8.6	0.0	8.6	0.3	7.2	0.0	0.0	10.8	10.8
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	1.5	0.0	1.5	0.1	0.7	0.0	0.0	1.6	1.6
HCM2KQueue:	0.0	0.0	0.0	10.1	0.0	10.1	0.4	7.9	0.0	0.0	12.4	12.4
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.18	1.20	1.18	1.20	1.20	1.17	1.17
70thHCM2kQ:	0.0	0.0	0.0	11.9	0.0	11.9	0.5	9.4	0.0	0.0	14.5	14.5
85thFactor:	1.60	1.60	1.60	1.51	1.60	1.51	1.60	1.53	1.60	1.60	1.50	1.50
85thHCM2kQ:	0.0	0.0	0.0	15.3	0.0	15.3	0.6	12.1	0.0	0.0	18.5	18.5
90thFactor:	1.80	1.80	1.80	1.64	1.80	1.64	1.79	1.67	1.80	1.80	1.62	1.62
90thHCM2kQ:	0.0	0.0	0.0	16.6	0.0	16.6	0.7	13.2	0.0	0.0	20.0	20.0
95thFactor:	2.10	2.10	2.10	1.84	2.10	1.84	2.09	1.89	2.10	2.10	1.80	1.80
95thHCM2kQ:	0.0	0.0	0.0	18.6	0.0	18.6	0.8	15.0	0.0	0.0	22.3	22.3
98thFactor:	2.70	2.70	2.70	2.16	2.70	2.16	2.67	2.24	2.70	2.70	2.09	2.09
98thHCM2kQ:	0.0	0.0	0.0	21.8	0.0	21.8	1.0	17.8	0.0	0.0	25.8	25.8

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	71.0	0.0	6.3	2.4	123	0.0	0.0	115	69.5

Name: year 1995 composite fleet
 Fuel Consumption: 56.407 pounds
 9.138 gallons
 Carbon Dioxide: 175.991 pounds
 Carbon Monoxide: 13.466 pounds
 Hydrocarbons: 2.347 pounds
 Nitrogen Oxides: 0.526 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 52.469 pounds
 8.500 gallons
 Carbon Dioxide: 163.704 pounds
 Carbon Monoxide: 13.159 pounds
 Hydrocarbons: 2.285 pounds
 Nitrogen Oxides: 0.467 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #20: Delta Shores Circle/Street C

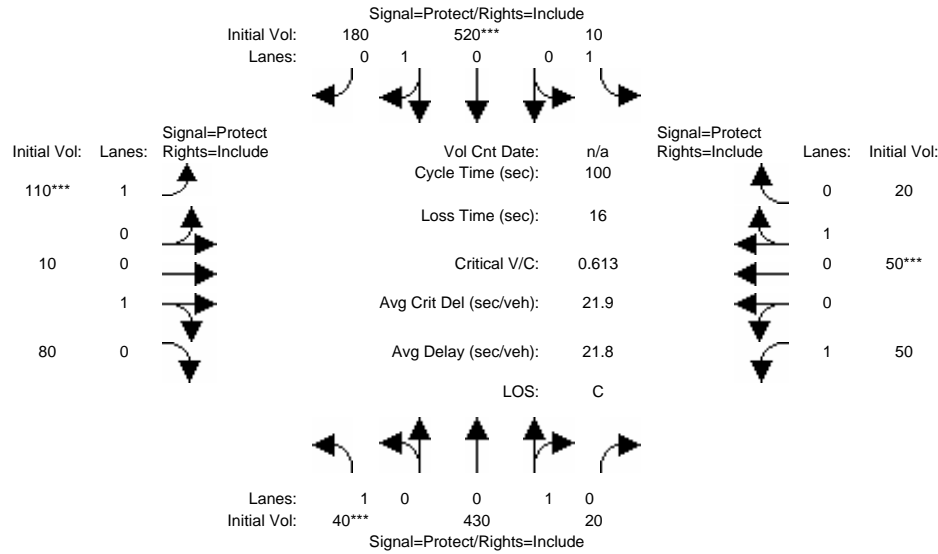


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.05	0.56	0.56	0.11	0.62	0.62	0.10	0.10	0.10	0.07	0.07	0.07
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	1.1	7.3	7.3	0.2	12.1	12.1	2.9	2.4	2.4	1.3	1.9	1.9
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.7	0.8	0.8	0.1	1.6	1.6	1.4	1.2	1.2	0.6	1.1	1.1
HCM2KQueue:	1.8	8.1	8.1	0.3	13.7	13.7	4.4	3.5	3.5	2.0	3.0	3.0
70thFactor:	1.20	1.18	1.18	1.20	1.17	1.17	1.19	1.19	1.19	1.20	1.19	1.19
70thHCM2kQ:	2.2	9.6	9.6	0.4	16.1	16.1	5.2	4.2	4.2	2.3	3.6	3.6
85thFactor:	1.58	1.53	1.53	1.60	1.49	1.49	1.56	1.57	1.57	1.58	1.57	1.57
85thHCM2kQ:	2.9	12.4	12.4	0.5	20.5	20.5	6.8	5.5	5.5	3.1	4.7	4.7
90thFactor:	1.77	1.67	1.67	1.79	1.60	1.60	1.72	1.74	1.74	1.76	1.74	1.74
90thHCM2kQ:	3.2	13.5	13.5	0.5	22.0	22.0	7.5	6.1	6.1	3.5	5.2	5.2
95thFactor:	2.04	1.88	1.88	2.09	1.78	1.78	1.97	1.99	1.99	2.04	2.01	2.01
95thHCM2kQ:	3.7	15.2	15.2	0.6	24.5	24.5	8.6	7.1	7.1	4.0	6.0	6.0
98thFactor:	2.57	2.24	2.24	2.68	2.05	2.05	2.41	2.46	2.46	2.56	2.49	2.49
98thHCM2kQ:	4.7	18.1	18.1	0.8	28.1	28.1	10.6	8.7	8.7	5.0	7.4	7.4

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	9.7	63.0	2.9	2.2	80.9	28.0	26.4	2.4	19.1	12.0	12.1	4.8

Name: year 1995 composite fleet
 Fuel Consumption: 41.511 pounds
 6.725 gallons
 Carbon Dioxide: 129.515 pounds
 Carbon Monoxide: 10.036 pounds
 Hydrocarbons: 1.795 pounds
 Nitrogen Oxides: 0.374 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 38.808 pounds
 6.287 gallons
 Carbon Dioxide: 121.080 pounds
 Carbon Monoxide: 9.826 pounds
 Hydrocarbons: 1.752 pounds
 Nitrogen Oxides: 0.333 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.09	0.53	0.53	0.08	0.53	0.53	0.15	0.13	0.13	0.09	0.07	0.07
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	3.3	11.3	11.3	0.5	15.9	15.9	5.6	2.6	2.6	0.8	1.1	1.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	2.2	1.5	1.5	0.2	3.0	3.0	2.5	0.8	0.8	0.2	0.4	0.4
HCM2KQueue:	5.5	12.7	12.7	0.7	18.9	18.9	8.1	3.3	3.3	1.0	1.5	1.5
70thFactor:	1.19	1.17	1.17	1.20	1.16	1.16	1.18	1.19	1.19	1.20	1.20	1.20
70thHCM2kQ:	6.5	15.0	15.0	0.8	22.0	22.0	9.6	4.0	4.0	1.2	1.8	1.8
85thFactor:	1.55	1.50	1.50	1.59	1.46	1.46	1.53	1.57	1.57	1.59	1.59	1.59
85thHCM2kQ:	8.5	19.1	19.1	1.1	27.6	27.6	12.4	5.2	5.2	1.6	2.4	2.4
90thFactor:	1.70	1.61	1.61	1.79	1.56	1.56	1.67	1.74	1.74	1.78	1.77	1.77
90thHCM2kQ:	9.4	20.5	20.5	1.2	29.4	29.4	13.5	5.8	5.8	1.8	2.7	2.7
95thFactor:	1.94	1.80	1.80	2.08	1.71	1.71	1.88	2.00	2.00	2.07	2.05	2.05
95thHCM2kQ:	10.7	22.9	22.9	1.4	32.3	32.3	15.3	6.6	6.6	2.0	3.1	3.1
98thFactor:	2.36	2.08	2.08	2.65	1.93	1.93	2.24	2.47	2.47	2.63	2.59	2.59
98thHCM2kQ:	12.9	26.5	26.5	1.8	36.6	36.6	18.2	8.2	8.2	2.6	3.9	3.9

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #20 Delta Shores Circle/Street C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	29.3	92.9	8.6	4.6	97.4	45.7	50.4	11.5	11.5	6.9	7.1	2.4

Name: year 1995 composite fleet
 Fuel Consumption: 60.023 pounds
 9.724 gallons
 Carbon Dioxide: 187.272 pounds
 Carbon Monoxide: 14.910 pounds
 Hydrocarbons: 2.787 pounds
 Nitrogen Oxides: 0.518 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 56.672 pounds
 9.181 gallons
 Carbon Dioxide: 176.817 pounds
 Carbon Monoxide: 14.647 pounds
 Hydrocarbons: 2.734 pounds
 Nitrogen Oxides: 0.465 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.10	0.18	0.18	0.05	0.14	0.32	0.18	0.44	0.44	0.16	0.42	0.42
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.3	5.4	5.4	0.5	1.7	5.4	5.4	7.2	7.2	3.3	11.1	11.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.1	1.7	1.7	0.3	0.4	0.9	1.7	0.8	0.8	0.8	1.8	1.8
HCM2KQueue:	0.3	7.1	7.1	0.8	2.1	6.2	7.1	8.0	8.0	4.1	12.9	12.9
70thFactor:	1.20	1.18	1.18	1.20	1.19	1.19	1.18	1.18	1.18	1.19	1.17	1.17
70thHCM2kQ:	0.4	8.4	8.4	1.0	2.5	7.4	8.4	9.4	9.4	4.8	15.1	15.1
85thFactor:	1.60	1.54	1.54	1.59	1.58	1.54	1.54	1.53	1.53	1.56	1.50	1.50
85thHCM2kQ:	0.5	10.9	10.9	1.3	3.3	9.6	10.9	12.2	12.2	6.3	19.2	19.2
90thFactor:	1.79	1.68	1.68	1.78	1.76	1.69	1.68	1.67	1.67	1.73	1.61	1.61
90thHCM2kQ:	0.6	11.9	11.9	1.5	3.7	10.6	11.9	13.3	13.3	7.0	20.7	20.7
95thFactor:	2.09	1.90	1.90	2.07	2.03	1.92	1.90	1.89	1.89	1.98	1.79	1.79
95thHCM2kQ:	0.7	13.5	13.5	1.7	4.3	12.0	13.5	15.0	15.0	8.0	23.1	23.1
98thFactor:	2.68	2.28	2.28	2.64	2.55	2.32	2.28	2.24	2.24	2.43	2.07	2.07
98thHCM2kQ:	0.8	16.2	16.2	2.2	5.4	14.5	16.2	17.9	17.9	9.9	26.6	26.6

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	2.3	27.8	20.9	4.8	15.7	48.2	48.7	62.7	1.7	29.4	79.7	19.9

Name: year 1995 composite fleet
 Fuel Consumption: 59.283 pounds
 9.604 gallons
 Carbon Dioxide: 184.961 pounds
 Carbon Monoxide: 14.778 pounds
 Hydrocarbons: 2.778 pounds
 Nitrogen Oxides: 0.509 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 56.046 pounds
 9.079 gallons
 Carbon Dioxide: 174.864 pounds
 Carbon Monoxide: 14.523 pounds
 Hydrocarbons: 2.727 pounds
 Nitrogen Oxides: 0.457 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.11	0.22	0.22	0.05	0.16	0.39	0.23	0.44	0.44	0.13	0.34	0.34
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.2	6.7	6.7	1.6	2.5	6.3	7.1	9.0	9.0	3.4	10.5	10.5
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	0.1	2.0	2.0	1.5	0.5	0.9	2.0	1.2	1.2	1.1	2.1	2.1
HCM2KQueue:	0.3	8.7	8.7	3.1	3.0	7.3	9.2	10.2	10.2	4.5	12.6	12.6
70thFactor:	1.20	1.18	1.18	1.19	1.19	1.18	1.18	1.18	1.18	1.19	1.17	1.17
70thHCM2kQ:	0.4	10.2	10.2	3.7	3.6	8.6	10.8	12.0	12.0	5.3	14.8	14.8
85thFactor:	1.60	1.52	1.52	1.57	1.57	1.54	1.52	1.51	1.51	1.56	1.50	1.50
85thHCM2kQ:	0.5	13.2	13.2	4.9	4.7	11.2	13.9	15.5	15.5	7.0	18.9	18.9
90thFactor:	1.79	1.66	1.66	1.74	1.74	1.68	1.65	1.64	1.64	1.72	1.61	1.61
90thHCM2kQ:	0.5	14.4	14.4	5.4	5.2	12.2	15.1	16.8	16.8	7.7	20.4	20.4
95thFactor:	2.09	1.87	1.87	2.00	2.01	1.90	1.86	1.84	1.84	1.97	1.80	1.80
95thHCM2kQ:	0.6	16.2	16.2	6.3	6.0	13.8	17.0	18.8	18.8	8.8	22.7	22.7
98thFactor:	2.68	2.21	2.21	2.49	2.50	2.27	2.19	2.16	2.16	2.41	2.08	2.08
98thHCM2kQ:	0.8	19.2	19.2	7.8	7.4	16.5	20.1	22.0	22.0	10.8	26.3	26.3

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #21 Delta Shores Circle/Street A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	2.2	25.3	34.5	14.8	22.3	56.9	64.2	79.6	1.9	30.3	90.6	4.3

Name: year 1995 composite fleet
 Fuel Consumption: 71.678 pounds
 11.612 gallons
 Carbon Dioxide: 223.636 pounds
 Carbon Monoxide: 17.989 pounds
 Hydrocarbons: 3.420 pounds
 Nitrogen Oxides: 0.606 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 67.942 pounds
 11.007 gallons
 Carbon Dioxide: 211.978 pounds
 Carbon Monoxide: 17.695 pounds
 Hydrocarbons: 3.360 pounds
 Nitrogen Oxides: 0.546 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #22: Delta Shores Circle North/24th Street

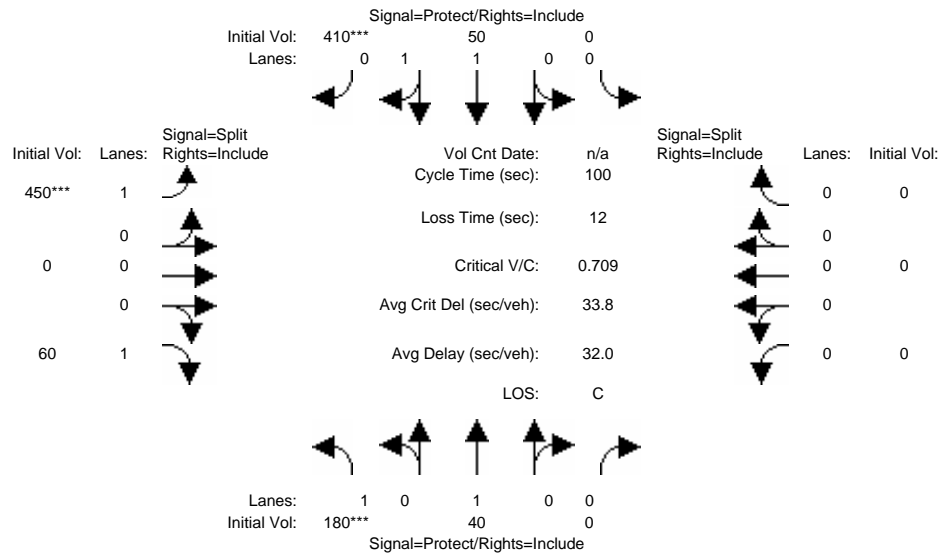


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.14	0.52	0.00	0.00	0.38	0.38	0.36	0.00	0.36	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.8	0.5	0.0	0.0	0.9	10.2	10.7	0.0	1.1	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Q2:	2.0	0.0	0.0	0.0	0.1	2.2	2.2	0.0	0.1	0.0	0.0	0.0
HCM2KQueue:	6.8	0.6	0.0	0.0	1.0	12.4	13.0	0.0	1.2	0.0	0.0	0.0
70thFactor:	1.18	1.20	1.20	1.20	1.20	1.17	1.17	1.20	1.20	1.20	1.20	1.20
70thHCM2kQ:	8.0	0.7	0.0	0.0	1.2	14.6	15.2	0.0	1.5	0.0	0.0	0.0
85thFactor:	1.54	1.59	1.60	1.60	1.59	1.50	1.49	1.60	1.59	1.60	1.60	1.60
85thHCM2kQ:	10.4	0.9	0.0	0.0	1.6	18.6	19.4	0.0	2.0	0.0	0.0	0.0
90thFactor:	1.69	1.79	1.80	1.80	1.78	1.62	1.61	1.80	1.78	1.80	1.80	1.80
90thHCM2kQ:	11.4	1.0	0.0	0.0	1.8	20.0	20.9	0.0	2.2	0.0	0.0	0.0
95thFactor:	1.91	2.08	2.10	2.10	2.07	1.80	1.79	2.10	2.06	2.10	2.10	2.10
95thHCM2kQ:	12.9	1.2	0.0	0.0	2.1	22.3	23.2	0.0	2.5	0.0	0.0	0.0
98thFactor:	2.29	2.66	2.70	2.70	2.62	2.09	2.07	2.70	2.61	2.70	2.70	2.70
98thHCM2kQ:	15.5	1.6	0.0	0.0	2.7	25.9	26.8	0.0	3.2	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	42.9	4.9	0.0	0.0	8.0	87.1	96.7	0.0	10.0	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 41.421 pounds
 6.710 gallons
 Carbon Dioxide: 129.234 pounds
 Carbon Monoxide: 10.369 pounds
 Hydrocarbons: 1.963 pounds
 Nitrogen Oxides: 0.352 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 39.223 pounds
 6.354 gallons
 Carbon Dioxide: 122.377 pounds
 Carbon Monoxide: 10.197 pounds
 Hydrocarbons: 1.928 pounds
 Nitrogen Oxides: 0.317 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #22: Delta Shores Circle North/24th Street

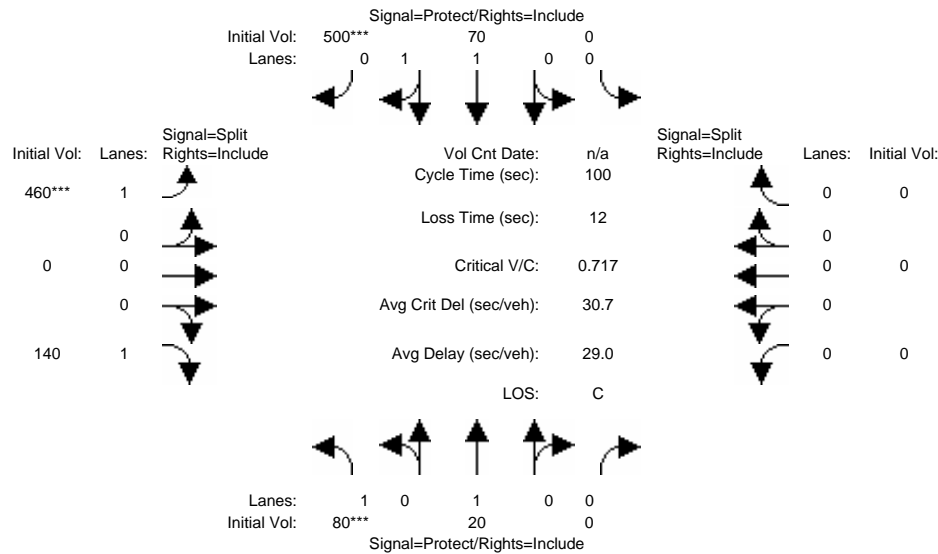


Table of traffic engineering data including: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.06	0.52	0.00	0.00	0.45	0.45	0.36	0.00	0.36	0.00	0.00	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	2.2	0.3	0.0	0.0	1.2	11.8	11.0	0.0	2.7	0.0	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Q2:	1.8	0.0	0.0	0.0	0.1	2.3	2.3	0.0	0.3	0.0	0.0	0.0
HCM2KQueue:	3.9	0.3	0.0	0.0	1.3	14.2	13.3	0.0	3.0	0.0	0.0	0.0
70thFactor:	1.19	1.20	1.20	1.20	1.20	1.17	1.17	1.20	1.19	1.20	1.20	1.20
70thHCM2kQ:	4.7	0.4	0.0	0.0	1.5	16.6	15.6	0.0	3.6	0.0	0.0	0.0
85thFactor:	1.56	1.60	1.60	1.60	1.59	1.49	1.49	1.60	1.57	1.60	1.60	1.60
85thHCM2kQ:	6.2	0.5	0.0	0.0	2.0	21.1	19.9	0.0	4.8	0.0	0.0	0.0
90thFactor:	1.73	1.79	1.80	1.80	1.78	1.60	1.61	1.80	1.74	1.80	1.80	1.80
90thHCM2kQ:	6.8	0.5	0.0	0.0	2.3	22.6	21.4	0.0	5.3	0.0	0.0	0.0
95thFactor:	1.98	2.09	2.10	2.10	2.06	1.77	1.79	2.10	2.01	2.10	2.10	2.10
95thHCM2kQ:	7.8	0.6	0.0	0.0	2.6	25.1	23.8	0.0	6.1	0.0	0.0	0.0
98thFactor:	2.44	2.68	2.70	2.70	2.61	2.04	2.06	2.70	2.49	2.70	2.70	2.70
98thHCM2kQ:	9.6	0.8	0.0	0.0	3.3	28.8	27.4	0.0	7.6	0.0	0.0	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #22 Delta Shores Circle North/24th Street

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	19.6	2.4	0.0	0.0	10.0	101.2	99.0	0.0	24.5	0.0	0.0	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 41.399 pounds
 6.707 gallons
 Carbon Dioxide: 129.164 pounds
 Carbon Monoxide: 10.274 pounds
 Hydrocarbons: 1.917 pounds
 Nitrogen Oxides: 0.359 pounds

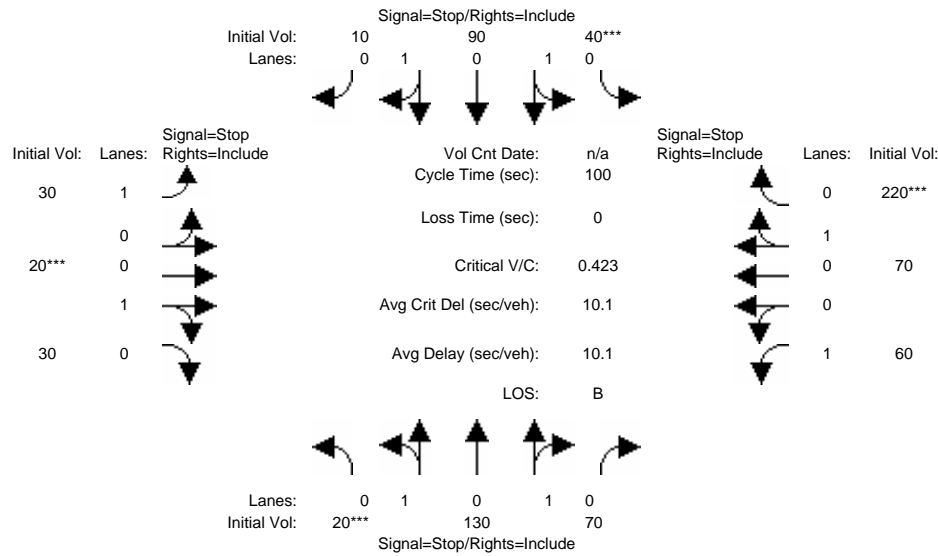
Name: year 2000 composite fleet
 Fuel Consumption: 39.071 pounds
 6.330 gallons
 Carbon Dioxide: 121.903 pounds
 Carbon Monoxide: 10.091 pounds
 Hydrocarbons: 1.880 pounds
 Nitrogen Oxides: 0.322 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (alternative)
Baseline WP AM

Intersection #23: Street C/ 24th Street



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	0 0 0	0 0 0	0 0 0	0 0 0
Volume Module:				
Base Vol:	20 130 70	40 90 10	30 20 30	60 70 220
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	20 130 70	40 90 10	30 20 30	60 70 220
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Volume:	20 130 70	40 90 10	30 20 30	60 70 220
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Reduced Vol:	20 130 70	40 90 10	30 20 30	60 70 220
PCE Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
MLF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Final Vol.:	20 130 70	40 90 10	30 20 30	60 70 220
Saturation Flow Module:				
Adjustment:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Lanes:	0.18 1.18 0.64	0.57 1.29 0.14	1.00 0.40 0.60	1.00 0.24 0.76
Final Sat.:	106 706 404	314 737 84	532 247 370	570 166 520
Capacity Analysis Module:				
Vol/Sat:	0.19 0.18 0.17	0.13 0.12 0.12	0.06 0.08 0.08	0.11 0.42 0.42
Crit Moves:	****	****	****	****
Delay/Veh:	9.9 9.7 9.1	9.7 9.4 9.2	9.5 8.6 8.6	9.5 11.2 11.2
Delay Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
AdjDel/Veh:	9.9 9.7 9.1	9.7 9.4 9.2	9.5 8.6 8.6	9.5 11.2 11.2
LOS by Move:	A A A	A A A	A A A	A B B
ApproachDel:	9.5	9.5	8.9	10.9
Delay Adj:	1.00	1.00	1.00	1.00
ApprAdjDel:	9.5	9.5	8.9	10.9
LOS by Appr:	A	A	A	B
AllWayAvgQ:	0.2 0.2 0.2	0.1 0.1 0.1	0.1 0.1 0.1	0.1 0.7 0.7
Time Period: 0.25 hour				
HevVeh:	2%	2%	2%	2%
Alpha Value: 0.01				
GroupType:	5	5	5	5
P[C1]:	0.36	0.32	0.28	0.47
P[C2]:	0.10	0.15	0.25	0.07
P[C3]:	0.37	0.33	0.21	0.35
P[C4]:	0.15	0.19	0.22	0.11
P[C5]:	0.01	0.02	0.03	0.01
Padj[C1]:	0.013	0.014	0.015	0.011
Padj[C2]:	0.006	0.006	0.005	0.005
Padj[C3]:	-0.009	-0.008	-0.003	-0.009
Padj[C4]:	-0.009	-0.011	-0.013	-0.007
Padj[C5]:	-0.001	-0.002	-0.003	-0.001
Lanes:	L1 L2	L1 L2	L1 L2	L1 L2
LaneType:	LTTHRU RTTHRU	LTTHRU RTTHRU	LEFT RTTHRU	LEFT RTTHRU
HeadwayAdj:	0.125 -0.411	0.320 -0.066	0.534 -0.386	0.534 -0.497
Volume:	110 110	70 70	30 50	60 290
Capacity:	581 635	550 585	532 617	570 686
DegOfUtil:	0.18 0.16	0.12 0.11	0.05 0.08	0.10 0.41
DepHeadway:	5.90 5.36	6.18 5.80	6.40 5.48	6.10 5.07
ServiceTime:	3.6 3.1	3.9 3.5	4.1 3.2	3.8 2.8
Delay:	9.9 9.1	9.7 9.2	9.5 8.6	9.5 11.2
Queue:	0.2 0.2	0.1 0.1	0.1 0.1	0.1 0.7
Approach:	North Bound	South Bound	East Bound	West Bound
ApproachDel:	9.5	9.5	8.9	10.9
Delay Adj:	1.00	1.00	1.00	1.00
ApprAdjDel:	9.5	9.5	8.9	10.9
LOS by Appr:	A	A	A	B
OverallDel:			10.1	
OverallLOS:			B	

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Movement:  L - T - R  L - T - R  L - T - R  L - T - R
-----|-----|-----|-----|
Control:    Stop Sign  Stop Sign  Stop Sign  Stop Sign
Lanes:      0 1 0 1 0  0 1 0 1 0  1 0 0 1 0  1 0 0 1 0
Initial Vol: 20 130 70  40 90 10  30 20 30  60 70 220
-----|-----|-----|-----|
Major Street Volume: 430
Minor Approach Volume: 220
Minor Approach Volume Threshold: 737
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SIGNAL WARRANT DISCLAIMER

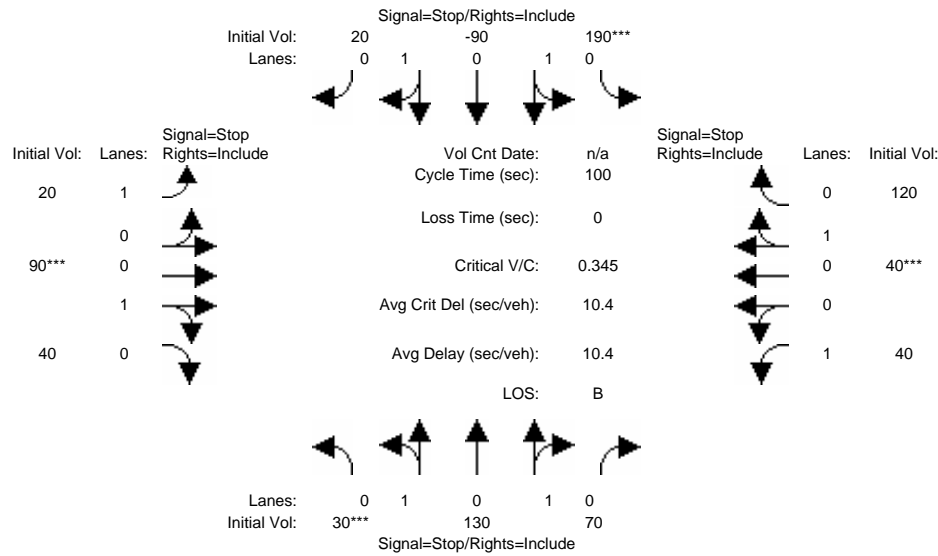
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (alternative)
Baseline WP PM

Intersection #23: Street C/ 24th Street



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	0 0 0	0 0 0	0 0 0	0 0 0
Volume Module:				
Base Vol:	30 130 70	190 -90 20	20 90 40	40 40 120
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	30 130 70	190 -90 20	20 90 40	40 40 120
User Adj:	1.00 1.00 1.00	1.00 0.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	1.00 1.00 1.00	1.00 0.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Volume:	30 130 70	190 0 20	20 90 40	40 40 120
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Reduced Vol:	30 130 70	190 0 20	20 90 40	40 40 120
PCE Adj:	1.00 1.00 1.00	1.00 0.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
MLF Adj:	1.00 1.00 1.00	1.00 0.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Final Vol.:	30 130 70	190 0 20	20 90 40	40 40 120
Saturation Flow Module:				
Adjustment:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Lanes:	0.26 1.13 0.61	1.00 0.81 0.19	1.00 0.69 0.31	1.00 0.25 0.75
Final Sat.:	149 665 379	551 492 116	506 391 174	513 151 452
Capacity Analysis Module:				
Vol/Sat:	0.20 0.20 0.18	0.34 0.00 0.17	0.04 0.23 0.23	0.08 0.27 0.27
Crit Moves:	****	****	****	****
Delay/Veh:	10.2 9.9 9.3	12.1 9.5 9.5	9.7 10.3 10.3	9.9 10.2 10.2
Delay Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
AdjDel/Veh:	10.2 9.9 9.3	12.1 9.5 9.5	9.7 10.3 10.3	9.9 10.2 10.2
LOS by Move:	B A A	B A A	A B B	A B B
ApproachDel:	9.8	11.2	10.2	10.1
Delay Adj:	1.00	1.00	1.00	1.00
ApprAdjDel:	9.8	11.2	10.2	10.1
LOS by Appr:	A	B	B	B
AllWayAvgQ:	0.2 0.2 0.2	0.5 0.2 0.2	0.0 0.3 0.3	0.1 0.3 0.3
Time Period: 0.25 hour				
HevVeh:	2%	2%	2%	2%
Alpha Value: 0.01				
GroupType:	5	5	5	5
P[C1]:	0.29	0.35	0.26	0.28
P[C2]:	0.23	0.18	0.11	0.09
P[C3]:	0.22	0.27	0.33	0.36
P[C4]:	0.22	0.18	0.25	0.23
P[C5]:	0.03	0.02	0.05	0.04
Padj[C1]:	0.015	0.014	0.017	0.017
Padj[C2]:	0.005	0.005	0.009	0.008
Padj[C3]:	-0.004	-0.006	-0.007	-0.008
Padj[C4]:	-0.013	-0.011	-0.015	-0.013
Padj[C5]:	-0.003	-0.002	-0.005	-0.004
Lanes:	L1 L2	L1 L2	L1 L2	L1 L2
LaneType:	LTTHRU RTTHRU	LTTHRU RTTHRU	LEFT RTTHRU	LEFT RTTHRU
HeadwayAdj:	0.164 -0.392	0.534 -0.099	0.534 -0.181	0.534 -0.491
Volume:	115 115	190 105	20 130	40 160
Capacity:	569 623	551 608	506 565	513 603
DegOfUtil:	0.19 0.18	0.33 0.17	0.04 0.22	0.07 0.25
DepHeadway:	6.04 5.49	6.31 5.67	6.71 5.99	6.64 5.62
ServiceTime:	3.7 3.2	4.0 3.4	4.4 3.7	4.3 3.3
Delay:	10.2 9.3	12.1 9.5	9.7 10.3	9.9 10.2
Queue:	0.2 0.2	0.5 0.2	0.0 0.3	0.1 0.3
Approach:	North Bound	South Bound	East Bound	West Bound
ApproachDel:	9.8	11.2	10.2	10.1
Delay Adj:	1.00	1.00	1.00	1.00
ApprAdjDel:	9.8	11.2	10.2	10.1
LOS by Appr:	A	B	B	B
OverallDel:			10.4	
OverallLOS:			B	

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Movement:  L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Stop Sign   Stop Sign   Stop Sign   Stop Sign
Lanes:      0 1 0 1 0   0 1 0 1 0   1 0 0 1 0   1 0 0 1 0
Initial Vol: 30 130 70 190 -90 20 20 90 40 40 40 120
-----|-----|-----|-----|
Major Street Volume:          350
Minor Approach Volume:        200
Minor Approach Volume Threshold: 826
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SIGNAL WARRANT DISCLAIMER

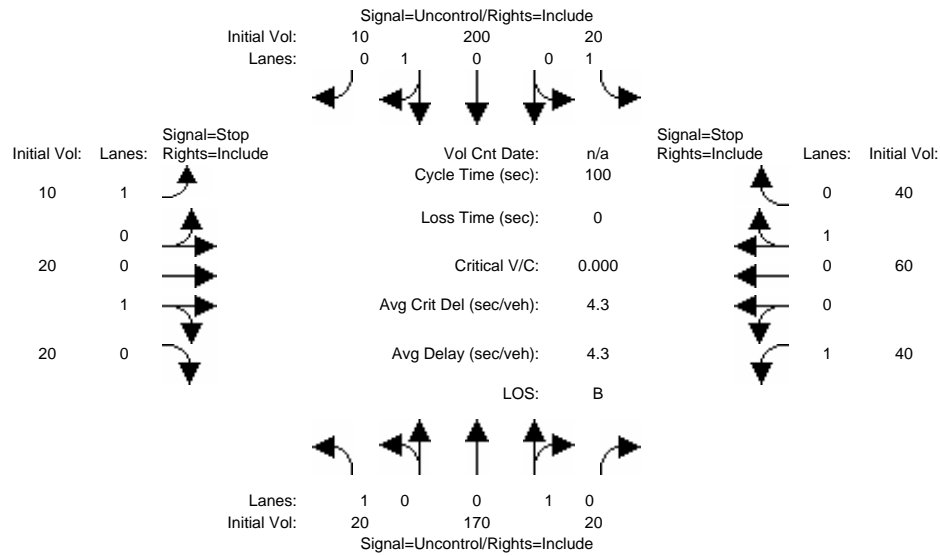
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline WP AM

Intersection #24: Street A/Street C



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	20	170	20	20	200	10	10	20	20	40	60	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	20	170	20	20	200	10	10	20	20	40	60	40
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	20	170	20	20	200	10	10	20	20	40	60	40
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	20	170	20	20	200	10	10	20	20	40	60	40
Critical Gap Module:												
Critical Gap:	4.1	xxxx	xxxx	4.1	xxxx	xxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxx	2.2	xxxx	xxxx	3.5	4.0	3.3	3.5	4.0	3.3
Capacity Module:												
Conflict Vol:	210	xxxx	xxxx	190	xxxx	xxxx	515	475	205	485	470	180
Potent Cap.:	1361	xxxx	xxxx	1384	xxxx	xxxx	470	488	836	492	492	863
Move Cap.:	1361	xxxx	xxxx	1384	xxxx	xxxx	397	474	836	455	477	863
Volume/Cap:	0.01	xxxx	xxxx	0.01	xxxx	xxxx	0.03	0.04	0.02	0.09	0.13	0.05
Level of Service Module:												
2Way95thQ:	0.0	xxxx	xxxx	0.0	xxxx	xxxx	0.1	xxxx	xxxx	0.3	xxxx	xxxx
Control Del:	7.7	xxxx	xxxx	7.6	xxxx	xxxx	14.3	xxxx	xxxx	13.7	xxxx	xxxx
LOS by Move:	A	*	*	A	*	*	B	*	*	B	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	605	xxxx	xxxx	581	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.2	xxxx	xxxx	0.6	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	11.4	xxxx	xxxx	12.5	xxxx	xxxx
Shared LOS:	*	*	*	*	*	*	B	*	*	B	*	*
ApproachDel:	xxxxxx	xxxxxx		12.0		12.8						
ApproachLOS:	*	*		B		B						
HevVeh:	2%	2%		2%		2%						
Grade:	0%	0%		0%		0%						
Peds/Hour:	0	0		0		0						
Pedestrian Walk Speed:	4.00 feet/sec											
LaneWidth:	12 feet			12 feet			12 feet			12 feet		
Time Period:	0.25 hour											

Peak Hour Delay Signal Warrant Report

 Intersection #24 Street A/Street C

 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Lanes:	1	0	1	0	1	0	1	0	1	1	0	1
Initial Vol:	20	170	20	20	200	10	10	20	20	40	60	40
ApproachDel:	xxxxxx			xxxxxx			12.0			12.8		

Approach[eastbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.2]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=50]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=630]
 FAIL - Total volume less than 800 for intersection with four or more approaches.

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=140]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=630]
 FAIL - Total volume less than 800 for intersection with four or more approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #24 Street A/Street C
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:      1 0 0 1 0   1 0 0 1 0   1 0 0 1 0   1 0 0 1 0
Initial Vol: 20 170 20  20 200 10  10 20 20  40 60 40
-----|-----|-----|-----|
Major Street Volume:      440
Minor Approach Volume:    140
Minor Approach Volume Threshold: 727
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP PM

Intersection #24: Street A/Street C

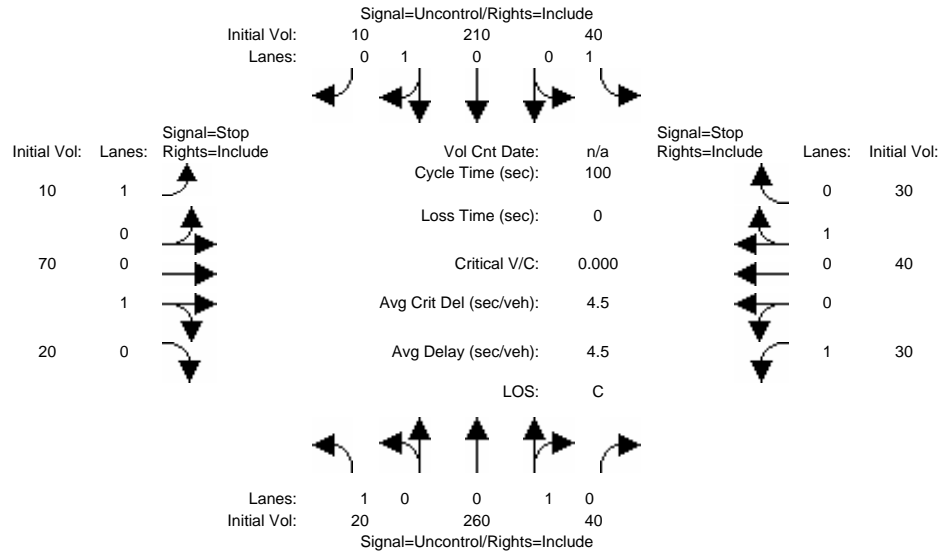


Table with columns for Approach, Movement, Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module. It contains detailed traffic engineering data for the intersection.

Peak Hour Delay Signal Warrant Report
Intersection #24 Street A/Street C
Base Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound, South Bound, East Bound, West Bound
Control: Uncontrolled, Uncontrolled, Stop Sign, Stop Sign
Signal Warrant Rule #1: [vehicle-hours=0.4]
Signal Warrant Rule #2: [approach volume=100]
Signal Warrant Rule #3: [approach count=4][total volume=780]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

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Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #24 Street A/Street C
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:      1 0 0 1 0    1 0 0 1 0    1 0 0 1 0    1 0 0 1 0
Initial Vol: 20 260 40    40 210 10    10 70 20    30 40 30
-----|-----|-----|-----|-----|
Major Street Volume:           580
Minor Approach Volume:         100
Minor Approach Volume Threshold: 608
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP AM

Intersection #25: Street C/Street B

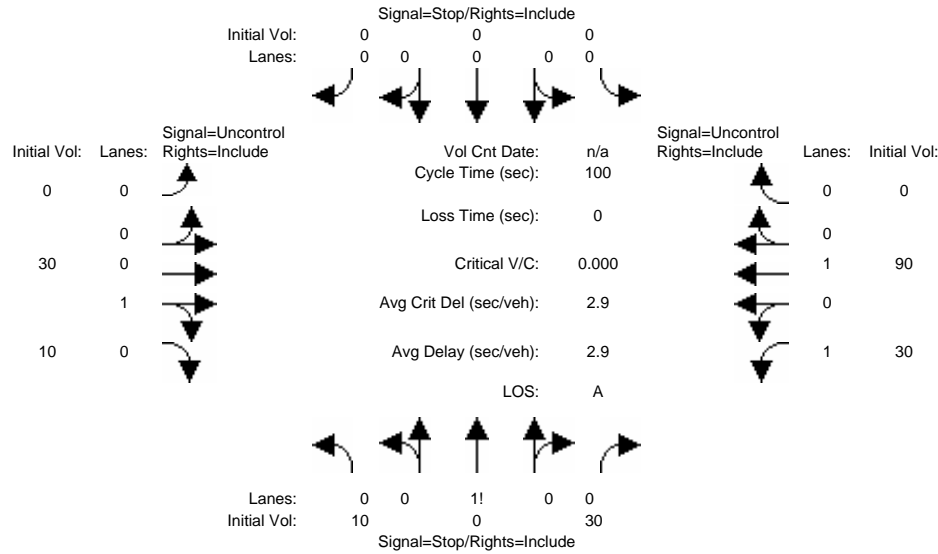


Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include: Approach, Movement, Volume Module (Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Vol), Critical Gap Module (Critical Gap, FollowUpTim), Capacity Module (Chnflct Vol, Potent Cap, Move Cap, Volume/Cap), Level of Service Module (2Way95thQ, Control Del, LOS by Move, Shared Cap, Shrd Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS), and other metrics like HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report

Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 1 0 1 0 1 0 0
Initial Vol: 10 0 30 0 0 0 0 0 30 10 30 90 0
ApproachDel: 8.9 xxxxxx xxxxxx xxxxxx
Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=40]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=200]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	0	1	0	1
Initial Vol:	10	0	30	0	0	0	0	30	10	30	90	0
Major Street Volume:	160											
Minor Approach Volume:	40											
Minor Approach Volume Threshold:	916											

SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP PM

Intersection #25: Street C/Street B

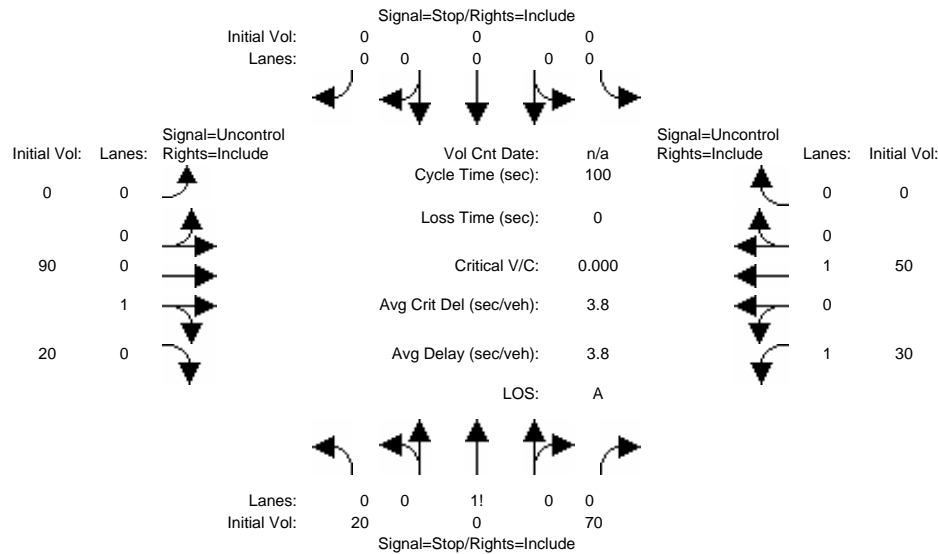


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #25 Street C/Street B
Base Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Signal Warrant Rule #1: [vehicle-hours=0.2]
Signal Warrant Rule #2: [approach volume=90]
Signal Warrant Rule #3: [approach count=3][total volume=280]
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	20	0	70	0	0	0	0	90	20	30	50	0
Major Street Volume:	190											
Minor Approach Volume:	90											
Minor Approach Volume Threshold:	857											

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline WP AM

Intersection #26: Street D/Street E

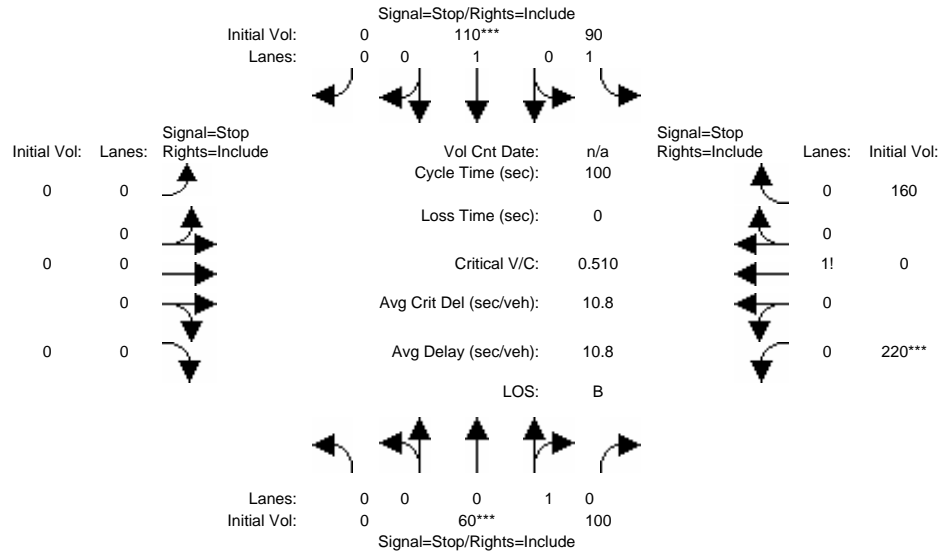


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, Time Period, and various performance metrics like Delay/Veh, LOS, and Queue. Includes a summary section for Peak Hour Volume Signal Warrant Report [Urban].


```

Movement:  L - T - R  L - T - R  L - T - R  L - T - R
-----|-----|-----|-----|
Control:    Stop Sign  Stop Sign  Stop Sign  Stop Sign
Lanes:      0 0 0 1 0  1 0 1 0 0  0 0 0 0 0  0 0 1 0 0
Initial Vol: 0 60 100  90 110  0  0 0 0 0  220 0 160
-----|-----|-----|-----|
Major Street Volume: 380
Minor Approach Volume: 200
Minor Approach Volume Threshold: 597
-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline WP PM

Intersection #26: Street D/Street E

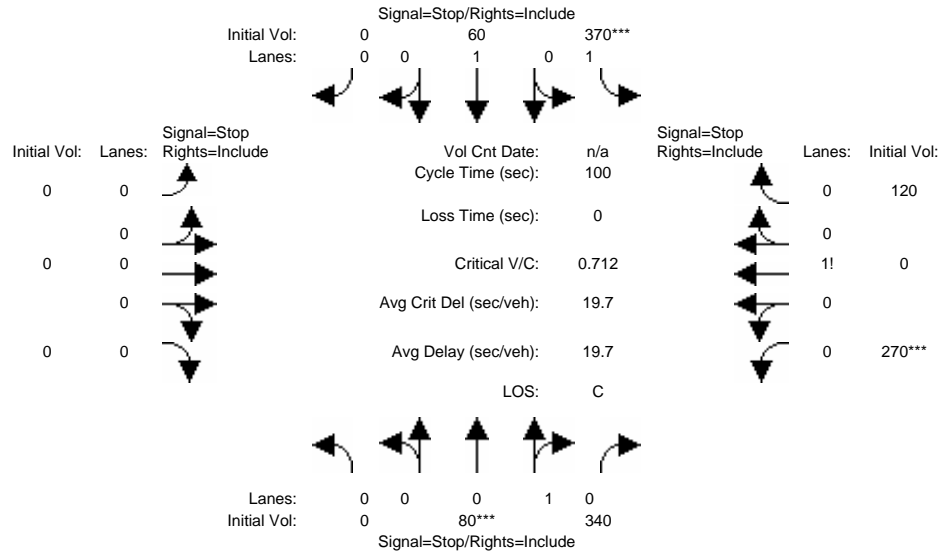


Table containing traffic engineering data including: Approach, Movement, Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, Time Period, Alpha Value, GroupType, P, Padj, Lanes, LaneType, HeadwayAdj, Volume, Capacity, DegOfUtil, DepHeadway, ServiceTime, Delay, Queue, and OverallLOS for North, South, East, and West bounds.

```

Movement:  L - T - R  L - T - R  L - T - R  L - T - R
-----|-----|-----|-----|
Control:    Stop Sign  Stop Sign  Stop Sign  Stop Sign
Lanes:      0 0 0 1 0  1 0 1 0 0  0 0 0 0 0  0 0 1 0 0
Initial Vol: 0 80 340 370 60 0  0 0 0 0 0  0 0 0 0 270 0 120
-----|-----|-----|-----|
Major Street Volume:      850
Minor Approach Volume:    390
Minor Approach Volume Threshold: 341
-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP AM

Intersection #27: Street E/Street G

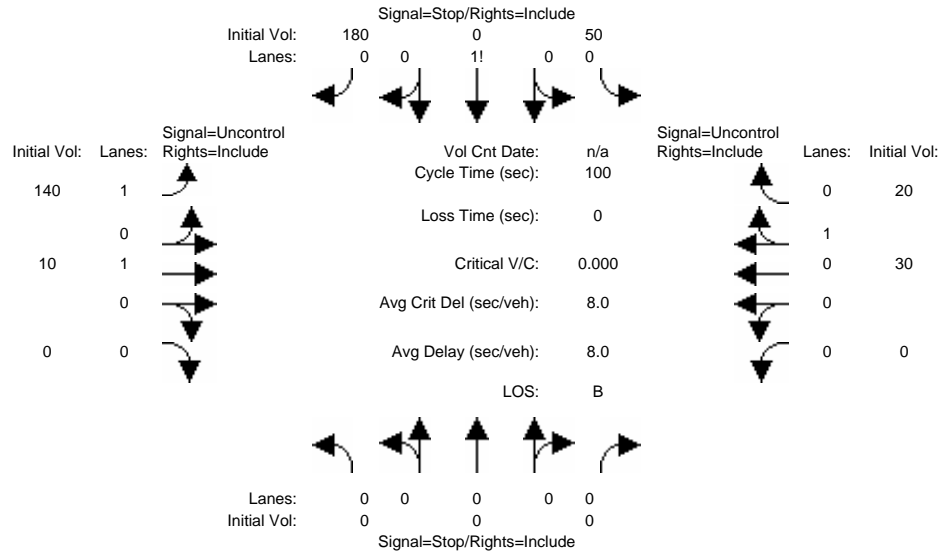


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module (Base Vol, Growth Adj, etc.), Critical Gap Module, Capacity Module, and Level of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #27 Street E/Street G
Base Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Signal Warrant Rule #1: [vehicle-hours=0.7] FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=230] SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=430] FAIL - Total volume less than 650 for intersection with less than four approaches.
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.
Peak Hour Volume Signal Warrant Report [Urban]
Intersection #27 Street E/Street G

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	0	0	0	0	1	0	1	0	1	0	0	0	0	1
Initial Vol:	0	0	0	0	50	0	180		140	10	0		0	30	20	
Major Street Volume:	200															
Minor Approach Volume:	230															
Minor Approach Volume Threshold:	839															

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP PM

Intersection #27: Street E/Street G

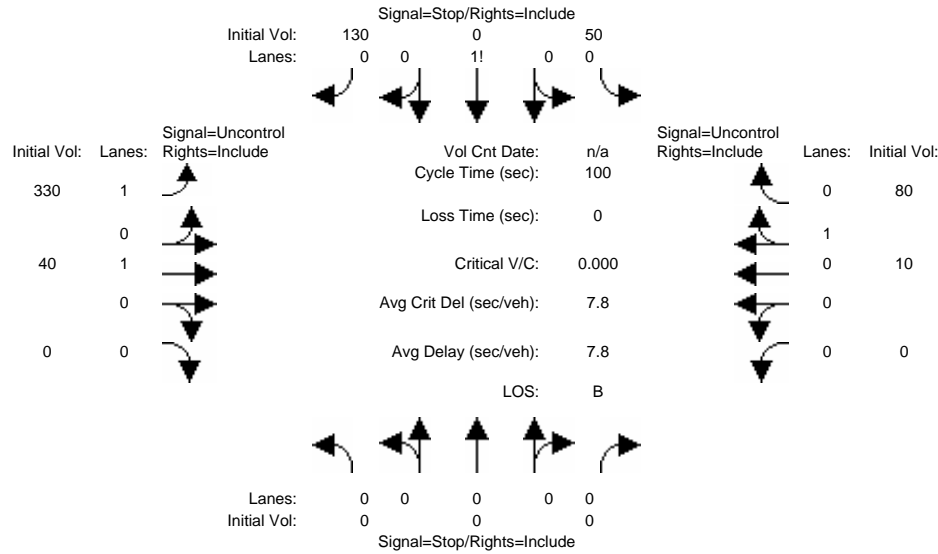


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #27 Street E/Street G
Base Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Signal Warrant Rule #1: [vehicle-hours=0.7]
Signal Warrant Rule #2: [approach volume=180]
Signal Warrant Rule #3: [approach count=3][total volume=640]
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	0	0	0	0	1	0	1	0	1	0	0	0	0	1
Initial Vol:	0	0	0	0	50	0	130		330	40	0		0	10	80	
Major Street Volume:	460															
Minor Approach Volume:	180															
Minor Approach Volume Threshold:	552															

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #28: Delta Shores Circle South/Street D South

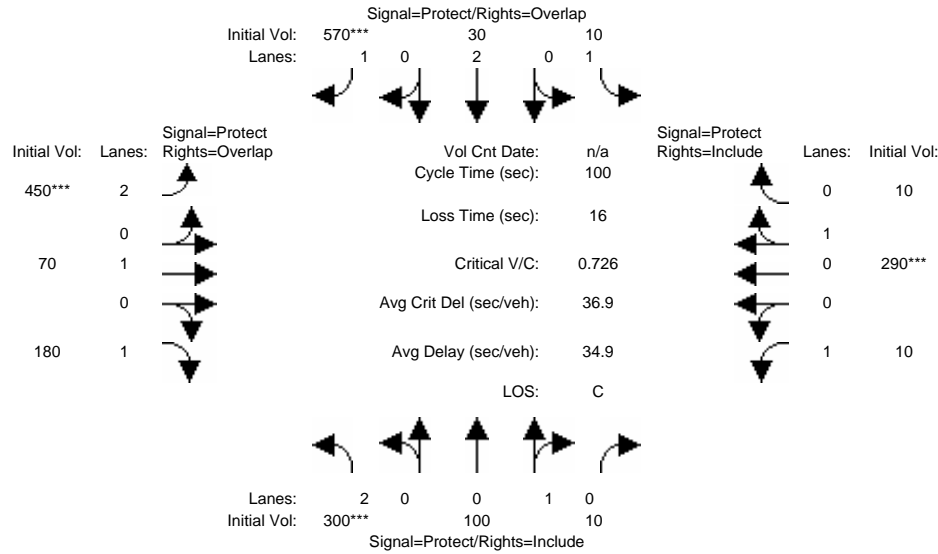


Table with columns for Approach, Movement, and various performance metrics like Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.25	0.25	0.18	0.32	0.50	0.18	0.24	0.36	0.17	0.22	0.22
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.1	2.4	2.4	0.2	0.3	12.5	6.1	1.5	3.6	0.2	7.7	7.7
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	2.1	0.3	0.3	0.0	0.0	2.4	2.2	0.2	0.5	0.0	2.3	2.3
HCM2KQueue:	6.2	2.7	2.7	0.3	0.3	14.9	8.3	1.7	4.1	0.3	10.0	10.0
70thFactor:	1.19	1.19	1.19	1.20	1.20	1.17	1.18	1.20	1.19	1.20	1.18	1.18
70thHCM2kQ:	7.4	3.3	3.3	0.3	0.4	17.4	9.8	2.1	4.9	0.3	11.8	11.8
85thFactor:	1.54	1.57	1.57	1.60	1.60	1.48	1.53	1.58	1.56	1.60	1.51	1.51
85thHCM2kQ:	9.6	4.3	4.3	0.4	0.5	22.1	12.6	2.7	6.4	0.4	15.2	15.2
90thFactor:	1.69	1.75	1.75	1.79	1.79	1.59	1.66	1.77	1.73	1.79	1.64	1.64
90thHCM2kQ:	10.5	4.8	4.8	0.5	0.6	23.7	13.8	3.1	7.1	0.5	16.4	16.4
95thFactor:	1.93	2.02	2.02	2.09	2.09	1.76	1.88	2.04	1.98	2.09	1.84	1.84
95thHCM2kQ:	11.9	5.5	5.5	0.5	0.7	26.2	15.6	3.5	8.1	0.6	18.4	18.4
98thFactor:	2.32	2.51	2.51	2.68	2.67	2.02	2.23	2.58	2.43	2.68	2.16	2.16
98thHCM2kQ:	14.4	6.9	6.9	0.7	0.9	30.0	18.5	4.5	10.0	0.7	21.6	21.6

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	72.3	19.8	2.0	2.1	5.2	112.2	106.1	13.9	32.7	2.1	67.2	2.3

Name: year 1995 composite fleet
 Fuel Consumption: 74.864 pounds
 12.128 gallons
 Carbon Dioxide: 233.577 pounds
 Carbon Monoxide: 18.878 pounds
 Hydrocarbons: 3.617 pounds
 Nitrogen Oxides: 0.626 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 71.093 pounds
 11.517 gallons
 Carbon Dioxide: 221.810 pounds
 Carbon Monoxide: 18.581 pounds
 Hydrocarbons: 3.557 pounds
 Nitrogen Oxides: 0.565 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #28: Delta Shores Circle South/Street D South

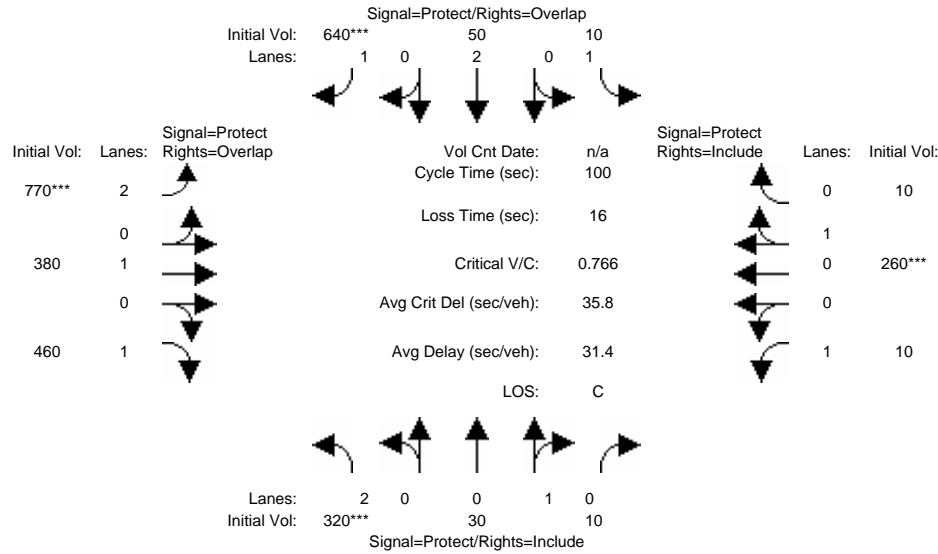


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.12	0.21	0.21	0.15	0.23	0.53	0.29	0.39	0.51	0.10	0.19	0.19
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	4.4	0.9	0.9	0.2	0.6	14.1	10.1	8.1	8.8	0.3	7.1	7.1
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q2:	2.4	0.1	0.1	0.0	0.1	2.9	2.8	1.1	1.3	0.1	2.6	2.6
HCM2KQueue:	6.8	1.0	1.0	0.3	0.6	17.0	12.8	9.2	10.1	0.3	9.7	9.7
70thFactor:	1.18	1.20	1.20	1.20	1.20	1.17	1.17	1.18	1.18	1.20	1.18	1.18
70thHCM2kQ:	8.1	1.2	1.2	0.3	0.8	19.8	15.0	10.8	11.9	0.4	11.5	11.5
85thFactor:	1.54	1.59	1.59	1.60	1.59	1.47	1.50	1.52	1.51	1.60	1.52	1.52
85thHCM2kQ:	10.5	1.6	1.6	0.4	1.0	25.0	19.2	14.0	15.3	0.5	14.7	14.7
90thFactor:	1.68	1.78	1.78	1.79	1.79	1.57	1.61	1.65	1.64	1.79	1.65	1.65
90thHCM2kQ:	11.5	1.8	1.8	0.5	1.1	26.7	20.6	15.2	16.6	0.6	16.0	16.0
95thFactor:	1.91	2.07	2.07	2.09	2.08	1.73	1.79	1.86	1.84	2.09	1.85	1.85
95thHCM2kQ:	13.0	2.1	2.1	0.6	1.3	29.5	23.0	17.1	18.6	0.7	18.0	18.0
98thFactor:	2.29	2.62	2.62	2.68	2.65	1.97	2.07	2.19	2.16	2.68	2.17	2.17
98thHCM2kQ:	15.6	2.7	2.7	0.7	1.7	33.5	26.6	20.2	21.8	0.8	21.1	21.1

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #28 Delta Shores Circle South/Street D South

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	77.5	6.1	2.0	2.1	9.7	126.8	175.5	73.0	79.4	2.3	61.6	2.4

Name: year 1995 composite fleet
 Fuel Consumption: 101.324 pounds
 16.414 gallons
 Carbon Dioxide: 316.131 pounds
 Carbon Monoxide: 25.319 pounds
 Hydrocarbons: 4.777 pounds
 Nitrogen Oxides: 0.867 pounds

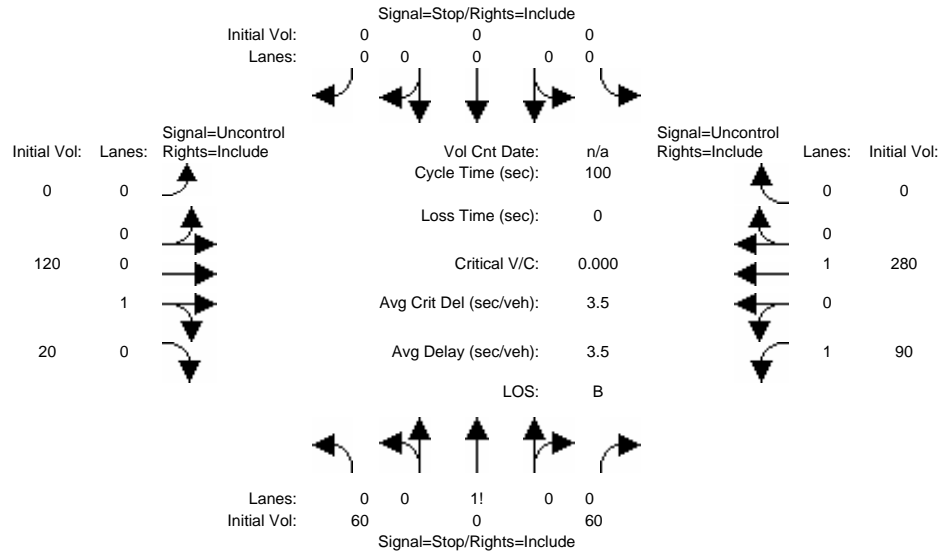
Name: year 2000 composite fleet
 Fuel Consumption: 95.877 pounds
 15.532 gallons
 Carbon Dioxide: 299.135 pounds
 Carbon Monoxide: 24.891 pounds
 Hydrocarbons: 4.690 pounds
 Nitrogen Oxides: 0.780 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP AM

Intersection #29: Street C/Stone-Boswell Access West



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	60	0	60	0	0	0	0	120	20	90	280	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	60	0	60	0	0	0	0	120	20	90	280	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	60	0	60	0	0	0	0	120	20	90	280	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	60	0	60	0	0	0	0	120	20	90	280	0
Critical Gap Module:												
Critical Gap:	6.4	xxxxx	6.2	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	4.1	xxxxx	xxxxx
FollowUpTim:	3.5	xxxxx	3.3	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	2.2	xxxxx	xxxxx
Capacity Module:												
Cnflict Vol:	590	xxxxx	130	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	140	xxxxx	xxxxx
Potent Cap.:	470	xxxxx	920	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1443	xxxxx	xxxxx
Move Cap.:	448	xxxxx	920	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1443	xxxxx	xxxxx
Volume/Cap:	0.13	xxxxx	0.07	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.06	xxxxx	xxxxx
Level of Service Module:												
2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.2	xxxxx	xxxxx
Control Del:	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	7.7	xxxxx	xxxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxxx	602	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx
SharedQueue:	xxxxxx	0.7	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
Shrd ConDel:	xxxxxx	12.5	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
Shared LOS:	*	B	*	*	*	*	*	*	*	*	A	*
ApproachDel:	12.5			xxxxxxx			xxxxxxx			xxxxxxx		
ApproachLOS:	B			*			*			*		
HevVeh:	2%			2%			2%			2%		
Grade:	0%			0%			0%			0%		
Peds/Hour:	0			0			0			0		
Pedestrian Walk Speed:	4.00 feet/sec											
LaneWidth:	12 feet			12 feet			12 feet			12 feet		
Time Period:	0.25 hour											

Peak Hour Delay Signal Warrant Report

Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	60	0	60	0	0	0	0	120	20	90	280	0
ApproachDel:	12.5			xxxxxxx			xxxxxxx			xxxxxxx		

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.4]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=120]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=630]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	60	0	60	0	0	0	0	120	20	90	280	0
Major Street Volume:	510											
Minor Approach Volume:	120											
Minor Approach Volume Threshold:	517											

SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Baseline WP PM

Intersection #29: Street C/Stone-Boswell Access West

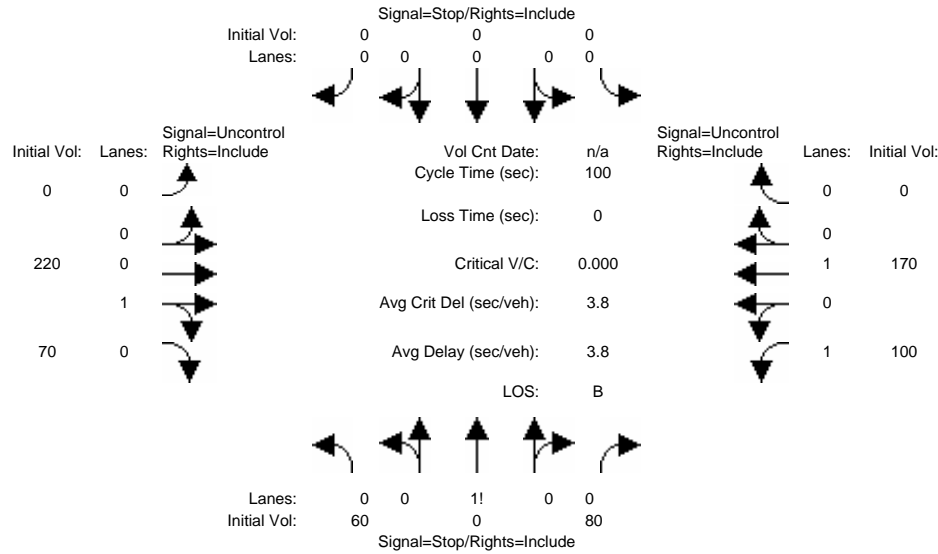


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). It contains data for Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #29 Street C/Stone-Boswell Access West
Base Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 1 0 1 0 1 0 0
Initial Vol: 60 0 80 0 0 0 0 0 220 70 100 170 0
ApproachDel: 13.3 xxxxxx xxxxxxxx xxxxxxxx xxxxxxxx
Signal Warrant Rule #1: [vehicle-hours=0.5] FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=140] SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=700] SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.
Peak Hour Volume Signal Warrant Report [Urban]
Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	60	0	80	0	0	0	0	220	70	100	170	0
Major Street Volume:	560											
Minor Approach Volume:	140											
Minor Approach Volume Threshold:	485											

SIGNAL WARRANT DISCLAIMER

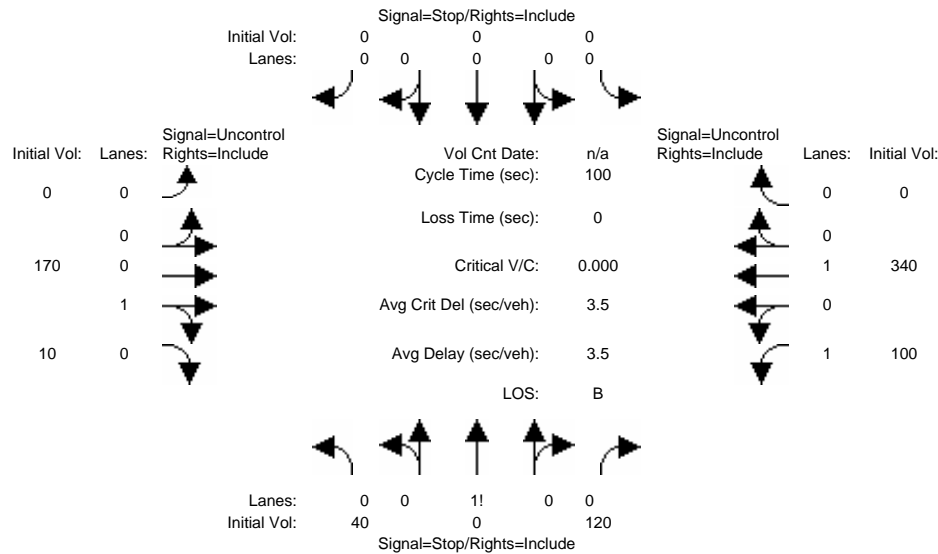
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline WP AM

Intersection #30: Street C/Stone-Boswell Access East



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	40	0	120	0	0	0	0	170	10	100	340	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	40	0	120	0	0	0	0	170	10	100	340	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	40	0	120	0	0	0	0	170	10	100	340	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	40	0	120	0	0	0	0	170	10	100	340	0
Critical Gap Module:												
Critical Gap:	6.4	xxxxx	6.2	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	4.1	xxxxx	xxxxx
FollowUpTim:	3.5	xxxxx	3.3	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	2.2	xxxxx	xxxxx
Capacity Module:												
Cnflict Vol:	715	xxxxx	175	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	180	xxxxx	xxxxx
Potent Cap.:	397	xxxxx	868	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1396	xxxxx	xxxxx
Move Cap.:	376	xxxxx	868	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1396	xxxxx	xxxxx
Volume/Cap:	0.11	xxxxx	0.14	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.07	xxxxx	xxxxx
Level of Service Module:												
2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.2	xxxxx	xxxxx
Control Del:	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	7.8	xxxxxx	xxxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxxx	654	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx	xxxxx	xxxxx	xxxxxx
SharedQueue:	xxxxxx	1.0	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
Shrd ConDel:	xxxxxx	12.3	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
Shared LOS:	*	B	*	*	*	*	*	*	*	*	*	*
ApproachDel:	12.3		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx
ApproachLOS:	B		*	*		*	*		*	*		*
HevVeh:	2%		2%	2%		2%	2%		2%	2%		2%
Grade:	0%		0%	0%		0%	0%		0%	0%		0%
Peds/Hour:	0		0	0		0	0		0	0		0
Pedestrian Walk Speed:	4.00 feet/sec											
LaneWidth:	12 feet		12 feet	12 feet		12 feet	12 feet		12 feet	12 feet		12 feet
Time Period:	0.25 hour											

Peak Hour Delay Signal Warrant Report

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	
Lanes:	0	0	1	0	0	0	0	0	1	0	1	0	0
Initial Vol:	40	0	120	0	0	0	0	170	10	100	340	0	
ApproachDel:	12.3		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx	

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=160]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=780]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.
Peak Hour Volume Signal Warrant Report [Urban]

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	40	0	120	0	0	0	0	170	10	100	340	0

Major Street Volume: 620
 Minor Approach Volume: 160
 Minor Approach Volume Threshold: 450

SIGNAL WARRANT DISCLAIMER

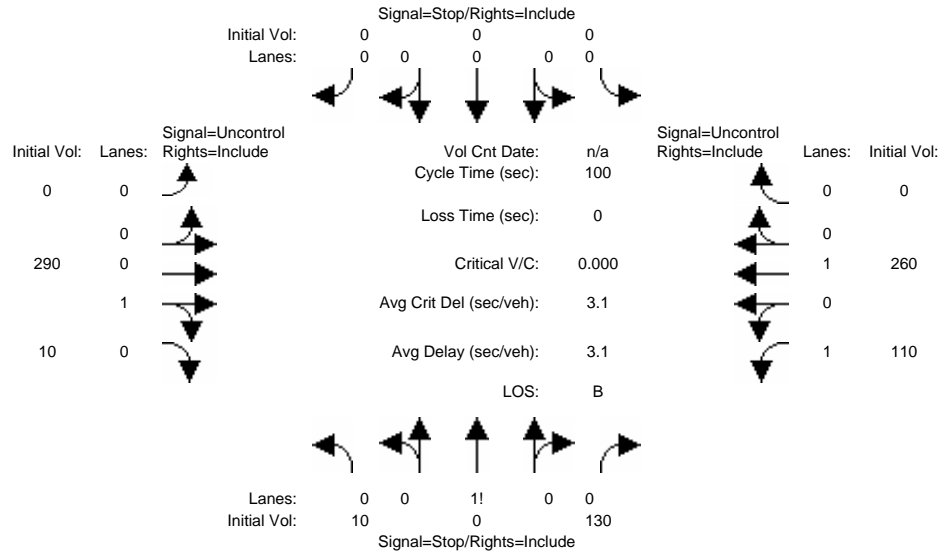
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Baseline WP PM

Intersection #30: Street C/Stone-Boswell Access East



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:												
Base Vol:	10	0	130	0	0	0	0	290	10	110	260	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	0	130	0	0	0	0	290	10	110	260	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	0	130	0	0	0	0	290	10	110	260	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	10	0	130	0	0	0	0	290	10	110	260	0
Critical Gap Module:												
Critical Gap:	6.4	xxxxx	6.2	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	4.1	xxxxx	xxxxx
FollowUpTim:	3.5	xxxxx	3.3	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	2.2	xxxxx	xxxxx
Capacity Module:												
Cnflict Vol:	775	xxxxx	295	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	300	xxxxx	xxxxx
Potent Cap.:	366	xxxxx	744	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1261	xxxxx	xxxxx
Move Cap.:	342	xxxxx	744	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1261	xxxxx	xxxxx
Volume/Cap:	0.03	xxxxx	0.17	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.09	xxxxx	xxxxx
Level of Service Module:												
2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.3	xxxxx	xxxxx
Control Del:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	8.1	xxxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxxx	687	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
SharedQueue:	xxxxx	0.8	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Shrd ConDel:	xxxxx	11.6	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Shared LOS:	*	B	*	*	*	*	*	*	*	*	*	*
ApproachDel:	11.6		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx
ApproachLOS:	B		*	*		*	*		*	*		*
HevVeh:	2%		2%	2%		2%	2%		2%	2%		2%
Grade:	0%		0%	0%		0%	0%		0%	0%		0%
Peds/Hour:	0		0	0		0	0		0	0		0
Pedestrian Walk Speed:	4.00 feet/sec											
LaneWidth:	12 feet			12 feet			12 feet			12 feet		
Time Period:	0.25 hour											

Peak Hour Delay Signal Warrant Report

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	10	0	130	0	0	0	0	290	10	110	260	0
ApproachDel:	11.6		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx	xxxxxxx		xxxxxxx

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=140]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=810]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #30 Street C/Stone-Boswell Access East

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Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:    Stop Sign      Stop Sign      Uncontrolled    Uncontrolled
Lanes:      0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      1 0 1 0 0
Initial Vol: 10 0 130      0 0 0 0      0 290 10      110 260 0
-----|-----|-----|-----|-----|
Major Street Volume:      670
Minor Approach Volume:    140
Minor Approach Volume Threshold: 423
-----|-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

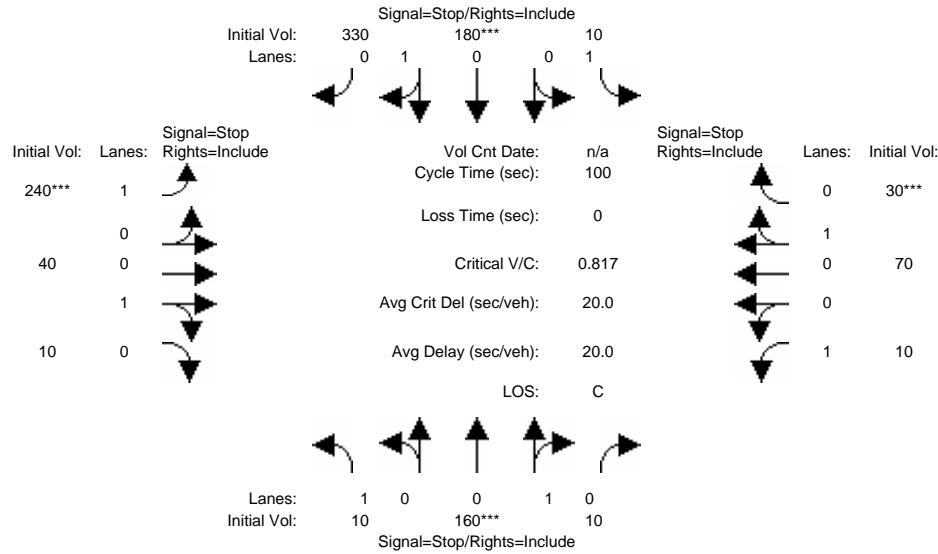
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline WP AM

Intersection #31: Street C/Detroit Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:												
Base Vol:	10	160	10	10	180	330	240	40	10	10	70	30
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	160	10	10	180	330	240	40	10	10	70	30
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	160	10	10	180	330	240	40	10	10	70	30
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	10	160	10	10	180	330	240	40	10	10	70	30
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	10	160	10	10	180	330	240	40	10	10	70	30
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.94	0.06	1.00	0.35	0.65	1.00	0.80	0.20	1.00	0.70	0.30
Final Sat.:	478	489	31	528	220	404	477	413	103	437	334	143
Capacity Analysis Module:												
Vol/Sat:	0.02	0.33	0.33	0.02	0.82	0.82	0.50	0.10	0.10	0.02	0.21	0.21
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Delay/Veh:	9.9	12.2	12.2	9.5	27.7	27.7	16.3	9.9	9.9	10.4	11.2	11.2
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	9.9	12.2	12.2	9.5	27.7	27.7	16.3	9.9	9.9	10.4	11.2	11.2
LOS by Move:	A	B	B	A	D	D	C	A	A	B	B	B
ApproachDel:	12.0			27.3			15.2			11.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.0			27.3			15.2			11.1		
LOS by Appr:	B			D			C			B		
AllWayAvgQ:	0.0	0.4	0.4	0.0	3.3	3.3	0.9	0.1	0.1	0.0	0.2	0.2
Time Period: 0.25 hour												
HevVeh:	2%			2%			2%			2%		
Alpha Value: 0.01												
GroupType:	5			5			5			5		
P[C1]:	0.07			0.25			0.10			0.06		
P[C2]:	0.30			0.12			0.03			0.07		
P[C3]:	0.10			0.35			0.48			0.29		
P[C4]:	0.44			0.24			0.33			0.44		
P[C5]:	0.09			0.04			0.05			0.14		
Padj[C1]:	0.022			0.017			0.022			0.025		
Padj[C2]:	0.009			0.008			0.013			0.015		
Padj[C3]:	0.003			-0.007			-0.010			-0.001		
Padj[C4]:	-0.025			-0.014			-0.019			-0.025		
Padj[C5]:	-0.009			-0.004			-0.005			-0.014		
Lanes:	L1	L2		L1	L2		L1	L2		L1	L2	
LaneType:	LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU	
HeadwayAdj:	0.534	-0.007		0.534	-0.419		0.534	-0.106		0.534	-0.176	
Volume:	10	170		10	510		240	50		10	100	
Capacity:	478	519		528	625		477	516		437	478	
DegOfUtil:	0.02	0.31		0.02	0.81		0.48	0.09		0.02	0.19	
DepHeadway:	7.09	6.55		6.64	5.69		7.19	6.55		7.58	6.87	
ServiceTime:	4.8	4.3		4.3	3.4		4.9	4.2		5.3	4.6	
Delay:	9.9	12.2		9.5	27.7		16.3	9.9		10.4	11.2	
Queue:	0.0	0.4		0.0	3.3		0.9	0.1		0.0	0.2	
Approach:	North Bound			South Bound			East Bound			West Bound		
ApproachDel:	12.0			27.3			15.2			11.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.0			27.3			15.2			11.1		
LOS by Appr:	B			D			C			B		
OverallDel:							20.0					
OverallLOS:							C					
Peak Hour Volume Signal Warrant Report [Urban]												

Intersection #31 Street C/Detroit Boulevard												

Base Volume Alternative: Peak Hour Warrant NOT Met												

Approach:	North Bound			South Bound			East Bound			West Bound		

```

Movement:  L - T - R  L - T - R  L - T - R  L - T - R
-----|-----|-----|-----|
Control:    Stop Sign  Stop Sign  Stop Sign  Stop Sign
Lanes:      1 0 0 1 0  1 0 0 1 0  1 0 0 1 0  1 0 0 1 0
Initial Vol: 10 160 10  10 180 330  240 40 10  10 70 30 30
-----|-----|-----|-----|
Major Street Volume:      700
Minor Approach Volume:    290
Minor Approach Volume Threshold: 527
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

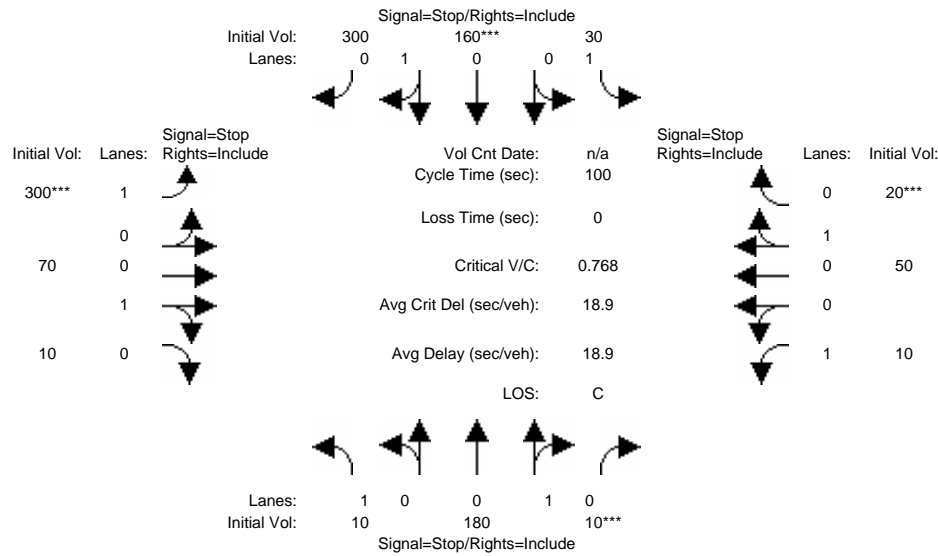
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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Baseline WP PM

Intersection #31: Street C/Detroit Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:	-----											
Base Vol:	10	180	10	30	160	300	300	70	10	10	50	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	180	10	30	160	300	300	70	10	10	50	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	180	10	30	160	300	300	70	10	10	50	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	10	180	10	30	160	300	300	70	10	10	50	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	10	180	10	30	160	300	300	70	10	10	50	20
Saturation Flow Module:	-----											
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.95	0.05	1.00	0.35	0.65	1.00	0.87	0.13	1.00	0.71	0.29
Final Sat.:	470	482	27	509	208	391	488	459	66	422	329	132
Capacity Analysis Module:	-----											
Vol/Sat:	0.02	0.37	0.37	0.06	0.77	0.77	0.61	0.15	0.15	0.02	0.15	0.15
Crit Moves:	****			****			****			****		
Delay/Veh:	10.1	13.1	13.1	10.0	24.5	24.5	19.7	10.4	10.4	10.6	10.9	10.9
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	10.1	13.1	13.1	10.0	24.5	24.5	19.7	10.4	10.4	10.6	10.9	10.9
LOS by Move:	B	B	B	A	C	C	C	B	B	B	B	B
ApproachDel:	12.9			23.6			17.8			10.8		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.9			23.6			17.8			10.8		
LOS by Appr:	B			C			C			B		
AllWayAvgQ:	0.0	0.5	0.5	0.1	2.6	2.6	1.3	0.2	0.2	0.0	0.1	0.1
Time Period:	0.25 hour											
HevVeh:	2%			2%			2%			2%		
Alpha Value:	0.01											
GroupType:	5			5			5			5		
P[C1]:	0.07			0.19			0.12			0.05		
P[C2]:	0.23			0.11			0.02			0.10		
P[C3]:	0.14			0.38			0.48			0.20		
P[C4]:	0.49			0.29			0.33			0.47		
P[C5]:	0.08			0.04			0.04			0.18		
Padj[C1]:	0.023			0.019			0.021			0.026		
Padj[C2]:	0.011			0.010			0.012			0.016		
Padj[C3]:	0.002			-0.008			-0.010			0.002		
Padj[C4]:	-0.029			-0.017			-0.019			-0.026		
Padj[C5]:	-0.008			-0.004			-0.004			-0.018		
Lanes:	L1	L2		L1	L2		L1	L2		L1	L2	
LaneType:	LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU	
HeadwayAdj:	0.534	-0.003		0.534	-0.423		0.534	-0.053		0.534	-0.166	
Volume:	10	190		30	460		300	80		10	70	
Capacity:	470	509		509	599		488	525		422	461	
DegOfUtil:	0.02	0.35		0.06	0.75		0.59	0.15		0.02	0.14	
DepHeadway:	7.26	6.72		6.86	5.91		7.13	6.54		7.75	7.05	
ServiceTime:	5.0	4.4		4.6	3.6		4.8	4.2		5.5	4.8	
Delay:	10.1	13.1		10.0	24.5		19.7	10.4		10.6	10.9	
Queue:	0.0	0.5		0.1	2.6		1.3	0.2		0.0	0.1	
Approach:	North Bound			South Bound			East Bound			West Bound		
ApproachDel:	12.9			23.6			17.8			10.8		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.9			23.6			17.8			10.8		
LOS by Appr:	B			C			C			B		
OverallDel:	18.9											
OverallLOS:	C											
Peak Hour Volume Signal Warrant Report [Urban]												

Intersection #31 Street C/Detroit Boulevard												

Base Volume Alternative: Peak Hour Warrant NOT Met												

Approach:	North Bound			South Bound			East Bound			West Bound		

```

Movement:  L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Stop Sign   Stop Sign   Stop Sign   Stop Sign
Lanes:      1 0 0 1 0   1 0 0 1 0   1 0 0 1 0   1 0 0 1 0
Initial Vol: 10 180 10   30 160 300   300 70 10   10 50 20
-----|-----|-----|-----|
Major Street Volume:      690
Minor Approach Volume:    380
Minor Approach Volume Threshold: 534
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP AM

Intersection #32:

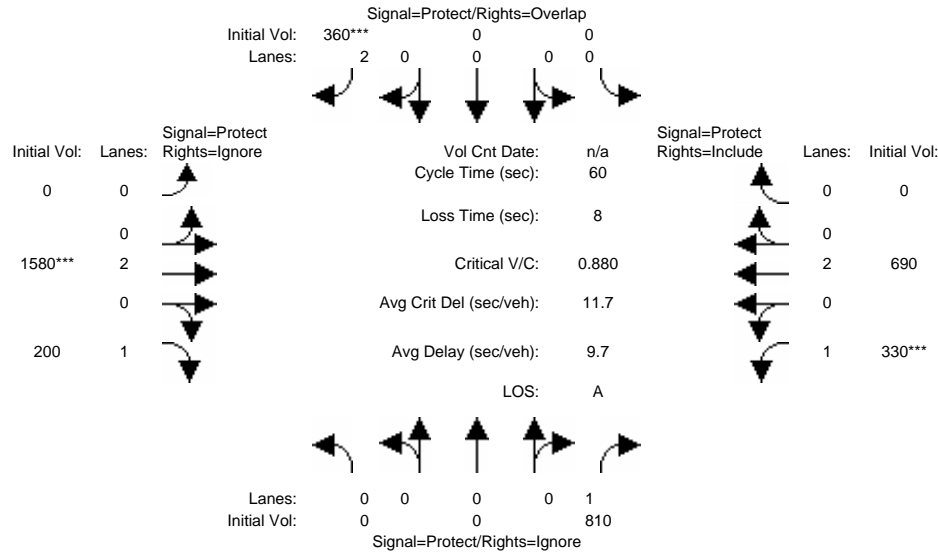


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	3.2	0.0	6.0	0.0	5.1	2.3	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	2.4	0.0	1.4	0.0	2.8	0.4	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	5.7	0.0	7.4	0.0	8.0	2.6	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.19	1.20	1.18	1.20	1.18	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	6.7	0.0	8.8	0.0	9.4	3.2	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.55	1.60	1.53	1.60	1.53	1.57	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	8.8	0.0	11.4	0.0	12.2	4.2	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.70	1.80	1.68	1.80	1.67	1.75	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	9.7	0.0	12.4	0.0	13.3	4.6	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.94	2.10	1.90	2.10	1.89	2.02	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	11.0	0.0	14.1	0.0	15.0	5.3	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.35	2.70	2.27	2.70	2.24	2.52	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	13.3	0.0	16.8	0.0	17.8	6.6	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	85.8	0.0	171	0.0	77.1	64.3	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 54.797 pounds
 8.877 gallons
 Carbon Dioxide: 170.966 pounds
 Carbon Monoxide: 12.204 pounds
 Hydrocarbons: 1.859 pounds
 Nitrogen Oxides: 0.558 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 49.747 pounds
 8.059 gallons
 Carbon Dioxide: 155.212 pounds
 Carbon Monoxide: 11.816 pounds
 Hydrocarbons: 1.779 pounds
 Nitrogen Oxides: 0.487 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #32:

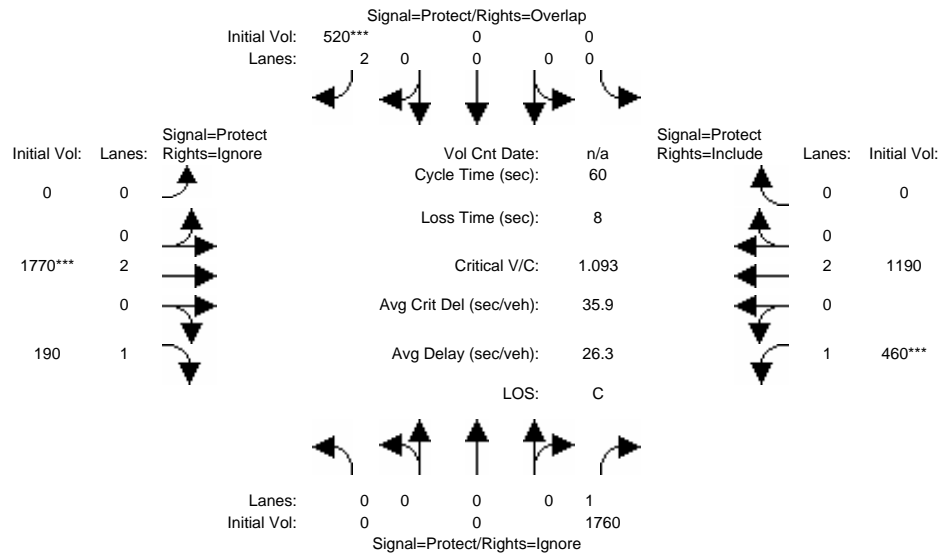


Table containing traffic engineering data: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.17	0.00	0.76	0.00	0.24	0.70	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.0	0.0	4.9	0.0	7.5	0.0	7.7	4.7	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.0	0.0	7.9	0.0	1.9	0.0	10.1	0.9	0.0
HCM2KQueue:	0.0	0.0	0.0	0.0	0.0	12.9	0.0	9.3	0.0	17.8	5.6	0.0
70thFactor:	1.20	1.20	1.20	1.20	1.20	1.17	1.20	1.18	1.20	1.16	1.19	1.20
70thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	15.1	0.0	11.0	0.0	20.7	6.7	0.0
85thFactor:	1.60	1.60	1.60	1.60	1.60	1.50	1.60	1.52	1.60	1.47	1.55	1.60
85thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	19.2	0.0	14.2	0.0	26.1	8.7	0.0
90thFactor:	1.80	1.80	1.80	1.80	1.80	1.61	1.80	1.65	1.80	1.56	1.70	1.80
90thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	20.7	0.0	15.4	0.0	27.8	9.6	0.0
95thFactor:	2.10	2.10	2.10	2.10	2.10	1.79	2.10	1.86	2.10	1.72	1.94	2.10
95thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	23.1	0.0	17.3	0.0	30.6	10.9	0.0
98thFactor:	2.70	2.70	2.70	2.70	2.70	2.07	2.70	2.19	2.70	1.95	2.35	2.70
98thHCM2kQ:	0.0	0.0	0.0	0.0	0.0	26.7	0.0	20.4	0.0	34.8	13.2	0.0

Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

Intersection #32

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	0.0	0.0	132.7	0.0	213	0.0	118.1	134	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 118.711 pounds
 19.231 gallons
 Carbon Dioxide: 370.380 pounds
 Carbon Monoxide: 29.230 pounds
 Hydrocarbons: 5.438 pounds
 Nitrogen Oxides: 0.989 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 111.864 pounds
 18.122 gallons
 Carbon Dioxide: 349.016 pounds
 Carbon Monoxide: 28.700 pounds
 Hydrocarbons: 5.329 pounds
 Nitrogen Oxides: 0.890 pounds

DISCLAIMER

The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.47	0.00	0.47	0.00	0.45	0.00	0.00	0.45	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	5.0	0.0	12.5	0.0	14.4	0.0	0.0	14.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	0.5	0.0	2.6	0.0	2.7	0.0	0.0	2.7	0.0
HCM2KQueue:	0.0	0.0	0.0	5.5	0.0	15.1	0.0	17.1	0.0	0.0	17.1	0.0
70thFactor:	1.20	1.20	1.20	1.19	1.20	1.17	1.20	1.17	1.20	1.20	1.17	1.20
70thHCM2kQ:	0.0	0.0	0.0	6.6	0.0	17.7	0.0	19.9	0.0	0.0	19.9	0.0
85thFactor:	1.60	1.60	1.60	1.55	1.60	1.48	1.60	1.47	1.60	1.60	1.47	1.60
85thHCM2kQ:	0.0	0.0	0.0	8.6	0.0	22.4	0.0	25.1	0.0	0.0	25.1	0.0
90thFactor:	1.80	1.80	1.80	1.70	1.80	1.59	1.80	1.57	1.80	1.80	1.57	1.80
90thHCM2kQ:	0.0	0.0	0.0	9.4	0.0	24.0	0.0	26.9	0.0	0.0	26.9	0.0
95thFactor:	2.10	2.10	2.10	1.94	2.10	1.76	2.10	1.73	2.10	2.10	1.73	2.10
95thHCM2kQ:	0.0	0.0	0.0	10.7	0.0	26.6	0.0	29.6	0.0	0.0	29.6	0.0
98thFactor:	2.70	2.70	2.70	2.35	2.70	2.01	2.70	1.97	2.70	2.70	1.97	2.70
98thHCM2kQ:	0.0	0.0	0.0	13.0	0.0	30.5	0.0	33.7	0.0	0.0	33.7	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	87.4	0.0	112.5	0.0	355	0.0	0.0	355	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 130.309 pounds
 21.110 gallons
 Carbon Dioxide: 406.564 pounds
 Carbon Monoxide: 31.699 pounds
 Hydrocarbons: 5.695 pounds
 Nitrogen Oxides: 1.195 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 122.004 pounds
 19.765 gallons
 Carbon Dioxide: 380.653 pounds
 Carbon Monoxide: 31.047 pounds
 Hydrocarbons: 5.563 pounds
 Nitrogen Oxides: 1.064 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Baseline WP PM

Intersection #33:

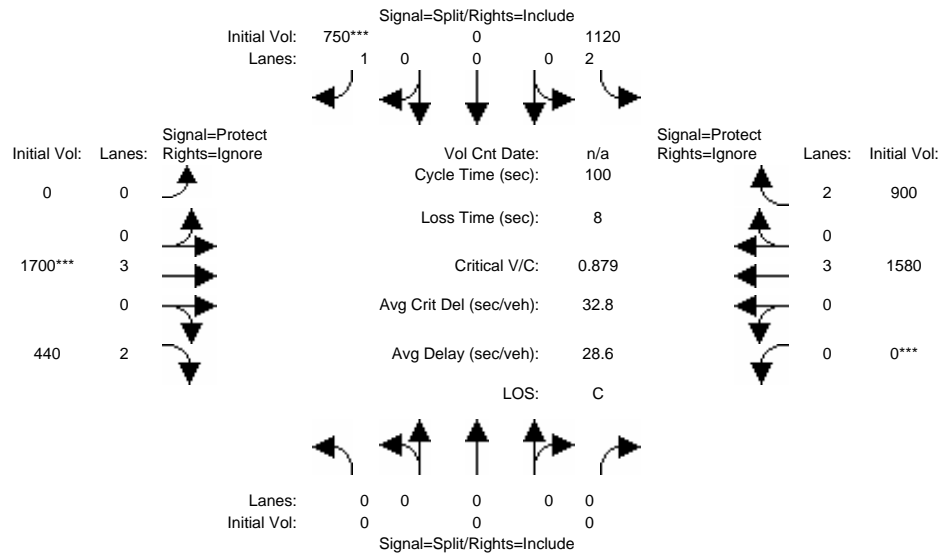


Table containing traffic engineering data: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.54	0.00	0.54	0.00	0.38	0.00	0.00	0.38	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	11.0	0.0	18.2	0.0	16.1	0.0	0.0	14.4	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.5	0.0	5.2	0.0	5.0	0.0	0.0	3.6	0.0
HCM2KQueue:	0.0	0.0	0.0	12.5	0.0	23.4	0.0	21.1	0.0	0.0	18.1	0.0
70thFactor:	1.20	1.20	1.20	1.17	1.20	1.16	1.20	1.16	1.20	1.20	1.16	1.20
70thHCM2kQ:	0.0	0.0	0.0	14.6	0.0	27.1	0.0	24.4	0.0	0.0	21.0	0.0
85thFactor:	1.60	1.60	1.60	1.50	1.60	1.44	1.60	1.45	1.60	1.60	1.46	1.60
85thHCM2kQ:	0.0	0.0	0.0	18.7	0.0	33.7	0.0	30.5	0.0	0.0	26.5	0.0
90thFactor:	1.80	1.80	1.80	1.61	1.80	1.52	1.80	1.54	1.80	1.80	1.56	1.80
90thHCM2kQ:	0.0	0.0	0.0	20.1	0.0	35.7	0.0	32.4	0.0	0.0	28.2	0.0
95thFactor:	2.10	2.10	2.10	1.80	2.10	1.66	2.10	1.69	2.10	2.10	1.72	2.10
95thHCM2kQ:	0.0	0.0	0.0	22.4	0.0	38.9	0.0	35.5	0.0	0.0	31.1	0.0
98thFactor:	2.70	2.70	2.70	2.08	2.70	1.87	2.70	1.90	2.70	2.70	1.95	2.70
98thHCM2kQ:	0.0	0.0	0.0	25.9	0.0	43.7	0.0	40.0	0.0	0.0	35.2	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #33

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	191.5	0.0	164.2	0.0	395	0.0	0.0	355	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 167.260 pounds
 27.096 gallons
 Carbon Dioxide: 521.851 pounds
 Carbon Monoxide: 41.445 pounds
 Hydrocarbons: 7.689 pounds
 Nitrogen Oxides: 1.477 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 157.701 pounds
 25.547 gallons
 Carbon Dioxide: 492.026 pounds
 Carbon Monoxide: 40.692 pounds
 Hydrocarbons: 7.538 pounds
 Nitrogen Oxides: 1.324 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.28	0.00	0.28	0.00	0.00	0.00	0.00	0.64	0.00	0.00	0.64	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	5.9	0.0	4.8	0.0	0.0	0.0	0.0	8.8	0.0	0.0	8.8	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	1.0	0.0	0.8	0.0	0.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
HCM2KQueue:	6.8	0.0	5.7	0.0	0.0	0.0	0.0	9.7	0.0	0.0	9.8	0.0
70thFactor:	1.18	1.20	1.19	1.20	1.20	1.20	1.20	1.18	1.20	1.20	1.18	1.20
70thHCM2kQ:	8.1	0.0	6.7	0.0	0.0	0.0	0.0	11.5	0.0	0.0	11.6	0.0
85thFactor:	1.54	1.60	1.55	1.60	1.60	1.60	1.60	1.52	1.60	1.60	1.52	1.60
85thHCM2kQ:	10.5	0.0	8.8	0.0	0.0	0.0	0.0	14.7	0.0	0.0	14.9	0.0
90thFactor:	1.68	1.80	1.70	1.80	1.80	1.80	1.80	1.65	1.80	1.80	1.64	1.80
90thHCM2kQ:	11.5	0.0	9.6	0.0	0.0	0.0	0.0	16.0	0.0	0.0	16.1	0.0
95thFactor:	1.91	2.10	1.94	2.10	2.10	2.10	2.10	1.85	2.10	2.10	1.85	2.10
95thHCM2kQ:	13.0	0.0	11.0	0.0	0.0	0.0	0.0	18.0	0.0	0.0	18.1	0.0
98thFactor:	2.29	2.70	2.35	2.70	2.70	2.70	2.70	2.17	2.70	2.70	2.17	2.70
98thHCM2kQ:	15.6	0.0	13.3	0.0	0.0	0.0	0.0	21.1	0.0	0.0	21.3	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	102.2	0.0	76.3	0.0	0.0	0.0	0.0	215	0.0	0.0	217	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 89.153 pounds
 14.443 gallons
 Carbon Dioxide: 278.157 pounds
 Carbon Monoxide: 20.607 pounds
 Hydrocarbons: 3.392 pounds
 Nitrogen Oxides: 0.861 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 82.004 pounds
 13.285 gallons
 Carbon Dioxide: 255.854 pounds
 Carbon Monoxide: 20.055 pounds
 Hydrocarbons: 3.279 pounds
 Nitrogen Oxides: 0.758 pounds

DISCLAIMER

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Delta Shores Phase 1

Level Of Service Computation Report
2000 HCM Operations (alternative)
Baseline WP PM

Intersection #34:

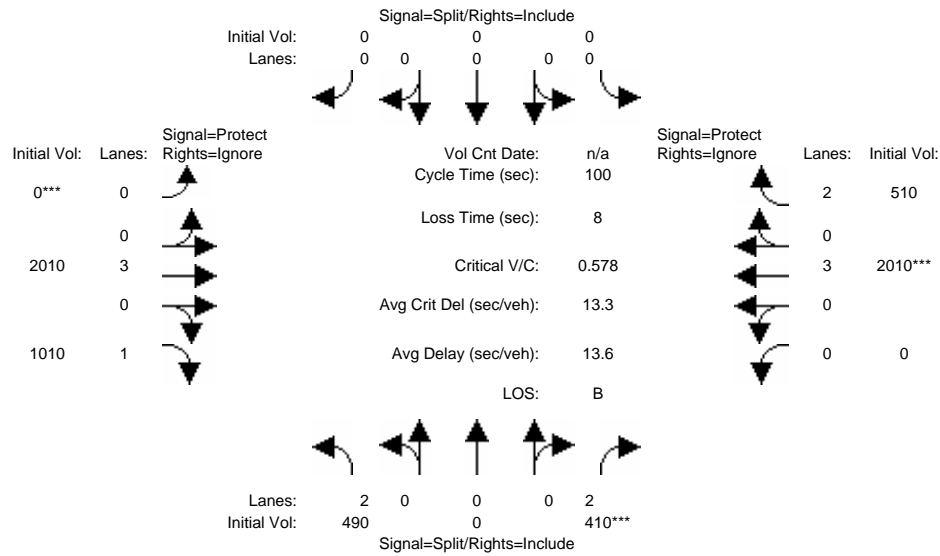


Table containing detailed traffic engineering data including: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.25	0.00	0.25	0.00	0.00	0.00	0.00	0.67	0.00	0.00	0.67	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	6.1	0.0	5.7	0.0	0.0	0.0	0.0	11.0	0.0	0.0	11.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	1.2	0.0	1.3	0.0	0.0	0.0	0.0	1.3	0.0	0.0	1.3	0.0
HCM2KQueue:	7.3	0.0	7.0	0.0	0.0	0.0	0.0	12.3	0.0	0.0	12.3	0.0
70thFactor:	1.18	1.20	1.18	1.20	1.20	1.20	1.20	1.17	1.20	1.20	1.17	1.20
70thHCM2kQ:	8.7	0.0	8.2	0.0	0.0	0.0	0.0	14.5	0.0	0.0	14.5	0.0
85thFactor:	1.53	1.60	1.54	1.60	1.60	1.60	1.60	1.50	1.60	1.60	1.50	1.60
85thHCM2kQ:	11.3	0.0	10.7	0.0	0.0	0.0	0.0	18.5	0.0	0.0	18.5	0.0
90thFactor:	1.68	1.80	1.68	1.80	1.80	1.80	1.80	1.62	1.80	1.80	1.62	1.80
90thHCM2kQ:	12.3	0.0	11.7	0.0	0.0	0.0	0.0	19.9	0.0	0.0	19.9	0.0
95thFactor:	1.90	2.10	1.91	2.10	2.10	2.10	2.10	1.80	2.10	2.10	1.80	2.10
95thHCM2kQ:	13.9	0.0	13.3	0.0	0.0	0.0	0.0	22.3	0.0	0.0	22.3	0.0
98thFactor:	2.27	2.70	2.29	2.70	2.70	2.70	2.70	2.09	2.70	2.70	2.09	2.70
98thHCM2kQ:	16.6	0.0	15.9	0.0	0.0	0.0	0.0	25.8	0.0	0.0	25.8	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #34

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	106.9	0.0	89.9	0.0	0.0	0.0	0.0	270	0.0	0.0	270	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 105.175 pounds
 17.038 gallons
 Carbon Dioxide: 328.147 pounds
 Carbon Monoxide: 24.250 pounds
 Hydrocarbons: 3.967 pounds
 Nitrogen Oxides: 1.025 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 96.642 pounds
 15.656 gallons
 Carbon Dioxide: 301.522 pounds
 Carbon Monoxide: 23.590 pounds
 Hydrocarbons: 3.831 pounds
 Nitrogen Oxides: 0.901 pounds

DISCLAIMER

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.33	0.00	0.00	0.00	0.59	0.00	0.00	0.59	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	7.0	0.0	0.0	0.0	9.7	0.0	0.0	7.1	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.1	0.0	0.0	0.0	1.1	0.0	0.0	0.7	0.0
HCM2kQueue:	0.0	0.0	0.0	8.1	0.0	0.0	0.0	10.8	0.0	0.0	7.8	0.0
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.18	1.20	1.20	1.18	1.20
HCM2k70thQ:	0.0	0.0	0.0	9.5	0.0	0.0	0.0	12.7	0.0	0.0	9.3	0.0
85thFactor:	1.60	1.60	1.60	1.53	1.60	1.60	1.60	1.51	1.60	1.60	1.53	1.60
HCM2k85thQ:	0.0	0.0	0.0	12.4	0.0	0.0	0.0	16.3	0.0	0.0	12.0	0.0
90thFactor:	1.80	1.80	1.80	1.67	1.80	1.80	1.80	1.63	1.80	1.80	1.67	1.80
HCM2k90thQ:	0.0	0.0	0.0	13.5	0.0	0.0	0.0	17.6	0.0	0.0	13.1	0.0
95thFactor:	2.10	2.10	2.10	1.88	2.10	2.10	2.10	1.83	2.10	2.10	1.89	2.10
HCM2k95thQ:	0.0	0.0	0.0	15.2	0.0	0.0	0.0	19.7	0.0	0.0	14.8	0.0
98thFactor:	2.70	2.70	2.70	2.24	2.70	2.70	2.70	2.14	2.70	2.70	2.25	2.70
HCM2k98thQ:	0.0	0.0	0.0	18.1	0.0	0.0	0.0	23.0	0.0	0.0	17.6	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	121.9	0.0	0.0	0.0	165	0.0	0.0	122	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 59.360 pounds
 9.616 gallons
 Carbon Dioxide: 185.202 pounds
 Carbon Monoxide: 13.878 pounds
 Hydrocarbons: 2.332 pounds
 Nitrogen Oxides: 0.567 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 54.814 pounds
 8.880 gallons
 Carbon Dioxide: 171.019 pounds
 Carbon Monoxide: 13.526 pounds
 Hydrocarbons: 2.260 pounds
 Nitrogen Oxides: 0.501 pounds

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum PM

Intersection #1: Pocket Road/I-5 SB Ramps

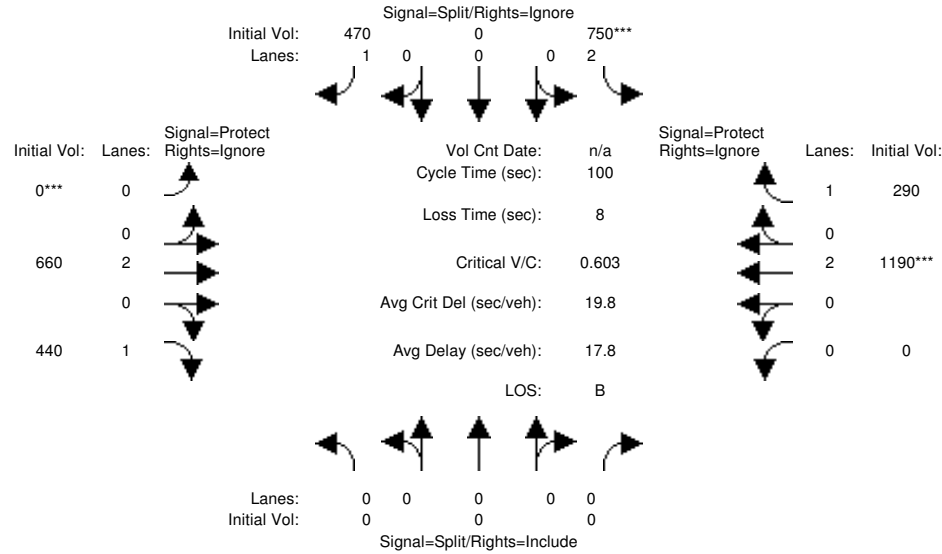


Table containing traffic engineering data including Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module, and Intersection #1 Pocket Road/I-5 SB Ramps.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.00 0.00  0.00  0.36 0.00  0.00  0.00 0.56  0.00  0.00 0.56  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  8.8 0.0  0.0  0.0 5.2  0.0  0.0 11.6  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  0.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  1.5 0.0  0.0  0.0 0.5  0.0  0.0 1.5  0.0
HCM2kQueue:   0.0 0.0  0.0  10.2 0.0  0.0  0.0 5.7  0.0  0.0 13.1  0.0
-----|-----|-----|-----|
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.20  1.20 1.19  1.20  1.20 1.17  1.20
HCM2k70thQ:   0.0 0.0  0.0  12.0 0.0  0.0  0.0 6.8  0.0  0.0 15.3  0.0
-----|-----|-----|-----|
85thFactor:   1.60 1.60  1.60  1.51 1.60  1.60  1.60 1.55  1.60  1.60 1.49  1.60
HCM2k85thQ:   0.0 0.0  0.0  15.5 0.0  0.0  0.0 8.9  0.0  0.0 19.5  0.0
-----|-----|-----|-----|
90thFactor:   1.80 1.80  1.80  1.64 1.80  1.80  1.80 1.70  1.80  1.80 1.61  1.80
HCM2k90thQ:   0.0 0.0  0.0  16.8 0.0  0.0  0.0 9.8  0.0  0.0 21.0  0.0
-----|-----|-----|-----|
95thFactor:   2.10 2.10  2.10  1.84 2.10  2.10  2.10 1.94  2.10  2.10 1.79  2.10
HCM2k95thQ:   0.0 0.0  0.0  18.8 0.0  0.0  0.0 11.1  0.0  0.0 23.4  0.0
-----|-----|-----|-----|
98thFactor:   2.70 2.70  2.70  2.16 2.70  2.70  2.70 2.34  2.70  2.70 2.07  2.70
HCM2k98thQ:   0.0 0.0  0.0  22.0 0.0  0.0  0.0 13.5  0.0  0.0 27.0  0.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #1 Pocket Road/I-5 SB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   0.0 0.0  0.0 153.0  0.0 0.0  0.0 89.7  0.0  0.0 198  0.0
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 63.732 pounds
                  10.325 gallons
Carbon Dioxide:  198.844 pounds
Carbon Monoxide: 15.103 pounds
Hydrocarbons:    2.598 pounds
Nitrogen Oxides: 0.601 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 59.126 pounds
                  9.578 gallons
Carbon Dioxide:  184.473 pounds
Carbon Monoxide: 14.745 pounds
Hydrocarbons:    2.525 pounds
Nitrogen Oxides: 0.532 pounds
    
```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum AM

Intersection #2: Meadowview Road/I-5 NB Ramps

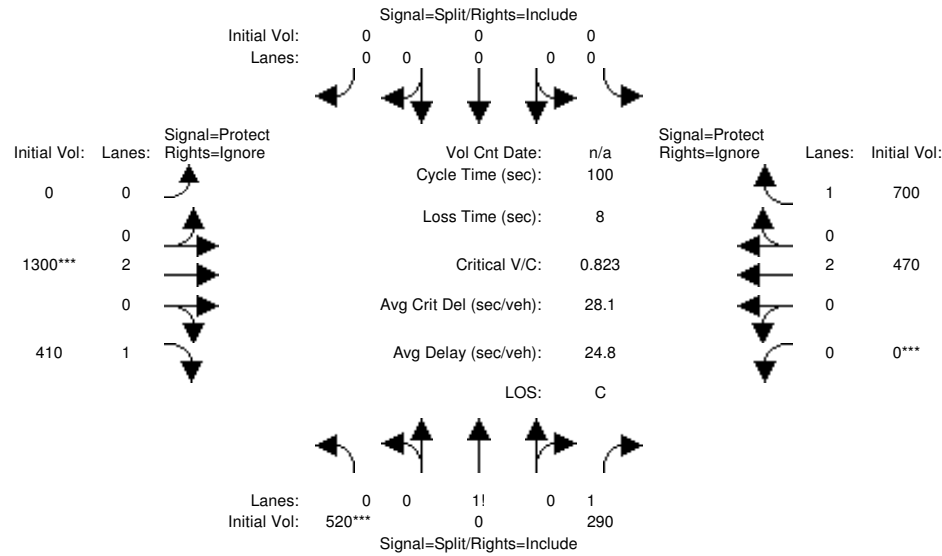


Table with columns: Approach, Movement, North Bound, South Bound, East Bound, West Bound. Rows include Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, and HCM Ops Adjusted Lane Utilization Module.

Table with columns: Lane, LTR, LTR, LTR, xxxxx, xxxxx, xxxxx, T, R, xxxxx, T, R. Rows include #LnsInGrps, HCM Ops Input Saturation Adj Module, and HCM Ops f(lt) Adj Case Module.

Table with columns: Ln Wid Adj, Hev Veh Adj, Grade Adj, Bus Stp Adj, Area Adj, RT Adj, LT Adj, PedBike Adj, HCM Sat Adj, Ustr Sat Adj, MLF Sat Adj, Fnl Sat Adj. Rows include HCM Ops Saturation Adj Module and Delay Adjustment Factor Module.

Table with columns: Coordinated, Signal Type, DelAdjFctr. Rows include Delay Adjustment Factor Module and Level Of Service Detailed Computation Report (HCM2000 Queue Method).

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #2 Meadowview Road/I-5 NB Ramps

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.47 0.00  0.47  0.00 0.00  0.00 0.45  0.00  0.00 0.45  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           15.9 0.0  4.0  0.0 0.0  0.0 16.6  0.0  0.0 4.4  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           3.8 0.0  0.4  0.0 0.0  0.0 3.8  0.0  0.0 0.4  0.0
HCM2KQueue:   19.8 0.0  4.5  0.0 0.0  0.0 20.5  0.0  0.0 4.8  0.0
-----
70thFactor:   1.16 1.20  1.19  1.20 1.20  1.20 1.16  1.20  1.20 1.19  1.20
HCM2k70thQ:   22.9 0.0  5.3  0.0 0.0  0.0 23.7  0.0  0.0 5.7  0.0
-----
85thFactor:   1.46 1.60  1.56  1.60 1.60  1.60 1.45  1.60  1.60 1.56  1.60
HCM2k85thQ:   28.8 0.0  7.0  0.0 0.0  0.0 29.7  0.0  0.0 7.5  0.0
-----
90thFactor:   1.55 1.80  1.72  1.80 1.80  1.80 1.54  1.80  1.80 1.71  1.80
HCM2k90thQ:   30.6 0.0  7.7  0.0 0.0  0.0 31.6  0.0  0.0 8.2  0.0
-----
95thFactor:   1.70 2.10  1.97  2.10 2.10  2.10 1.69  2.10  2.10 1.96  2.10
HCM2k95thQ:   33.6 0.0  8.8  0.0 0.0  0.0 34.6  0.0  0.0 9.4  0.0
-----
98thFactor:   1.92 2.70  2.41  2.70 2.70  2.70 1.91  2.70  2.70 2.39  2.70
HCM2k98thQ:   37.9 0.0  10.8  0.0 0.0  0.0 39.0  0.0  0.0 11.5  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #2 Meadowview Road/I-5 NB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  112.1 0.0  44.4  0.0 0.0  0.0 284  0.0  0.0 75.0  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 76.651 pounds
                  12.417 gallons
Carbon Dioxide:  239.150 pounds
Carbon Monoxide: 18.754 pounds
Hydrocarbons:   3.408 pounds
Nitrogen Oxides: 0.691 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 71.933 pounds
                  11.653 gallons
Carbon Dioxide:  224.432 pounds
Carbon Monoxide: 18.384 pounds
Hydrocarbons:   3.333 pounds
Nitrogen Oxides: 0.617 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.47 0.00  0.47  0.00 0.00  0.00  0.00 0.45  0.00  0.00 0.45  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00  1.00 1.00  1.00
Q1:           13.4 0.0  3.4  0.0 0.0  0.0  0.0 14.3  0.0  0.0 11.3  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 0.00  1.00  0.00  0.00 1.00  0.00
Q2:           2.6 0.0  0.3  0.0 0.0  0.0  0.0  2.6  0.0  0.0  1.6  0.0
HCM2KQueue:   16.0 0.0  3.7  0.0 0.0  0.0  0.0 16.9  0.0  0.0 12.9  0.0
-----
70thFactor:  1.17 1.20  1.19  1.20 1.20  1.20  1.20 1.17  1.20  1.20 1.17  1.20
HCM2k70thQ:  18.6 0.0  4.4  0.0 0.0  0.0  0.0 19.7  0.0  0.0 15.1  0.0
-----
85thFactor:  1.48 1.60  1.57  1.60 1.60  1.60  1.60 1.47  1.60  1.60 1.50  1.60
HCM2k85thQ:  23.6 0.0  5.8  0.0 0.0  0.0  0.0 24.9  0.0  0.0 19.3  0.0
-----
90thFactor:  1.58 1.80  1.73  1.80 1.80  1.80  1.80 1.57  1.80  1.80 1.61  1.80
HCM2k90thQ:  25.2 0.0  6.4  0.0 0.0  0.0  0.0 26.6  0.0  0.0 20.8  0.0
-----
95thFactor:  1.75 2.10  1.99  2.10 2.10  2.10  2.10 1.73  2.10  2.10 1.79  2.10
HCM2k95thQ:  27.9 0.0  7.3  0.0 0.0  0.0  0.0 29.3  0.0  0.0 23.1  0.0
-----
98thFactor:  1.99 2.70  2.45  2.70 2.70  2.70  2.70 1.97  2.70  2.70 2.07  2.70
HCM2k98thQ:  31.8 0.0  9.0  0.0 0.0  0.0  0.0 33.4  0.0  0.0 26.7  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative

```

```

*****
Intersection #2 Meadowview Road/I-5 NB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  95.8 0.0  36.3  0.0 0.0  0.0  0.0 245  0.0  0.0 193  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  82.315 pounds
                  13.335 gallons
Carbon Dioxide:   256.822 pounds
Carbon Monoxide:  19.982 pounds
Hydrocarbons:     3.580 pounds
Nitrogen Oxides:  0.755 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  77.017 pounds
                  12.477 gallons
Carbon Dioxide:   240.294 pounds
Carbon Monoxide:  19.567 pounds
Hydrocarbons:     3.496 pounds
Nitrogen Oxides:  0.672 pounds
-----

```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum AM

Intersection #3: Meadowview Road/Freeport Boulevard

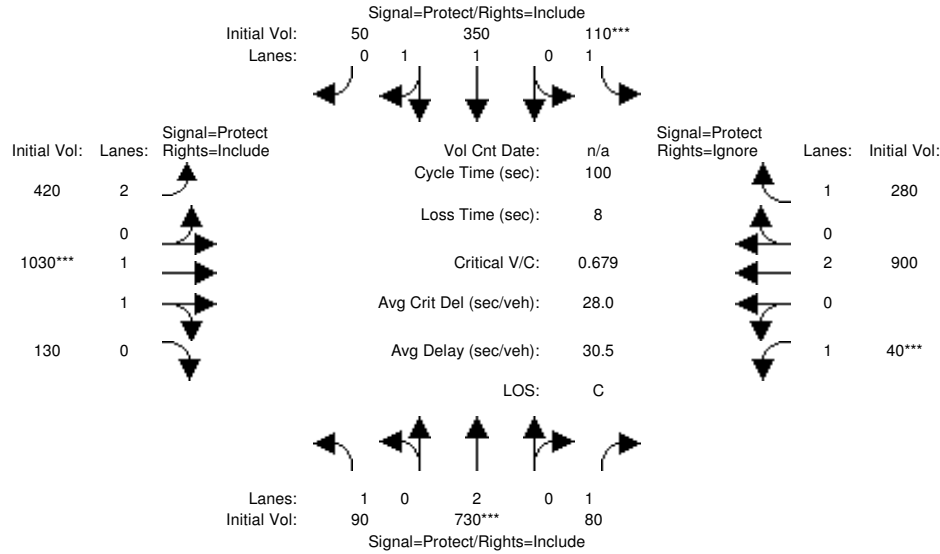


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.12 0.30  0.30  0.09 0.27  0.27  0.17 0.48  0.48  0.05 0.36  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           2.3  9.4  1.6  3.0  4.8  4.8  5.7 13.2  13.2  1.1 11.3  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  0.00
Q2:           0.7  2.1  0.2  1.7  0.7  0.7  2.1  2.1  2.1  0.7  2.2  0.0
HCM2kQueue:   3.0 11.5  1.8  4.7  5.6  5.6  7.7 15.3  15.3  1.8 13.5  0.0
-----
70thFactor:   1.19 1.18  1.20  1.19 1.19  1.19  1.18 1.17  1.17  1.20 1.17  1.20
HCM2k70thQ:   3.6 13.5  2.2  5.6  6.6  6.6  9.1 17.9  17.9  2.2 15.9  0.0
-----
85thFactor:   1.57 1.50  1.58  1.56 1.55  1.55  1.53 1.48  1.48  1.58 1.49  1.60
HCM2k85thQ:   4.8 17.3  2.9  7.3  8.6  8.6  11.8 22.6  22.6  2.9 20.2  0.0
-----
90thFactor:   1.74 1.63  1.76  1.72 1.70  1.70  1.67 1.59  1.59  1.77 1.60  1.80
HCM2k90thQ:   5.3 18.7  3.3  8.1  9.5  9.5  12.9 24.3  24.3  3.2 21.7  0.0
-----
95thFactor:   2.01 1.82  2.04  1.96 1.94  1.94  1.89 1.76  1.76  2.04 1.78  2.10
HCM2k95thQ:   6.1 20.9  3.8  9.2 10.8  10.8  14.6 26.9  26.9  3.7 24.1  0.0
-----
98thFactor:   2.49 2.11  2.57  2.40 2.35  2.35  2.25 2.01  2.01  2.57 2.05  2.70
HCM2k98thQ:   7.6 24.3  4.7  11.3 13.1  13.1  17.4 30.7  30.7  4.7 27.8  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #3 Meadowview Road/Freeport Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  20.9 161 14.8  26.7 72.3 10.3  99.0 200 25.3  9.7 193 0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 132.673 pounds
                  21.493 gallons
Carbon Dioxide:  413.939 pounds
Carbon Monoxide: 33.067 pounds
Hydrocarbons:    6.205 pounds
Nitrogen Oxides: 1.148 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 125.397 pounds
                  20.314 gallons
Carbon Dioxide:  391.239 pounds
Carbon Monoxide: 32.494 pounds
Hydrocarbons:    6.090 pounds
Nitrogen Oxides: 1.031 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.10 0.10  0.10  0.35 0.36  0.36  0.09 0.42  0.42  0.05 0.37  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           4.1 1.8  2.1  9.9 14.0  14.0  3.5 16.5  16.5  1.4 13.8  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  0.00
Q2:           3.0 0.5  0.9  1.9 4.3  4.3  2.5 4.5  4.5  1.1 3.4  0.0
HCM2kQueue:   7.1 2.3  3.0  11.9 18.3  18.3  6.0 21.0  21.0  2.4 17.2  0.0
-----
70thFactor:   1.18 1.19  1.19  1.17 1.16  1.16  1.19 1.16  1.16  1.19 1.17  1.20
HCM2k70thQ:   8.5 2.7  3.6  13.9 21.3  21.3  7.1 24.3  24.3  2.9 20.0  0.0
-----
85thFactor:   1.54 1.58  1.57  1.50 1.46  1.46  1.55 1.45  1.45  1.58 1.47  1.60
HCM2k85thQ:   11.0 3.6  4.7  17.8 26.8  26.8  9.3 30.4  30.4  3.8 25.3  0.0
-----
90thFactor:   1.68 1.76  1.74  1.62 1.56  1.56  1.70 1.54  1.54  1.75 1.57  1.80
HCM2k90thQ:   12.0 4.0  5.2  19.2 28.6  28.6  10.2 32.3  32.3  4.2 27.0  0.0
-----
95thFactor:   1.90 2.03  2.01  1.81 1.72  1.72  1.93 1.69  1.69  2.02 1.73  2.10
HCM2k95thQ:   13.6 4.7  6.0  21.5 31.4  31.4  11.6 35.4  35.4  4.9 29.8  0.0
-----
98thFactor:   2.28 2.54  2.50  2.10 1.94  1.94  2.33 1.90  1.90  2.53 1.97  2.70
HCM2k98thQ:   16.3 5.8  7.4  24.9 35.6  35.6  14.0 39.8  39.8  6.1 33.8  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

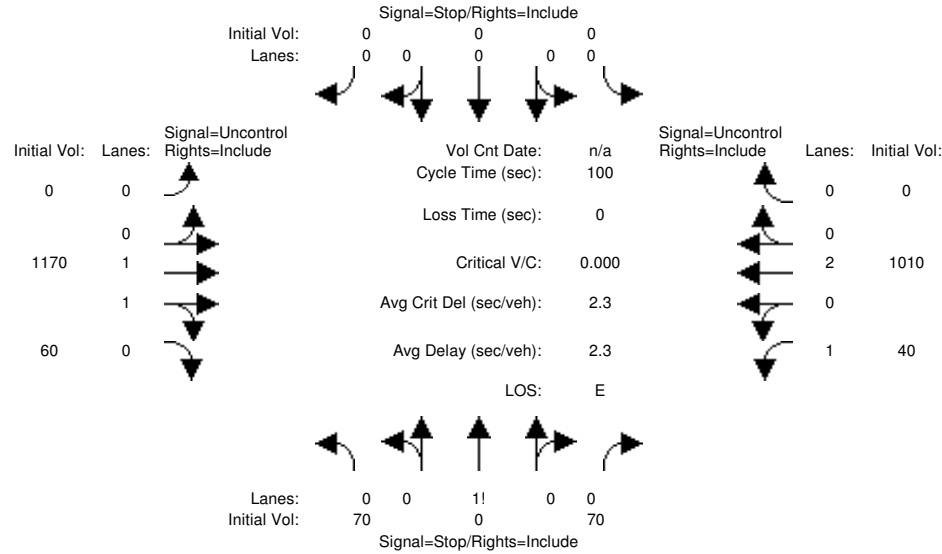
*****
Intersection #3 Meadowview Road/Freeport Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  36.9 30.2  18.9  89.4 167  71.9  61.3 250  31.8  12.2 237  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 168.014 pounds
                  27.218 gallons
Carbon Dioxide:  524.205 pounds
Carbon Monoxide: 42.508 pounds
Hydrocarbons:   8.174 pounds
Nitrogen Oxides: 1.407 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 159.718 pounds
                  25.874 gallons
Carbon Dioxide:  498.322 pounds
Carbon Monoxide: 41.852 pounds
Hydrocarbons:   8.043 pounds
Nitrogen Oxides: 1.271 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum AM

Intersection #4: Meadowview Road/Manorside Drive



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	70 0 70	0 0 0	0 1170 60	40 1010 0
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	70 0 70	0 0 0	0 1170 60	40 1010 0
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Volume:	70 0 70	0 0 0	0 1170 60	40 1010 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Volume:	70 0 70	0 0 0	0 1170 60	40 1010 0
Critical Gap Module:				
Critical Gap:	6.8 6.5 6.9	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx	4.1 xxxxx xxxxx
FollowUpTim:	3.5 4.0 3.3	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx	2.2 xxxxx xxxxx
Capacity Module:				
Cnflct Vol:	1785 2290 615	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	1230 xxxxx xxxxx
Potent Cap.:	73 39 434	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	562 xxxxx xxxxx
Move Cap.:	69 36 434	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	562 xxxxx xxxxx
Total Cap:	177 136 xxxxxx	142 122 xxxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Volume/Cap:	0.40 0.00 0.16	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	0.07 xxxxx xxxxx

Level Of Service Module:	North Bound	South Bound	East Bound	West Bound
ZWay95thQ:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	0.2 xxxxx xxxxx
Control Del:	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	11.9 xxxxx xxxxx
LOS by Move:	* * *	* * *	* * *	B * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx 251 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Shared Queue:	xxxxx 3.1 xxxxx	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx
Shrd ConDel:	xxxxx 35.9 xxxxx	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx	xxxxx xxxxx xxxxx
Shared LOS:	* E	* * *	* * *	* * *
ApproachDel:	35.9	xxxxxxx	xxxxxxx	xxxxxxx
ApproachLOS:	E	*	*	*

HevVeh:	2%	2%	2%	2%
Grade:	0%	0%	0%	0%
Peds/Hour:	0	0	0	0
Pedestrian Walk Speed:	4.00 feet/sec			
LaneWidth:	12 feet	12 feet	12 feet	12 feet
Time Period:	0.25 hour			

Two-Stage Gap Acceptance [Median Type: TWLTL] [Median Storage: 1 car]				
Two-Stage Gap Acceptance - Stage One Module:				
Cnflct Vol:	1200 1200 xxxxx	1090 1090 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Potent Cap.:	248 256 xxxxx	230 289 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Move Cap.:	248 256 xxxxx	213 269 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Two-Stage Gap Acceptance - Stage Two Module:				
Cnflct Vol:	585 1090 xxxxx	585 1230 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Potent Cap.:	520 289 xxxxx	464 248 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx
Move Cap.:	483 269 xxxxx	389 248 xxxxx	xxxx xxxxx xxxxx	xxxx xxxxx xxxxx

 Intersection #4 Meadowview Road/Manorside Drive

 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0 0	0 0 0 0 0 0	0 0 1 1 0 0	1 0 2 0 0 0
Initial Vol:	70 0 70	0 0 0 0	0 1170 60	40 1010 0
ApproachDel:	35.9	xxxxxxx	xxxxxxx	xxxxxxx

Approach[northbound] [lanes=1] [control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=1.4]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=140]
 SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3] [total volume=2420]
 SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	70	0	70	0	0	0	0	1170	60	40	1010	0
Major Street Volume:	2280											
Minor Approach Volume:	140											
Minor Approach Volume Threshold:	1 [less than minimum of 100]											

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum PM

Intersection #4: Meadowview Road/Manorside Drive

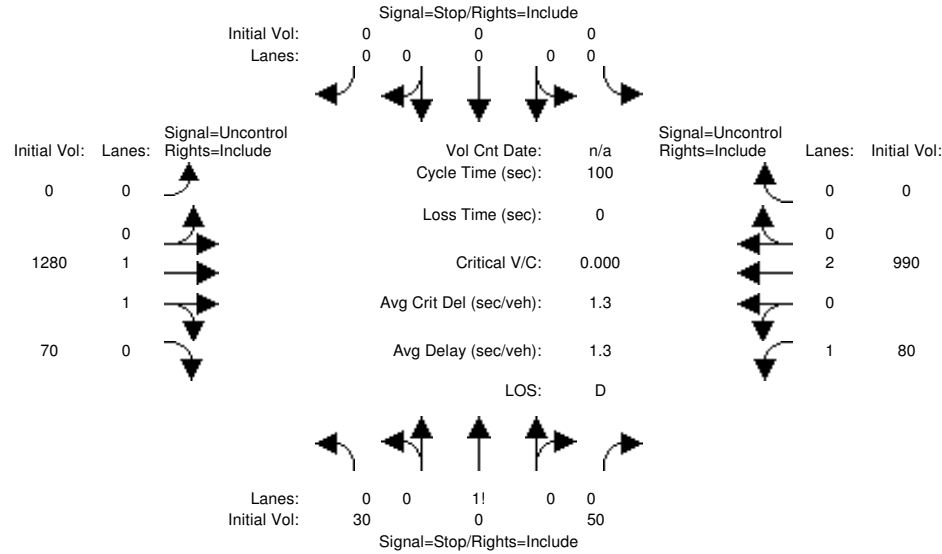


Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement (L, T, R), Volume Module (Base Vol, Growth Adj, Initial Bse, User Adj, PHF Volume, Reduct Vol, Final Volume), Critical Gap Module (Critical Gap, FollowUpTim), Capacity Module (Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap).

Level Of Service Module: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period.

Two-Stage Gap Acceptance [Median Type: TWLTL] [Median Storage: 1 car]
Two-Stage Gap Acceptance - Stage One Module: Conflict Vol, Potent Cap, Move Cap.
Two-Stage Gap Acceptance - Stage Two Module: Conflict Vol, Potent Cap, Move Cap.

Intersection #4 Meadowview Road/Manorside Drive
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement (L, T, R), Control (Stop Sign, Stop Sign, Uncontrolled, Uncontrolled), Lanes, Initial Vol, ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.6]
Signal Warrant Rule #2: [approach volume=80]
Signal Warrant Rule #3: [approach count=3][total volume=2500]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound					
Movement:	L	T	R		L	T	R		L	T	R		L	T	R			
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled					
Lanes:	0	0	1	0	0	0	0	0	0	0	1	1	0	1	0	2	0	0
Initial Vol:	30	0	50		0	0	0	0	0	1280	70		80	990	0			
Major Street Volume:	2420																	
Minor Approach Volume:	80																	
Minor Approach Volume Threshold:	-20 [less than minimum of 100]																	

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.20 0.23  0.23  0.13 0.16  0.16  0.14 0.23  0.23  0.24 0.34  0.34
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           3.9 2.8  7.9  4.9 1.7  2.5  5.4 9.3  3.0  9.2 11.6  11.5
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.8 0.4  3.1  2.8 0.3  0.6  3.0 3.2  0.5  3.2 2.6  3.7
HCM2kQueue:    4.7 3.1  11.0  7.6 2.0  3.1  8.5 12.5  3.5  12.4 14.1  15.2
-----
70thFactor:    1.19 1.19  1.18  1.18 1.20  1.19  1.18 1.17  1.19  1.17 1.17  1.17
HCM2k70thQ:    5.6 3.7  12.9  9.0 2.3  3.8  10.0 14.6  4.2  14.5 16.6  17.7
-----
85thFactor:    1.56 1.57  1.51  1.53 1.58  1.57  1.53 1.50  1.57  1.50 1.49  1.48
HCM2k85thQ:    7.3 4.9  16.6  11.7 3.1  4.9  12.9 18.7  5.5  18.5 21.0  22.4
-----
90thFactor:    1.72 1.74  1.63  1.67 1.76  1.74  1.66 1.61  1.74  1.62 1.60  1.59
HCM2k90thQ:    8.0 5.5  17.9  12.8 3.5  5.5  14.1 20.1  6.1  20.0 22.6  24.1
-----
95thFactor:    1.96 2.00  1.83  1.89 2.04  2.00  1.88 1.80  1.99  1.80 1.77  1.76
HCM2k95thQ:    9.2 6.3  20.1  14.5 4.0  6.3  15.9 22.4  7.1  22.3 25.1  26.7
-----
98thFactor:    2.40 2.49  2.13  2.26 2.56  2.48  2.22 2.08  2.46  2.09 2.04  2.01
HCM2k98thQ:    11.2 7.8  23.4  17.2 5.0  7.8  18.8 26.0  8.7  25.8 28.8  30.5
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #5 Meadowview Road/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    35.0 47.1  70.8  43.8 28.4  22.5  48.7 158  27.1  82.4 198  103.3
-----
Name: year 1995 composite fleet
Fuel Consumption: 156.039 pounds
                  25.278 gallons
Carbon Dioxide:  486.841 pounds
Carbon Monoxide: 39.835 pounds
Hydrocarbons:    7.785 pounds
Nitrogen Oxides: 1.266 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 148.893 pounds
                  24.121 gallons
Carbon Dioxide:  464.545 pounds
Carbon Monoxide: 39.270 pounds
Hydrocarbons:    7.672 pounds
Nitrogen Oxides: 1.149 pounds
    
```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum PM

Intersection #5: Meadowview Road/24th Street

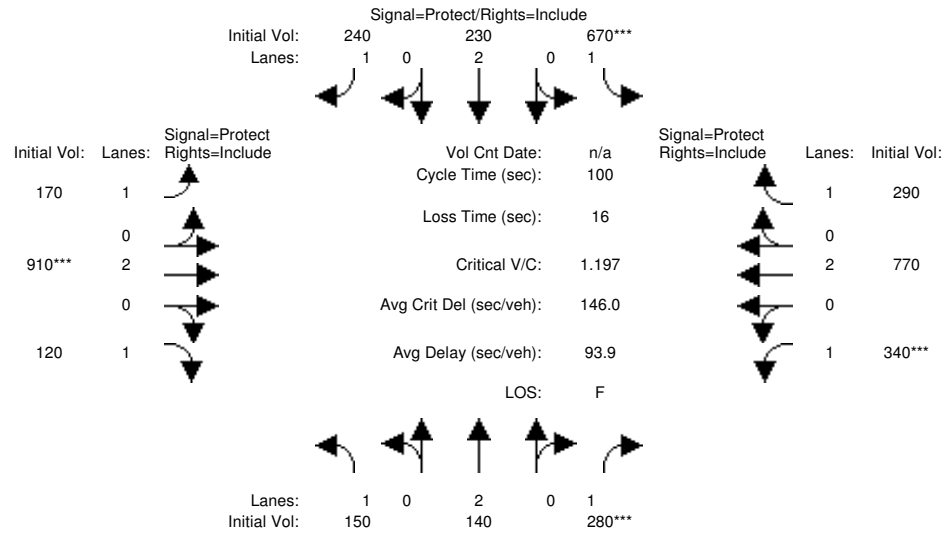


Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Min. Green, Volume Module, Saturation Flow Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: HCM Ops Saturation Adj Module, Delay Adjustment Factor Module, Signal Type, DelAdjFctr.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #5 Meadowview Road/24th Street

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.17 0.15 0.15  0.32 0.30 0.30  0.12 0.21 0.21  0.16 0.26 0.26
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00
Q1:            3.8 1.8 7.8  18.6 2.5 5.5  4.6 13.3 2.8  9.4 10.6 7.3
UpstreamVC:    0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00
UpstreamAdj:   0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00
EarlyArrAdj:   1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00
Q2:           1.0 0.4 9.5  18.3 0.3 1.0  3.0 14.1 0.5  10.9 3.7 2.1
HCM2kQueue:    4.8 2.2 17.2  36.9 2.8 6.5  7.6 27.4 3.4  20.3 14.3 9.4
-----
70thFactor:   1.19 1.19 1.17  1.14 1.19 1.18  1.18 1.15 1.19  1.16 1.17 1.18
HCM2k70thQ:   5.7 2.6 20.1  42.1 3.3 7.7  9.0 31.5 4.0  23.6 16.8 11.1
-----
85thFactor:   1.56 1.58 1.47  1.39 1.57 1.54  1.53 1.42 1.57  1.45 1.49 1.52
HCM2k85thQ:   7.4 3.4 25.3  51.3 4.4 10.1  11.6 38.9 5.3  29.5 21.3 14.3
-----
90thFactor:   1.71 1.76 1.57  1.46 1.75 1.69  1.67 1.50 1.74  1.54 1.60 1.65
HCM2k90thQ:   8.2 3.8 27.0  54.0 4.9 11.0  12.7 41.1 5.9  31.4 22.9 15.5
-----
95thFactor:   1.96 2.03 1.73  1.58 2.01 1.92  1.89 1.63 2.00  1.69 1.77 1.86
HCM2k95thQ:   9.4 4.4 29.8  58.3 5.6 12.5  14.4 44.7 6.7  34.4 25.4 17.4
-----
98thFactor:   2.39 2.55 1.97  1.76 2.51 2.31  2.26 1.82 2.47  1.91 2.03 2.19
HCM2k98thQ:  11.4 5.5 33.9  65.0 7.0 15.0  17.1 49.9 8.3  38.8 29.1 20.5
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #5 Meadowview Road/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  34.2 31.1  72.5 184.3 43.2 49.7  41.6 240 25.5  88.3 182 65.6
-----
Name: year 1995 composite fleet
Fuel Consumption: 335.479 pounds
                  54.348 gallons
Carbon Dioxide:  1046.693 pounds
Carbon Monoxide: 90.244 pounds
Hydrocarbons:   19.193 pounds
Nitrogen Oxides: 2.228 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 327.230 pounds
                  53.011 gallons
Carbon Dioxide:  1020.957 pounds
Carbon Monoxide: 89.589 pounds
Hydrocarbons:   19.063 pounds
Nitrogen Oxides: 2.090 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum AM

Intersection #6: Meadowview Road/Detroit Boulevard

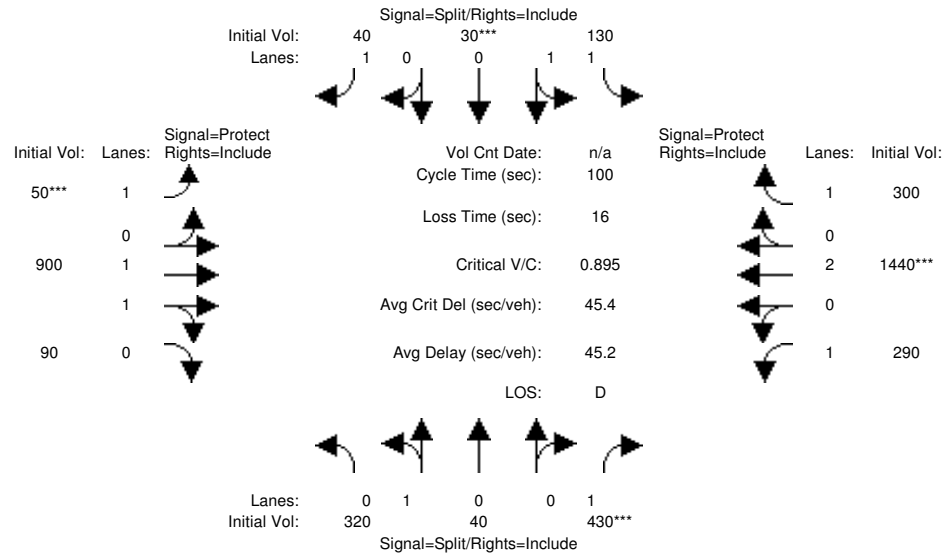


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.29 0.29  0.29  0.07 0.07  0.07  0.05 0.31  0.31  0.18 0.43  0.43
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           8.9  8.9 11.7  2.2  2.2  1.1  1.4 14.0  14.0  7.9 20.2  5.8
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           2.1  2.1  5.9  1.4  1.4  0.5  1.1  6.0  6.0  4.8  7.3  0.8
HCM2kQueue:   11.0 11.0  17.6  3.6  3.6  1.6  2.4 20.0  20.0  12.7 27.4  6.6
-----
70thFactor:   1.18 1.18  1.16  1.19 1.19  1.20  1.19 1.16  1.16  1.17 1.15  1.18
HCM2k70thQ:   13.0 13.0  20.4  4.3  4.3  1.9  2.9 23.2  23.2  14.9 31.6  7.8
-----
85thFactor:   1.51 1.51  1.47  1.57 1.57  1.58  1.58 1.45  1.45  1.50 1.42  1.54
HCM2k85thQ:   16.6 16.6  25.7  5.6  5.6  2.5  3.8 29.1  29.1  19.1 39.0  10.2
-----
90thFactor:   1.63 1.63  1.57  1.73 1.73  1.77  1.75 1.55  1.55  1.61 1.50  1.69
HCM2k90thQ:   18.0 18.0  27.5  6.2  6.2  2.8  4.2 30.9  30.9  20.5 41.2  11.2
-----
95thFactor:   1.83 1.83  1.73  1.99 1.99  2.05  2.02 1.70  1.70  1.80 1.63  1.92
HCM2k95thQ:   20.1 20.1  30.3  7.1  7.1  3.3  4.9 34.0  34.0  22.9 44.7  12.7
-----
98thFactor:   2.13 2.13  1.96  2.46 2.46  2.58  2.53 1.91  1.91  2.08 1.82  2.30
HCM2k98thQ:   23.5 23.5  34.4  8.8  8.8  4.1  6.1 38.3  38.3  26.4 50.0  15.2
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #6 Meadowview Road/Detroit Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  71.4  8.9 105.1  31.6  7.3  9.5  12.2 21.8  21.8  71.4  345  52.6
-----
Name: year 1995 composite fleet
Fuel Consumption:  179.523 pounds
                  29.083 gallons
Carbon Dioxide:   560.111 pounds
Carbon Monoxide:  46.187 pounds
Hydrocarbons:     9.139 pounds
Nitrogen Oxides:  1.426 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  171.831 pounds
                  27.837 gallons
Carbon Dioxide:   536.112 pounds
Carbon Monoxide:  45.578 pounds
Hydrocarbons:     9.017 pounds
Nitrogen Oxides:  1.299 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.20 0.20  0.20  0.09 0.09  0.09  0.07 0.38  0.38  0.17 0.48  0.48
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           2.3 2.3 11.9  5.7 5.7  1.6  0.8 25.6  25.6 11.1 13.7  2.4
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.4 0.4 16.6  9.2 9.2  0.7  0.3 32.2  32.2 15.6 2.2  0.2
HCM2kQueue:   2.7 2.7 28.6 14.9 14.9  2.3  1.1 57.8  57.8 26.8 15.9  2.6
-----
70thFactor:   1.19 1.19  1.15  1.17 1.17  1.19  1.20 1.12  1.12  1.15 1.17  1.19
HCM2k70thQ:   3.2 3.2 32.8 17.4 17.4  2.8  1.3 65.0  65.0 30.8 18.5  3.2
-----
85thFactor:   1.57 1.57  1.42  1.48 1.48  1.58  1.59 1.34  1.34  1.42 1.48  1.57
HCM2k85thQ:   4.3 4.3 40.4 22.0 22.0  3.6  1.7 77.7  77.7 38.1 23.5  4.2
-----
90thFactor:   1.75 1.75  1.50  1.59 1.59  1.76  1.78 1.42  1.42  1.50 1.58  1.75
HCM2k90thQ:   4.7 4.7 42.7 23.6 23.6  4.1  1.9 82.3  82.3 40.3 25.1  4.6
-----
95thFactor:   2.02 2.02  1.62  1.76 1.76  2.03  2.06 1.52  1.52  1.64 1.75  2.02
HCM2k95thQ:   5.5 5.5 46.4 26.2 26.2  4.7  2.3 88.1  88.1 43.8 27.8  5.3
-----
98thFactor:   2.51 2.51  1.81  2.02 2.02  2.54  2.62 1.71  1.71  1.83 1.99  2.52
HCM2k98thQ:   6.8 6.8 51.7 30.0 30.0  5.9  2.9 99.0  99.0 48.9 31.7  6.7
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #6 Meadowview Road/Detroit Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  14.7 6.3 117.5  95.5 10.3 14.2  7.1 467  81.6 107.3  234 21.6
-----
Name: year 1995 composite fleet
Fuel Consumption:  489.537 pounds
                  79.305 gallons
Carbon Dioxide:   1527.355 pounds
Carbon Monoxide:  133.794 pounds
Hydrocarbons:    29.132 pounds
Nitrogen Oxides:  3.025 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  480.759 pounds
                  77.883 gallons
Carbon Dioxide:   1499.969 pounds
Carbon Monoxide:  133.094 pounds
Hydrocarbons:    28.994 pounds
Nitrogen Oxides:  2.875 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum AM

Intersection #7: Mack Road/Franklin Boulevard

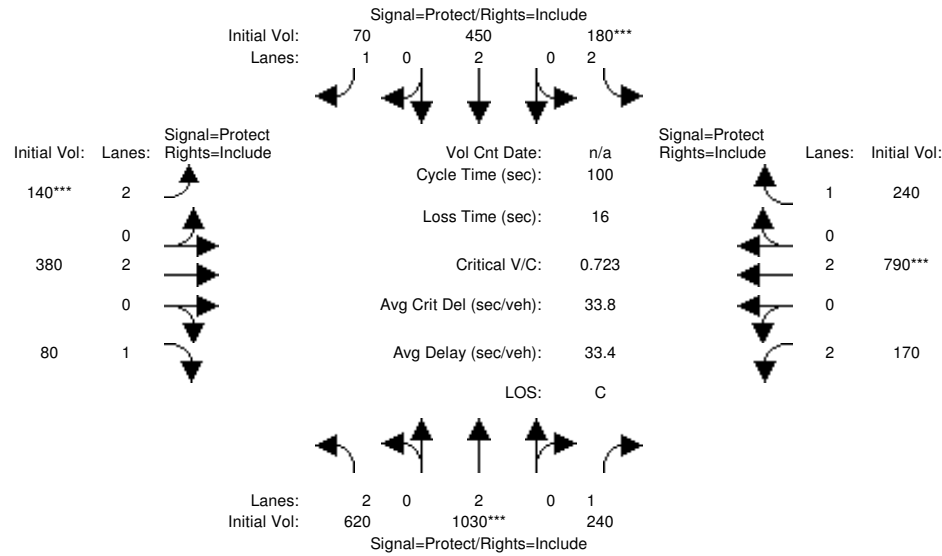


Table with 4 columns: Approach, Movement, Min. Green, Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with 4 columns: Sat/Lane, Adjustment, Lanes, Final Sat. Rows include Sat/Lane, Adjustment, Lanes, Final Sat.

Table with 4 columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Table with 4 columns: Lane Group, #LnsInGrps. Rows include Lane Group, #LnsInGrps.

Table with 4 columns: Lane Width, CrsswalkWid, % Hev Veh, Grade, Parking/Hr, Bus Stp/Hr, Area Type, Cnft Ped/Hr, ExclusiveRT, % RT Prtct.

Table with 4 columns: f(lt) Case. Rows include f(lt) Case.

Table with 4 columns: Ln Wid Adj, Hev Veh Adj, Grade Adj, Parking Adj, Bus Stp Adj, Area Adj, RT Adj, LT Adj, PedBike Adj, HCM Sat Adj, Ustr Sat Adj, MLF Sat Adj, Fnl Sat Adj.

Table with 4 columns: Coordinated, Signal Type, DelAdjFctr. Rows include Coordinated, Signal Type, DelAdjFctr.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.28 0.40  0.40  0.07 0.20  0.20  0.06 0.25  0.25  0.12 0.31  0.31
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           7.8 12.7  4.7  2.5 6.1  1.6  2.0 4.7  1.8  2.3 10.3  5.4
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.7  2.4  0.6  1.8 1.7  0.3  1.7 0.7  0.3  0.7 2.3  0.9
HCM2kQueue:   9.5 15.1  5.3  4.4 7.7  1.9  3.7 5.4  2.0  3.0 12.6  6.4
-----
70thFactor:   1.18 1.17  1.19  1.19 1.18  1.20  1.19 1.19  1.20  1.19 1.17  1.19
HCM2k70thQ:   11.2 17.6  6.3  5.2 9.1  2.3  4.4 6.4  2.4  3.5 14.8  7.6
-----
85thFactor:   1.52 1.48  1.55  1.56 1.53  1.58  1.57 1.55  1.58  1.57 1.50  1.54
HCM2k85thQ:   14.5 22.4  8.2  6.8 11.8  3.0  5.8 8.4  3.2  4.7 18.9  9.8
-----
90thFactor:   1.65 1.59  1.71  1.72 1.67  1.76  1.73 1.71  1.76  1.74 1.61  1.69
HCM2k90thQ:   15.7 24.0  9.0  7.5 12.9  3.4  6.4 9.2  3.5  5.2 20.4  10.8
-----
95thFactor:   1.85 1.76  1.95  1.97 1.89  2.04  1.99 1.94  2.04  2.01 1.80  1.92
HCM2k95thQ:   17.7 26.5  10.3  8.6 14.6  3.9  7.4 10.5  4.1  6.0 22.7  12.2
-----
98thFactor:   2.18 2.01  2.37  2.41 2.25  2.56  2.45 2.36  2.56  2.50 2.08  2.31
HCM2k98thQ:   20.8 30.4  12.5  10.6 17.4  4.9  9.1 12.8  5.1  7.4 26.2  14.7
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #7 Mack Road/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  136.5 217  42.3  44.0  104 14.7  34.4 79.9  15.8  39.5 176  48.9
-----
Name: year 1995 composite fleet
Fuel Consumption:  157.413 pounds
                  25.501 gallons
Carbon Dioxide:   491.128 pounds
Carbon Monoxide:  39.545 pounds
Hydrocarbons:     7.523 pounds
Nitrogen Oxides:  1.334 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  149.246 pounds
                  24.178 gallons
Carbon Dioxide:   465.647 pounds
Carbon Monoxide:  38.901 pounds
Hydrocarbons:     7.394 pounds
Nitrogen Oxides:  1.203 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.12 0.22  0.22  0.12 0.22  0.22  0.08 0.42  0.42  0.08 0.41  0.41
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           6.3  9.1  5.8  5.0 12.0  2.8  1.5 10.5  19.2  4.2 10.0  5.8
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           6.0  3.6  1.8  3.1  8.7  0.5  0.6  1.6  11.5  4.8  1.5  0.8
HCM2KQueue:   12.3 12.7  7.6  8.1 20.7  3.3  2.1 12.0  30.6  8.9 11.5  6.6
-----
70thFactor:   1.17 1.17  1.18  1.18 1.16  1.19  1.19 1.17  1.15  1.18 1.18  1.18
HCM2k70thQ:   14.5 14.9  9.0  9.6 24.0  4.0  2.5 14.1  35.1  10.5 13.5  7.8
-----
85thFactor:   1.50 1.50  1.53  1.53 1.45  1.57  1.58 1.50  1.41  1.52 1.50  1.54
HCM2k85thQ:   18.5 19.1  11.7  12.4 30.0  5.2  3.3 18.0  43.1  13.6 17.3  10.1
-----
90thFactor:   1.62 1.61  1.67  1.67 1.54  1.74  1.76 1.62  1.49  1.66 1.63  1.69
HCM2k90thQ:   19.9 20.5  12.7  13.6 31.9  5.8  3.7 19.5  45.6  14.8 18.7  11.1
-----
95thFactor:   1.80 1.80  1.89  1.88 1.69  2.00  2.03 1.81  1.61  1.87 1.82  1.92
HCM2k95thQ:   22.2 22.9  14.4  15.3 34.9  6.6  4.2 21.7  49.3  16.7 20.9  12.6
-----
98thFactor:   2.09 2.08  2.26  2.23 1.90  2.47  2.55 2.10  1.79  2.20 2.11  2.30
HCM2k98thQ:   25.7 26.4  17.2  18.2 39.4  8.2  5.3 25.2  55.0  19.7 24.3  15.1
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #7 Mack Road/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  110.7 156  52.6  88.0  208  25.3  26.0 179  178.6  72.8  172  52.1
-----
Name: year 1995 composite fleet
Fuel Consumption: 293.311 pounds
                  47.516 gallons
Carbon Dioxide:  915.131 pounds
Carbon Monoxide: 76.508 pounds
Hydrocarbons:   15.511 pounds
Nitrogen Oxides: 2.197 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 282.420 pounds
                  45.752 gallons
Carbon Dioxide:  881.151 pounds
Carbon Monoxide: 75.647 pounds
Hydrocarbons:   15.338 pounds
Nitrogen Oxides: 2.019 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.23 0.44  0.44  0.10 0.31  0.31  0.05 0.21  0.21  0.09 0.25  0.25
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            6.8 23.5  8.4   5.3 8.2   7.7   1.7 8.6   2.8   3.4 13.5  10.8
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            1.7 12.6  1.3   5.4 1.5  1.8   1.6 3.3  0.5   2.6 9.1   6.7
HCM2kQueue:    8.5 36.2  9.7  10.7 9.7   9.5   3.3 11.9  3.4   6.0 22.5  17.5
-----|-----|-----|-----|
70thFactor:    1.18 1.14  1.18  1.18 1.18  1.18  1.19 1.17  1.19  1.19 1.16  1.16
HCM2k70thQ:    10.1 41.2  11.4  12.6 11.4  11.2   3.9 14.0  4.0   7.1 26.1  20.3
-----|-----|-----|-----|
85thFactor:    1.53 1.39  1.52  1.51 1.52  1.52  1.57 1.50  1.57  1.55 1.44  1.47
HCM2k85thQ:    13.0 50.3  14.7  16.1 14.7  14.4   5.1 17.9  5.3   9.2 32.5  25.6
-----|-----|-----|-----|
90thFactor:    1.66 1.47  1.65  1.63 1.65  1.65  1.74 1.62  1.74  1.70 1.53  1.57
HCM2k90thQ:    14.2 53.0  16.0  17.5 15.9  15.6   5.7 19.3  5.9  10.1 34.5  27.4
-----|-----|-----|-----|
95thFactor:    1.87 1.58  1.85  1.83 1.85  1.85  2.00 1.81  2.00  1.93 1.67  1.73
HCM2k95thQ:    16.0 57.2  17.9  19.6 17.9  17.6   6.5 21.6  6.7  11.5 37.7  30.2
-----|-----|-----|-----|
98thFactor:    2.22 1.76  2.17  2.14 2.18  2.18  2.48 2.10  2.47  2.33 1.88  1.96
HCM2k98thQ:    18.9 63.7  21.1  22.9 21.0  20.7   8.1 25.1  8.3  13.9 42.3  34.2
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #8 Cosumnes River Boulevard/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    61.1 41.6  75.4  140 69.4  29.5 148  25.5  59.0 233 97.1
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 317.908 pounds
                    51.501 gallons
Carbon Dioxide:  991.872 pounds
Carbon Monoxide: 83.163 pounds
Hydrocarbons:   16.918 pounds
Nitrogen Oxides: 2.376 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 306.415 pounds
                    49.639 gallons
Carbon Dioxide:  956.015 pounds
Carbon Monoxide: 82.252 pounds
Hydrocarbons:   16.737 pounds
Nitrogen Oxides: 2.185 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:   0.08 0.29 0.29 0.15 0.36 0.36 0.11 0.24 0.24 0.16 0.29 0.29
ArrivalType:   3          3          3          3
ProgFactor:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Q1:           4.2 9.9 7.6 5.3 18.4 3.4 4.2 12.3 5.7 8.0 10.4 7.6
UpstreamVC:   0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
UpstreamAdj:  0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
EarlyArrAdj:  1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Q2:           4.3 2.5 1.9 2.3 9.2 0.4 2.4 7.5 1.5 6.0 2.8 2.0
HCM2kQueue:   8.5 12.4 9.6 7.6 27.6 3.8 6.5 19.8 7.2 14.1 13.3 9.6
-----
70thFactor:   1.18 1.17 1.18 1.18 1.15 1.19 1.18 1.16 1.18 1.17 1.17 1.18
HCM2k70thQ:   10.1 14.6 11.3 9.0 31.7 4.6 7.8 22.9 8.5 16.5 15.5 11.3
-----
85thFactor:   1.53 1.50 1.52 1.53 1.42 1.56 1.54 1.46 1.54 1.49 1.49 1.52
HCM2k85thQ:   13.0 18.6 14.6 11.6 39.2 6.0 10.1 28.8 11.0 20.9 19.8 14.6
-----
90thFactor:   1.66 1.62 1.65 1.67 1.50 1.73 1.69 1.55 1.68 1.60 1.61 1.65
HCM2k90thQ:   14.1 20.1 15.8 12.7 41.4 6.6 11.1 30.6 12.0 22.5 21.3 15.8
-----
95thFactor:   1.87 1.80 1.85 1.89 1.63 1.98 1.92 1.70 1.90 1.77 1.79 1.85
HCM2k95thQ:   16.0 22.4 17.8 14.4 45.0 7.6 12.5 33.6 13.6 25.0 23.7 17.8
-----
98thFactor:   2.22 2.08 2.18 2.26 1.82 2.44 2.30 1.92 2.28 2.04 2.06 2.18
HCM2k98thQ:   18.9 25.9 20.9 17.1 50.2 9.4 15.1 37.9 16.3 28.7 27.3 20.9
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #8 Cosumnes River Boulevard/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:     30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   37.5 170 68.8 93.1 315 30.7 72.9 210 51.3 140.1 179 68.8
-----
Name: year 1995 composite fleet
Fuel Consumption: 298.038 pounds
                  48.282 gallons
Carbon Dioxide:  929.877 pounds
Carbon Monoxide: 77.504 pounds
Hydrocarbons:   15.615 pounds
Nitrogen Oxides: 2.277 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 286.550 pounds
                  46.421 gallons
Carbon Dioxide:  894.035 pounds
Carbon Monoxide: 76.593 pounds
Hydrocarbons:   15.433 pounds
Nitrogen Oxides: 2.087 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.16 0.00  0.16  0.15 0.62  0.00  0.00 0.48  0.48
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  3.9 0.0  1.4  3.6 2.1  0.0  0.0 11.4  11.4
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  1.00 1.00  0.00  0.00 1.00  1.00
Q2:           0.0 0.0  0.0  4.6 0.0  0.6  4.4 0.4  0.0  0.0 6.8  6.8
HCM2kQueue:   0.0 0.0  0.0  8.5 0.0  2.0  8.0 2.5  0.0  0.0 18.2  18.2
-----
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.20  1.18 1.19  1.20  1.20 1.16  1.16
HCM2k70thQ:   0.0 0.0  0.0  10.0 0.0  2.4  9.5 3.0  0.0  0.0 21.1  21.1
-----
85thFactor:   1.60 1.60  1.60  1.53 1.60  1.58  1.53 1.58  1.60  1.60 1.46  1.46
HCM2k85thQ:   0.0 0.0  0.0  12.9 0.0  3.2  12.3 4.0  0.0  0.0 26.6  26.6
-----
90thFactor:   1.80 1.80  1.80  1.66 1.80  1.76  1.67 1.75  1.80  1.80 1.56  1.56
HCM2k90thQ:   0.0 0.0  0.0  14.1 0.0  3.5  13.4 4.4  0.0  0.0 28.4  28.4
-----
95thFactor:   2.10 2.10  2.10  1.87 2.10  2.04  1.88 2.02  2.10  2.10 1.72  1.72
HCM2k95thQ:   0.0 0.0  0.0  15.9 0.0  4.1  15.1 5.1  0.0  0.0 31.2  31.2
-----
98thFactor:   2.70 2.70  2.70  2.22 2.70  2.56  2.24 2.52  2.70  2.70 1.95  1.95
HCM2k98thQ:   0.0 0.0  0.0  18.8 0.0  5.1  18.0 6.3  0.0  0.0 35.4  35.4
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #16 Cosumnes River Boulevard/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  64.1 0.0  22.5  59.3 66.7  0.0  0.0 274  79.7
-----
Name: year 1995 composite fleet
Fuel Consumption: 82.048 pounds
                  13.292 gallons
Carbon Dioxide:  255.989 pounds
Carbon Monoxide: 20.131 pounds
Hydrocarbons:    3.669 pounds
Nitrogen Oxides: 0.742 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 77.061 pounds
                  12.484 gallons
Carbon Dioxide:  240.431 pounds
Carbon Monoxide: 19.739 pounds
Hydrocarbons:    3.590 pounds
Nitrogen Oxides: 0.663 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.23 0.00  0.23  0.13 0.55  0.00  0.00 0.42  0.42
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  4.4 0.0  3.3  2.7 5.5  0.0  0.0 7.6  7.6
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  1.00 1.00  0.00  0.00 1.00  1.00
Q2:           0.0 0.0  0.0  2.7 0.0  1.8  2.4 1.2  0.0  0.0 2.9  2.9
HCM2KQueue:   0.0 0.0  0.0  7.1 0.0  5.1  5.1 6.7  0.0  0.0 10.5  10.5
-----
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.19  1.19 1.18  1.20  1.20 1.18  1.18
HCM2k70thQ:   0.0 0.0  0.0  8.4 0.0  6.1  6.0 7.9  0.0  0.0 12.3  12.3
-----
85thFactor:   1.60 1.60  1.60  1.54 1.60  1.55  1.55 1.54  1.60  1.60 1.51  1.51
HCM2k85thQ:   0.0 0.0  0.0  10.9 0.0  7.9  7.9 10.3  0.0  0.0 15.8  15.8
-----
90thFactor:   1.80 1.80  1.80  1.68 1.80  1.71  1.71 1.69  1.80  1.80 1.64  1.64
HCM2k90thQ:   0.0 0.0  0.0  12.0 0.0  8.7  8.7 11.3  0.0  0.0 17.1  17.1
-----
95thFactor:   2.10 2.10  2.10  1.90 2.10  1.95  1.95 1.91  2.10  2.10 1.84  1.84
HCM2k95thQ:   0.0 0.0  0.0  13.5 0.0  10.0  9.9 12.8  0.0  0.0 19.2  19.2
-----
98thFactor:   2.70 2.70  2.70  2.28 2.70  2.38  2.38 2.30  2.70  2.70 2.15  2.15
HCM2k98thQ:   0.0 0.0  0.0  16.2 0.0  12.1  12.1 15.4  0.0  0.0 22.5  22.5
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #16 Cosumnes River Boulevard/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   0.0 0.0  0.0  72.5 0.0  54.6  43.5 171  0.0  0.0 177  57.7
-----
Name: year 1995 composite fleet
Fuel Consumption: 69.530 pounds
                  11.264 gallons
Carbon Dioxide:  216.934 pounds
Carbon Monoxide: 16.350 pounds
Hydrocarbons:    2.740 pounds
Nitrogen Oxides: 0.696 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 64.233 pounds
                  10.406 gallons
Carbon Dioxide:  200.408 pounds
Carbon Monoxide: 15.934 pounds
Hydrocarbons:    2.656 pounds
Nitrogen Oxides: 0.612 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum AM

Intersection #32:

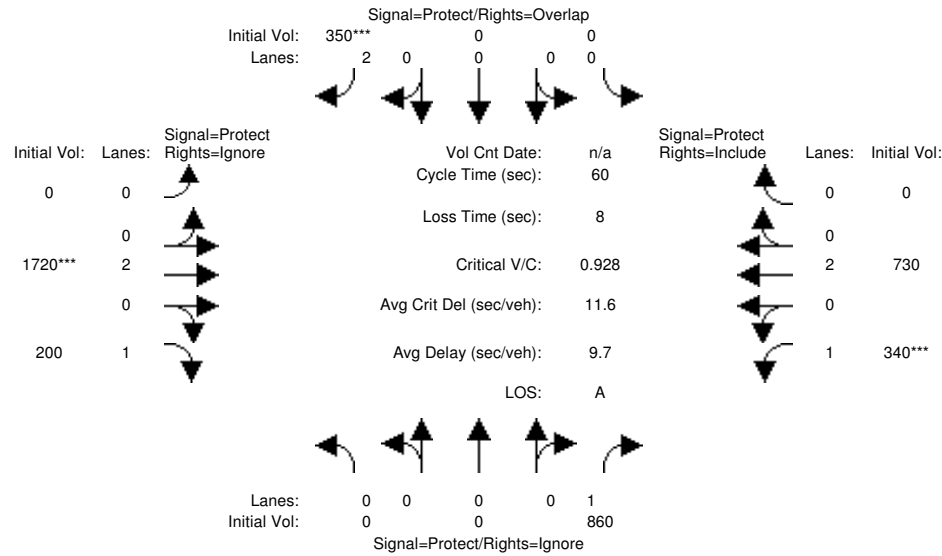


Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns (L, T, R). Rows include Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and HCM Ops f(lt) Adj Case Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns (L, T, R). Rows include HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns (L, T, R). Rows include HCM Ops Saturation Adj Module, Delay Adjustment Factor Module, and HCM Ops Saturation Adj Module.

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns (L, T, R). Rows include HCM Ops Saturation Adj Module, Delay Adjustment Factor Module, and HCM Ops Saturation Adj Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #32

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.00 0.00  0.00  0.00 0.00  0.17  0.00 0.75  0.00  0.24 0.70  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  0.0 0.0  3.1  0.0 7.0  0.0  5.3 2.4  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  0.00 0.00  1.00 0.00  1.00  0.00 1.00  1.00  0.00
Q2:           0.0 0.0  0.0  0.0 0.0  2.3  0.0 1.7  0.0  3.1 0.4  0.0
HCM2kQueue:   0.0 0.0  0.0  0.0 0.0  5.4  0.0 8.8  0.0  8.4 2.8  0.0
-----|-----|-----|-----|
70thFactor:   1.20 1.20  1.20  1.20 1.20  1.19  1.20 1.18  1.20  1.18 1.19  1.20
HCM2k70thQ:   0.0 0.0  0.0  0.0 0.0  6.4  0.0 10.4  0.0  10.0 3.4  0.0
-----|-----|-----|-----|
85thFactor:   1.60 1.60  1.60  1.60 1.60  1.55  1.60 1.52  1.60  1.53 1.57  1.60
HCM2k85thQ:   0.0 0.0  0.0  0.0 0.0  8.4  0.0 13.4  0.0  12.9 4.5  0.0
-----|-----|-----|-----|
90thFactor:   1.80 1.80  1.80  1.80 1.80  1.71  1.80 1.66  1.80  1.66 1.75  1.80
HCM2k90thQ:   0.0 0.0  0.0  0.0 0.0  9.2  0.0 14.5  0.0  14.0 5.0  0.0
-----|-----|-----|-----|
95thFactor:   2.10 2.10  2.10  2.10 2.10  1.94  2.10 1.87  2.10  1.88 2.01  2.10
HCM2k95thQ:   0.0 0.0  0.0  0.0 0.0  10.5  0.0 16.4  0.0  15.8 5.7  0.0
-----|-----|-----|-----|
98thFactor:   2.70 2.70  2.70  2.70 2.70  2.36  2.70 2.21  2.70  2.22 2.50  2.70
HCM2k98thQ:   0.0 0.0  0.0  0.0 0.0  12.7  0.0 19.4  0.0  18.8 7.1  0.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #32
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  0.0 0.0  83.1  0.0 201  0.0  80.0 69.0  0.0
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 58.152 pounds
                  9.421 gallons
Carbon Dioxide:  181.435 pounds
Carbon Monoxide: 12.947 pounds
Hydrocarbons:    1.967 pounds
Nitrogen Oxides: 0.596 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 52.777 pounds
                  8.550 gallons
Carbon Dioxide:  164.664 pounds
Carbon Monoxide: 12.533 pounds
Hydrocarbons:    1.882 pounds
Nitrogen Oxides: 0.520 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.00 0.00  0.17  0.00 0.75  0.00  0.24 0.70  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  0.0 0.0  4.7  0.0 9.4  0.0  8.3 5.3  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  0.00 0.00  1.00 0.00  1.00  0.00 1.00  1.00  0.00
Q2:           0.0 0.0  0.0  0.0 0.0  7.0  0.0 2.6  0.0 13.9 1.1  0.0
HCM2kQueue:   0.0 0.0  0.0  0.0 0.0 11.7  0.0 12.0  0.0 22.3 6.4  0.0
-----
70thFactor:   1.20 1.20  1.20  1.20 1.20  1.17  1.20 1.17  1.20  1.16 1.19  1.20
HCM2k70thQ:   0.0 0.0  0.0  0.0 0.0 13.8  0.0 14.0  0.0 25.8 7.6  0.0
-----
85thFactor:   1.60 1.60  1.60  1.60 1.60  1.50  1.60 1.50  1.60  1.44 1.54  1.60
HCM2k85thQ:   0.0 0.0  0.0  0.0 0.0 17.6  0.0 18.0  0.0 32.1 9.9  0.0
-----
90thFactor:   1.80 1.80  1.80  1.80 1.80  1.62  1.80 1.62  1.80  1.53 1.69  1.80
HCM2k90thQ:   0.0 0.0  0.0  0.0 0.0 19.0  0.0 19.4  0.0 34.1 10.8  0.0
-----
95thFactor:   2.10 2.10  2.10  2.10 2.10  1.81  2.10 1.81  2.10  1.67 1.92  2.10
HCM2k95thQ:   0.0 0.0  0.0  0.0 0.0 21.2  0.0 21.6  0.0 37.3 12.3  0.0
-----
98thFactor:   2.70 2.70  2.70  2.70 2.70  2.11  2.70 2.10  2.70  1.88 2.31  2.70
HCM2k98thQ:   0.0 0.0  0.0  0.0 0.0 24.7  0.0 25.1  0.0 41.9 14.8  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #32
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  0.0 0.0 126.4  0.0 267  0.0 132.4  152  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 133.370 pounds
                  21.606 gallons
Carbon Dioxide:  416.113 pounds
Carbon Monoxide: 33.012 pounds
Hydrocarbons:    6.189 pounds
Nitrogen Oxides: 1.106 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 125.907 pounds
                  20.397 gallons
Carbon Dioxide:  392.830 pounds
Carbon Monoxide: 32.434 pounds
Hydrocarbons:    6.070 pounds
Nitrogen Oxides: 0.996 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.52 0.00  0.52  0.00 0.40  0.00  0.00 0.40  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  9.6 0.0  30.8  0.0 23.0  0.0  0.0 28.0  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  1.2 0.0  39.5  0.0 16.5  0.0  0.0 36.2  0.0
HCM2kQueue:   0.0 0.0  0.0  10.8 0.0  70.3  0.0 39.5  0.0  0.0 64.1  0.0
-----
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.12  1.20 1.14  1.20  1.20 1.12  1.20
HCM2k70thQ:   0.0 0.0  0.0  12.7 0.0  78.6  0.0 44.9  0.0  0.0 71.8  0.0
-----
85thFactor:   1.60 1.60  1.60  1.51 1.60  1.33  1.60 1.38  1.60  1.60 1.34  1.60
HCM2k85thQ:   0.0 0.0  0.0  16.3 0.0  93.5  0.0 54.5  0.0  0.0 85.6  0.0
-----
90thFactor:   1.80 1.80  1.80  1.63 1.80  1.41  1.80 1.46  1.80  1.80 1.42  1.80
HCM2k90thQ:   0.0 0.0  0.0  17.6 0.0  99.3  0.0 57.5  0.0  0.0 90.8  0.0
-----
95thFactor:   2.10 2.10  2.10  1.83 2.10  1.51  2.10 1.57  2.10  2.10 1.52  2.10
HCM2k95thQ:   0.0 0.0  0.0  19.7 0.0  106.4  0.0 61.9  0.0  0.0 97.3  0.0
-----
98thFactor:   2.70 2.70  2.70  2.14 2.70  1.70  2.70 1.75  2.70  2.70 1.71  2.70
HCM2k98thQ:   0.0 0.0  0.0  23.1 0.0  119.9  0.0 69.0  0.0  0.0 109  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #33
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  0.0 0.0  0.0 167.2  0.0 446.6  0.0 610  0.0  0.0 898  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 751.684 pounds
                  121.773 gallons
Carbon Dioxide:  2345.254 pounds
Carbon Monoxide: 205.065 pounds
Hydrocarbons:   44.442 pounds
Nitrogen Oxides: 4.774 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 737.372 pounds
                  119.454 gallons
Carbon Dioxide:  2300.600 pounds
Carbon Monoxide: 203.914 pounds
Hydrocarbons:   44.216 pounds
Nitrogen Oxides: 4.519 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.54 0.00  0.54  0.00 0.38  0.00  0.00 0.38  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  20.4 0.0  33.6  0.0 23.3  0.0  0.0 28.4  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  5.4 0.0  48.4  0.0 20.5  0.0  0.0 41.4  0.0
HCM2kQueue:   0.0 0.0  0.0  25.8 0.0  82.0  0.0 43.8  0.0  0.0 69.7  0.0
-----
70thFactor:   1.20 1.20  1.20  1.15 1.20  1.11  1.20 1.13  1.20  1.20 1.12  1.20
HCM2k70thQ:   0.0 0.0  0.0  29.7 0.0  91.3  0.0 49.6  0.0  0.0 77.9  0.0
-----
85thFactor:   1.60 1.60  1.60  1.43 1.60  1.32  1.60 1.37  1.60  1.60 1.33  1.60
HCM2k85thQ:   0.0 0.0  0.0  36.8 0.0 108.2  0.0 60.0  0.0  0.0 92.7  0.0
-----
90thFactor:   1.80 1.80  1.80  1.51 1.80  1.41  1.80 1.44  1.80  1.80 1.41  1.80
HCM2k90thQ:   0.0 0.0  0.0  38.9 0.0 115.4  0.0 63.3  0.0  0.0 98.5  0.0
-----
95thFactor:   2.10 2.10  2.10  1.64 2.10  1.51  2.10 1.55  2.10  2.10 1.51  2.10
HCM2k95thQ:   0.0 0.0  0.0  42.4 0.0 123.6  0.0 68.0  0.0  0.0 105  0.0
-----
98thFactor:   2.70 2.70  2.70  1.84 2.70  1.70  2.70 1.73  2.70  2.70 1.70  2.70
HCM2k98thQ:   0.0 0.0  0.0  47.4 0.0 139.6  0.0 75.9  0.0  0.0 119  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #33
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  0.0 0.0  0.0 356.3  0.0 596.7  0.0 641  0.0  0.0 952  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  939.905 pounds
                  152.265 gallons
Carbon Dioxide:   2932.503 pounds
Carbon Monoxide:  257.520 pounds
Hydrocarbons:    56.130 pounds
Nitrogen Oxides:  5.882 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  923.633 pounds
                  149.629 gallons
Carbon Dioxide:   2881.736 pounds
Carbon Monoxide:  256.205 pounds
Hydrocarbons:    55.874 pounds
Nitrogen Oxides:  5.585 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum AM

Intersection #34:

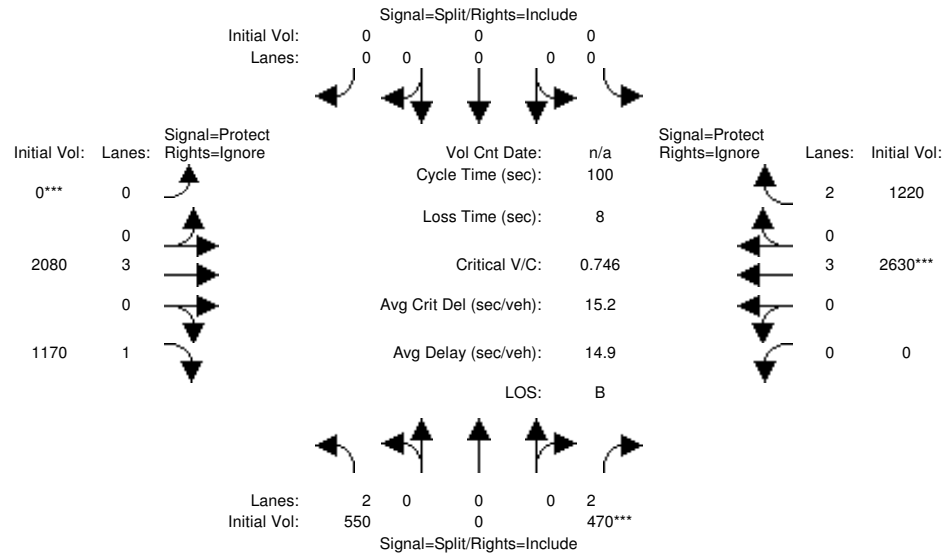


Table containing traffic engineering data: Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.23 0.00  0.23  0.00 0.00  0.00 0.69  0.00  0.00 0.69  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           7.3 0.0  6.9  0.0 0.0  0.0  0.0 11.0  0.0  0.0 17.0  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 0.00  0.00  0.00 1.00  0.00
Q2:           2.1 0.0  2.4  0.0 0.0  0.0  0.0 1.4  0.0  0.0 2.7  0.0
HCM2KQueue:   9.4 0.0  9.3  0.0 0.0  0.0  0.0 12.4  0.0  0.0 19.7  0.0
-----
70thFactor:  1.18 1.20  1.18  1.20 1.20  1.20 1.17  1.20  1.20 1.16  1.20
HCM2k70thQ:  11.1 0.0  11.0  0.0 0.0  0.0  0.0 14.5  0.0  0.0 22.9  0.0
-----
85thFactor:  1.52 1.60  1.52  1.60 1.60  1.60 1.50  1.60  1.60 1.46  1.60
HCM2k85thQ:  14.2 0.0  14.2  0.0 0.0  0.0  0.0 18.5  0.0  0.0 28.7  0.0
-----
90thFactor:  1.65 1.80  1.65  1.80 1.80  1.80 1.62  1.80  1.80 1.55  1.80
HCM2k90thQ:  15.5 0.0  15.4  0.0 0.0  0.0  0.0 20.0  0.0  0.0 30.6  0.0
-----
95thFactor:  1.86 2.10  1.86  2.10 2.10  2.10 1.80  2.10  2.10 1.70  2.10
HCM2k95thQ:  17.4 0.0  17.3  0.0 0.0  0.0  0.0 22.3  0.0  0.0 33.6  0.0
-----
98thFactor:  2.19 2.70  2.19  2.70 2.70  2.70 2.09  2.70  2.70 1.92  2.70
HCM2k98thQ:  20.5 0.0  20.4  0.0 0.0  0.0  0.0 25.8  0.0  0.0 37.9  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #34
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  126.7 0.0 109.4  0.0 0.0  0.0  0.0 270  0.0  0.0 417  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 128.383 pounds
                  20.798 gallons
Carbon Dioxide:  400.555 pounds
Carbon Monoxide: 29.879 pounds
Hydrocarbons:    4.966 pounds
Nitrogen Oxides: 1.247 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 118.324 pounds
                  19.169 gallons
Carbon Dioxide:  369.172 pounds
Carbon Monoxide: 29.099 pounds
Hydrocarbons:    4.805 pounds
Nitrogen Oxides: 1.098 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.22 0.00 0.22 | 0.00 0.00 0.00 | 0.00 0.70 0.00 | 0.00 0.70 0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00 1.00 | 1.00 1.00 1.00 | 1.00 1.00 1.00 | 1.00 1.00 1.00
Q1:           6.4 0.0 8.3 | 0.0 0.0 0.0 | 0.0 25.6 0.0 | 0.0 26.2 0.0
UpstreamVC:   0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00
UpstreamAdj:  0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00
EarlyArrAdj:  1.00 0.00 1.00 | 0.00 0.00 0.00 | 0.00 1.00 0.00 | 0.00 1.00 0.00
Q2:           1.8 0.0 4.4 | 0.0 0.0 0.0 | 0.0 6.0 0.0 | 0.0 6.3 0.0
HCM2KQueue:   8.2 0.0 12.7 | 0.0 0.0 0.0 | 0.0 31.6 0.0 | 0.0 32.6 0.0
-----|-----|-----|-----|
70thFactor:   1.18 1.20 1.17 | 1.20 1.20 1.20 | 1.20 1.15 1.20 | 1.20 1.14 1.20
HCM2k70thQ:   9.7 0.0 14.8 | 0.0 0.0 0.0 | 0.0 36.2 0.0 | 0.0 37.3 0.0
-----|-----|-----|-----|
85thFactor:   1.53 1.60 1.50 | 1.60 1.60 1.60 | 1.60 1.40 1.60 | 1.60 1.40 1.60
HCM2k85thQ:   12.5 0.0 18.9 | 0.0 0.0 0.0 | 0.0 44.4 0.0 | 0.0 45.7 0.0
-----|-----|-----|-----|
90thFactor:   1.67 1.80 1.61 | 1.80 1.80 1.80 | 1.80 1.48 1.80 | 1.80 1.48 1.80
HCM2k90thQ:   13.6 0.0 20.4 | 0.0 0.0 0.0 | 0.0 46.8 0.0 | 0.0 48.2 0.0
-----|-----|-----|-----|
95thFactor:   1.88 2.10 1.80 | 2.10 2.10 2.10 | 2.10 1.60 2.10 | 2.10 1.60 2.10
HCM2k95thQ:   15.4 0.0 22.7 | 0.0 0.0 0.0 | 0.0 50.7 0.0 | 0.0 52.1 0.0
-----|-----|-----|-----|
98thFactor:   2.23 2.70 2.08 | 2.70 2.70 2.70 | 2.70 1.79 2.70 | 2.70 1.78 2.70
HCM2k98thQ:   18.3 0.0 26.3 | 0.0 0.0 0.0 | 0.0 56.5 0.0 | 0.0 58.0 0.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #34
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  112.0 0.0 131.3 | 0.0 0.0 0.0 | 0.0 629 0.0 | 0.0 645 0.0
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 193.460 pounds
                  31.341 gallons
Carbon Dioxide:  603.596 pounds
Carbon Monoxide: 46.235 pounds
Hydrocarbons:    8.006 pounds
Nitrogen Oxides: 1.870 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 179.832 pounds
                  29.133 gallons
Carbon Dioxide:  561.075 pounds
Carbon Monoxide: 45.164 pounds
Hydrocarbons:    7.790 pounds
Nitrogen Oxides: 1.655 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #1: Pocket Road/I-5 SB Ramps

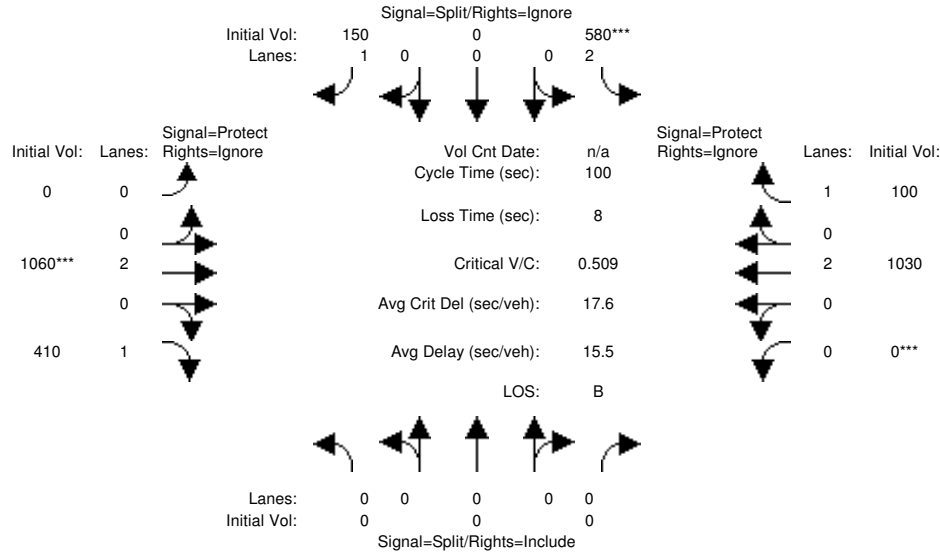


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, and Delay Adjustment Factor Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Green/Cycle:	0.00	0.00	0.00	0.33	0.00	0.00	0.00	0.59	0.00	0.00	0.59	0.00
ArrivalType:	3			3			3			3		
ProgFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	6.7	0.0	0.0	0.0	9.1	0.0	0.0	8.7	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00
Q2:	0.0	0.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
HCM2kQueue:	0.0	0.0	0.0	7.7	0.0	0.0	0.0	10.1	0.0	0.0	9.7	0.0
70thFactor:	1.20	1.20	1.20	1.18	1.20	1.20	1.20	1.18	1.20	1.20	1.18	1.20
HCM2k70thQ:	0.0	0.0	0.0	9.1	0.0	0.0	0.0	11.9	0.0	0.0	11.4	0.0
85thFactor:	1.60	1.60	1.60	1.53	1.60	1.60	1.60	1.51	1.60	1.60	1.52	1.60
HCM2k85thQ:	0.0	0.0	0.0	11.8	0.0	0.0	0.0	15.3	0.0	0.0	14.7	0.0
90thFactor:	1.80	1.80	1.80	1.67	1.80	1.80	1.80	1.64	1.80	1.80	1.65	1.80
HCM2k90thQ:	0.0	0.0	0.0	12.9	0.0	0.0	0.0	16.6	0.0	0.0	16.0	0.0
95thFactor:	2.10	2.10	2.10	1.89	2.10	2.10	2.10	1.84	2.10	2.10	1.85	2.10
HCM2k95thQ:	0.0	0.0	0.0	14.5	0.0	0.0	0.0	18.7	0.0	0.0	18.0	0.0
98thFactor:	2.70	2.70	2.70	2.25	2.70	2.70	2.70	2.16	2.70	2.70	2.17	2.70
HCM2k98thQ:	0.0	0.0	0.0	17.3	0.0	0.0	0.0	21.9	0.0	0.0	21.1	0.0

Fuel Consumption and Emissions
 2000 HCM Operations Method
 Base Volume Alternative

 Intersection #1 Pocket Road/I-5 SB Ramps

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Run Speed:	30 MPH			30 MPH			30 MPH			30 MPH		
NumOfStops:	0.0	0.0	0.0	116.6	0.0	0.0	0.0	156	0.0	0.0	150	0.0

Name: year 1995 composite fleet
 Fuel Consumption: 60.786 pounds
 9.847 gallons
 Carbon Dioxide: 189.652 pounds
 Carbon Monoxide: 14.202 pounds
 Hydrocarbons: 2.382 pounds
 Nitrogen Oxides: 0.582 pounds

Name: year 2000 composite fleet
 Fuel Consumption: 56.114 pounds
 9.090 gallons
 Carbon Dioxide: 175.076 pounds
 Carbon Monoxide: 13.840 pounds
 Hydrocarbons: 2.308 pounds
 Nitrogen Oxides: 0.514 pounds

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj PM

Intersection #1: Pocket Road/I-5 SB Ramps

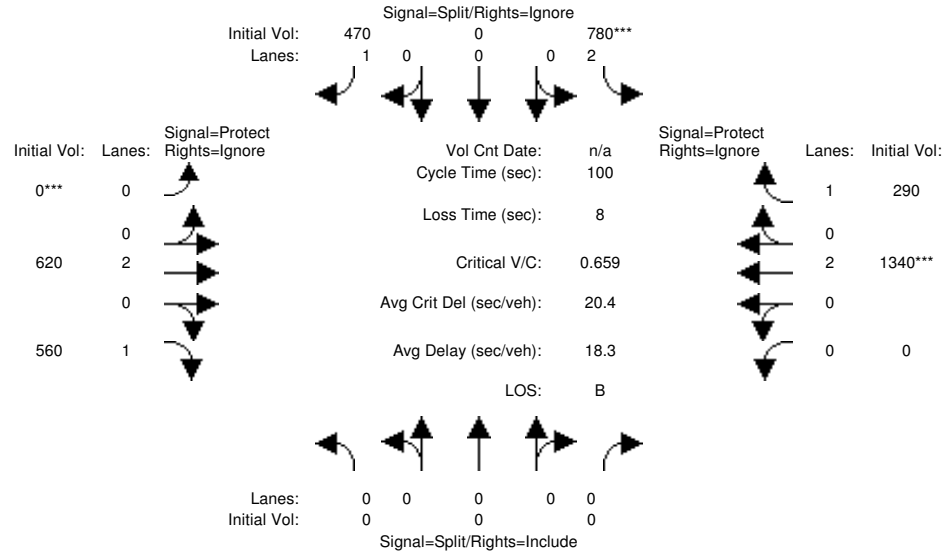


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module, and Level Of Service Detailed Computation Report.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.00 0.00  0.00  0.35 0.00  0.00  0.00 0.57  0.00  0.00 0.57  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  9.5 0.0  0.0  0.0 4.7  0.0  0.0 13.4  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  0.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  1.8 0.0  0.0  0.0 0.4  0.0  0.0 1.9  0.0
HCM2kQueue:   0.0 0.0  0.0  11.3 0.0  0.0  0.0 5.1  0.0  0.0 15.3  0.0
-----|-----|-----|-----|
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.20  1.20 1.19  1.20  1.20 1.17  1.20
HCM2k70thQ:   0.0 0.0  0.0  13.2 0.0  0.0  0.0 6.1  0.0  0.0 17.8  0.0
-----|-----|-----|-----|
85thFactor:   1.60 1.60  1.60  1.51 1.60  1.60  1.60 1.55  1.60  1.60 1.48  1.60
HCM2k85thQ:   0.0 0.0  0.0  17.0 0.0  0.0  0.0 7.9  0.0  0.0 22.6  0.0
-----|-----|-----|-----|
90thFactor:   1.80 1.80  1.80  1.63 1.80  1.80  1.80 1.71  1.80  1.80 1.59  1.80
HCM2k90thQ:   0.0 0.0  0.0  18.3 0.0  0.0  0.0 8.7  0.0  0.0 24.2  0.0
-----|-----|-----|-----|
95thFactor:   2.10 2.10  2.10  1.82 2.10  2.10  2.10 1.95  2.10  2.10 1.76  2.10
HCM2k95thQ:   0.0 0.0  0.0  20.5 0.0  0.0  0.0 10.0  0.0  0.0 26.8  0.0
-----|-----|-----|-----|
98thFactor:   2.70 2.70  2.70  2.12 2.70  2.70  2.70 2.38  2.70  2.70 2.01  2.70
HCM2k98thQ:   0.0 0.0  0.0  23.9 0.0  0.0  0.0 12.1  0.0  0.0 30.7  0.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
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*****
Intersection #1 Pocket Road/I-5 SB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0 165.3  0.0 0.0  0.0 79.9  0.0  0.0 229  0.0
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 68.190 pounds
                  11.047 gallons
Carbon Dioxide:  212.754 pounds
Carbon Monoxide: 16.202 pounds
Hydrocarbons:    2.799 pounds
Nitrogen Oxides: 0.642 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 63.318 pounds
                  10.258 gallons
Carbon Dioxide:  197.553 pounds
Carbon Monoxide: 15.823 pounds
Hydrocarbons:    2.722 pounds
Nitrogen Oxides: 0.568 pounds
    
```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.53 0.00  0.53  0.00 0.00  0.00  0.00 0.39  0.00  0.00 0.39  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:          19.9 0.0  3.8  0.0 0.0  0.0  0.0 16.8  0.0  0.0  4.8  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 0.00  1.00  0.00  1.00  0.00
Q2:           5.5 0.0  0.4  0.0 0.0  0.0  0.0  5.2  0.0  0.0  0.5  0.0
HCM2KQueue:   25.4 0.0  4.1  0.0 0.0  0.0  0.0 22.0  0.0  0.0  5.3  0.0
-----
70thFactor:   1.15 1.20  1.19  1.20 1.20  1.20  1.20 1.16  1.20  1.20 1.19  1.20
HCM2k70thQ:   29.3 0.0  4.9  0.0 0.0  0.0  0.0 25.5  0.0  0.0  6.3  0.0
-----
85thFactor:   1.43 1.60  1.56  1.60 1.60  1.60  1.60 1.44  1.60  1.60 1.55  1.60
HCM2k85thQ:   36.3 0.0  6.4  0.0 0.0  0.0  0.0 31.8  0.0  0.0  8.3  0.0
-----
90thFactor:   1.51 1.80  1.73  1.80 1.80  1.80  1.80 1.53  1.80  1.80 1.71  1.80
HCM2k90thQ:   38.4 0.0  7.1  0.0 0.0  0.0  0.0 33.7  0.0  0.0  9.1  0.0
-----
95thFactor:   1.65 2.10  1.98  2.10 2.10  2.10  2.10 1.68  2.10  2.10 1.95  2.10
HCM2k95thQ:   41.8 0.0  8.2  0.0 0.0  0.0  0.0 36.9  0.0  0.0 10.4  0.0
-----
98thFactor:   1.84 2.70  2.43  2.70 2.70  2.70  2.70 1.88  2.70  2.70 2.36  2.70
HCM2k98thQ:   46.8 0.0 10.0  0.0 0.0  0.0  0.0 41.5  0.0  0.0 12.6  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #2 Meadowview Road/I-5 NB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  146.6 0.0  39.9  0.0 0.0  0.0  0.0 287  0.0  0.0 82.4  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  87.327 pounds
                  14.147 gallons
Carbon Dioxide:   272.460 pounds
Carbon Monoxide:  21.694 pounds
Hydrocarbons:     4.048 pounds
Nitrogen Oxides:  0.761 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  82.433 pounds
                  13.354 gallons
Carbon Dioxide:   257.192 pounds
Carbon Monoxide:  21.308 pounds
Hydrocarbons:     3.971 pounds
Nitrogen Oxides:  0.683 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj PM

Intersection #2: Meadowview Road/I-5 NB Ramps

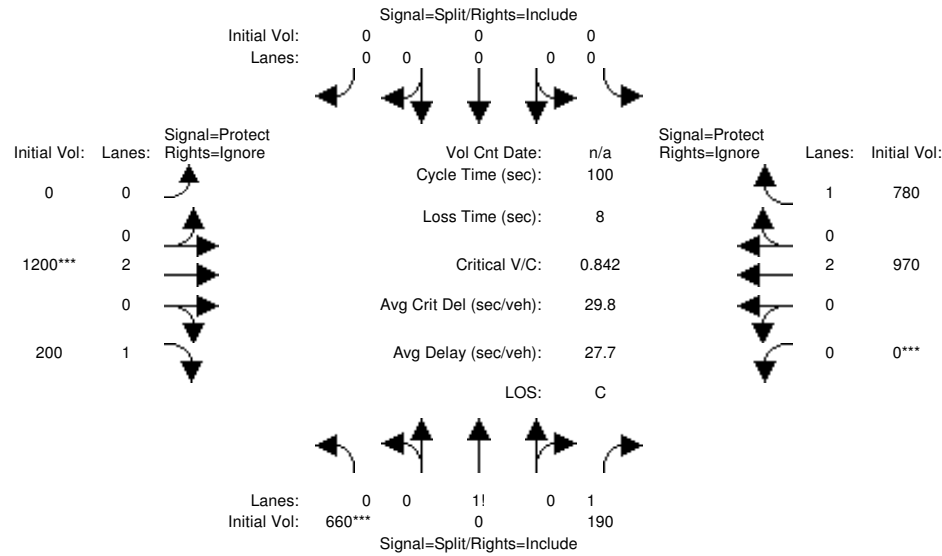


Table with columns: Approach, Movement, Min. Green, Volume Module (Base Vol, Growth Adj, etc.), Sat. Flow Module (Sat/Lane, Adjustment, etc.), Capacity Analysis Module (Vol/Sat, Crit Moves, etc.).

Table with columns: HCM Ops Adjusted Lane Utilization Module (Lanes, Lane Group, #LnsInGrps), HCM Ops Input Saturation Adj Module (Lane Width, CrsswalkWid, etc.), HCM Ops f(lt) Adj Case Module (f(lt) Case).

Table with columns: HCM Ops Saturation Adj Module (Ln Wid Adj, Hev Veh Adj, etc.), Delay Adjustment Factor Module (Coordinated, Signal Type, etc.).

Table with columns: HCM Ops Saturation Adj Module (Ln Wid Adj, Hev Veh Adj, etc.), Delay Adjustment Factor Module (Coordinated, Signal Type, etc.).

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #2 Meadowview Road/I-5 NB Ramps

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.52 0.00  0.52  0.00 0.00  0.00  0.00 0.40  0.00  0.00 0.40  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00 1.00  1.00
Q1:          17.9 0.0  2.5  0.0 0.0  0.0  0.0 15.9  0.0  0.0 11.7  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 0.00  1.00  0.00  0.00 1.00  0.00
Q2:           4.3 0.0  0.2  0.0 0.0  0.0  0.0  4.2  0.0  0.0  2.0  0.0
HCM2KQueue:   22.2 0.0  2.7  0.0 0.0  0.0  0.0 20.0  0.0  0.0 13.7  0.0
-----
70thFactor:   1.16 1.20  1.19  1.20 1.20  1.20  1.20 1.16  1.20  1.20 1.17  1.20
HCM2k70thQ:   25.7 0.0  3.3  0.0 0.0  0.0  0.0 23.2  0.0  0.0 16.0  0.0
-----
85thFactor:   1.44 1.60  1.57  1.60 1.60  1.60  1.60 1.45  1.60  1.60 1.49  1.60
HCM2k85thQ:   32.1 0.0  4.3  0.0 0.0  0.0  0.0 29.1  0.0  0.0 20.4  0.0
-----
90thFactor:   1.53 1.80  1.75  1.80 1.80  1.80  1.80 1.55  1.80  1.80 1.60  1.80
HCM2k90thQ:   34.1 0.0  4.8  0.0 0.0  0.0  0.0 31.0  0.0  0.0 21.9  0.0
-----
95thFactor:   1.67 2.10  2.02  2.10 2.10  2.10  2.10 1.70  2.10  2.10 1.78  2.10
HCM2k95thQ:   37.2 0.0  5.5  0.0 0.0  0.0  0.0 34.0  0.0  0.0 24.3  0.0
-----
98thFactor:   1.88 2.70  2.51  2.70 2.70  2.70  2.70 1.91  2.70  2.70 2.05  2.70
HCM2k98thQ:   41.8 0.0  6.9  0.0 0.0  0.0  0.0 38.3  0.0  0.0 28.0  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #2 Meadowview Road/I-5 NB Ramps
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  141.2 0.0  25.4  0.0 0.0  0.0  0.0 271  0.0  0.0 200  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 96.162 pounds
                  15.578 gallons
Carbon Dioxide:  300.024 pounds
Carbon Monoxide: 23.764 pounds
Hydrocarbons:    4.390 pounds
Nitrogen Oxides: 0.853 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 90.577 pounds
                  14.673 gallons
Carbon Dioxide:  282.601 pounds
Carbon Monoxide: 23.324 pounds
Hydrocarbons:    4.302 pounds
Nitrogen Oxides: 0.763 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj AM

Intersection #3: Meadowview Road/Freeport Boulevard

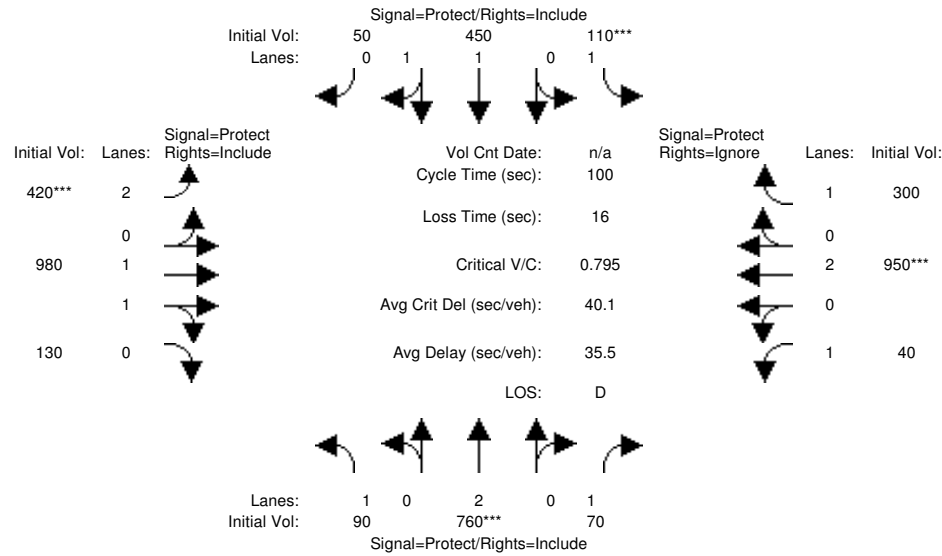


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.09 0.27  0.27  0.08 0.26  0.26  0.15 0.43  0.43  0.07 0.34  0.00
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           2.4 10.3  1.5   3.0 6.3  6.3   5.8 13.7  13.7  1.1 12.6  0.0
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  0.00
Q2:           1.1  3.1  0.2   2.3 1.2  1.2   2.8 2.7  2.7   0.5 3.2  0.0
HCM2kQueue:    3.5 13.5  1.7   5.3 7.5  7.5   8.6 16.4  16.4  1.6 15.8  0.0
-----
70thFactor:    1.19 1.17  1.20  1.19 1.18  1.18  1.18 1.17  1.17  1.20 1.17  1.20
HCM2k70thQ:    4.2 15.8  2.0   6.3 8.9  8.9  10.1 19.2  19.2  1.9 18.5  0.0
-----
85thFactor:    1.57 1.49  1.58  1.55 1.53  1.53  1.53 1.47  1.47  1.58 1.48  1.60
HCM2k85thQ:    5.5 20.1  2.7   8.3 11.6  11.6  13.1 24.2  24.2  2.5 23.4  0.0
-----
90thFactor:    1.74 1.60  1.77  1.71 1.67  1.67  1.66 1.58  1.58  1.77 1.58  1.80
HCM2k90thQ:    6.1 21.6  3.0   9.1 12.6  12.6  14.2 25.9  25.9  2.7 25.0  0.0
-----
95thFactor:    1.99 1.78  2.05  1.95 1.89  1.89  1.87 1.74  1.74  2.05 1.75  2.10
HCM2k95thQ:    7.0 24.0  3.4  10.4 14.3  14.3  16.1 28.6  28.6  3.2 27.7  0.0
-----
98thFactor:    2.46 2.06  2.58  2.36 2.26  2.26  2.22 1.98  1.98  2.59 2.00  2.70
HCM2k98thQ:    8.7 27.6  4.3  12.6 17.1  17.1  19.0 32.6  32.6  4.0 31.6  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #3 Meadowview Road/Freeport Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    21.5 177  13.4  27.0 97.6  10.8 101.2  207 27.5  9.6 215  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 151.455 pounds
                  24.536 gallons
Carbon Dioxide:  472.539 pounds
Carbon Monoxide: 38.239 pounds
Hydrocarbons:    7.331 pounds
Nitrogen Oxides: 1.272 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 143.867 pounds
                  23.306 gallons
Carbon Dioxide:  448.865 pounds
Carbon Monoxide: 37.640 pounds
Hydrocarbons:    7.211 pounds
Nitrogen Oxides: 1.149 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj PM

Intersection #3: Meadowview Road/Freeport Boulevard

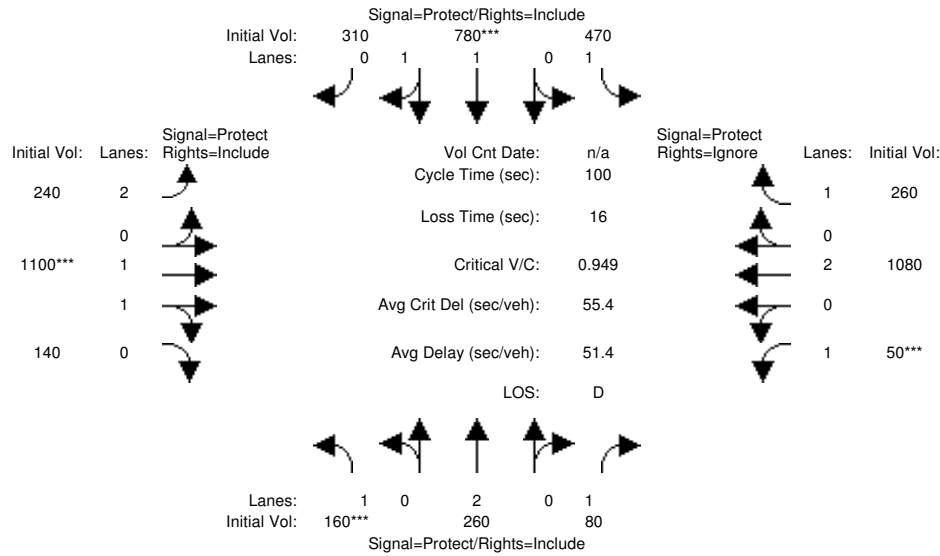


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.09 0.09 0.09  0.33 0.33 0.33  0.08 0.37 0.37  0.05 0.34 0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00
Q1:           4.4 3.7 2.1  11.9 15.7 15.7  3.4 17.9 17.9  1.4 15.0 0.0
UpstreamVC:   0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00
UpstreamAdj:  0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00  0.00 0.00 0.00
EarlyArrAdj:  1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 1.00  1.00 1.00 0.00
Q2:           4.2 2.5 1.1  3.3 7.5 7.5   3.2 8.0 8.0   1.1 5.4 0.0
HCM2kQueue:   8.6 6.2 3.2  15.2 23.3 23.3  6.6 25.8 25.8  2.4 20.4 0.0
-----
70thFactor:   1.18 1.19 1.19  1.17 1.16 1.16  1.18 1.15 1.15  1.19 1.16 1.20
HCM2k70thQ:  10.2 7.4 3.8   17.7 26.9 26.9   7.8 29.8 29.8   2.9 23.7 0.0
-----
85thFactor:   1.52 1.54 1.57  1.48 1.44 1.44  1.54 1.43 1.43  1.58 1.45 1.60
HCM2k85thQ:  13.2 9.6 5.0   22.4 33.5 33.5  10.1 36.9 36.9   3.8 29.6 0.0
-----
90thFactor:   1.66 1.69 1.74  1.59 1.52 1.52  1.69 1.51 1.51  1.75 1.54 1.80
HCM2k90thQ:  14.3 10.6 5.6   24.1 35.5 35.5  11.1 39.0 39.0   4.2 31.5 0.0
-----
95thFactor:   1.87 1.92 2.00  1.76 1.66 1.66  1.92 1.64 1.64  2.02 1.69 2.10
HCM2k95thQ:  16.2 12.0 6.4   26.7 38.7 38.7  12.6 42.4 42.4   4.9 34.6 0.0
-----
98thFactor:   2.21 2.32 2.48  2.01 1.87 1.87  2.30 1.84 1.84  2.53 1.91 2.70
HCM2k98thQ:  19.1 14.5 8.0   30.5 43.4 43.4  15.2 47.5 47.5   6.1 39.0 0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

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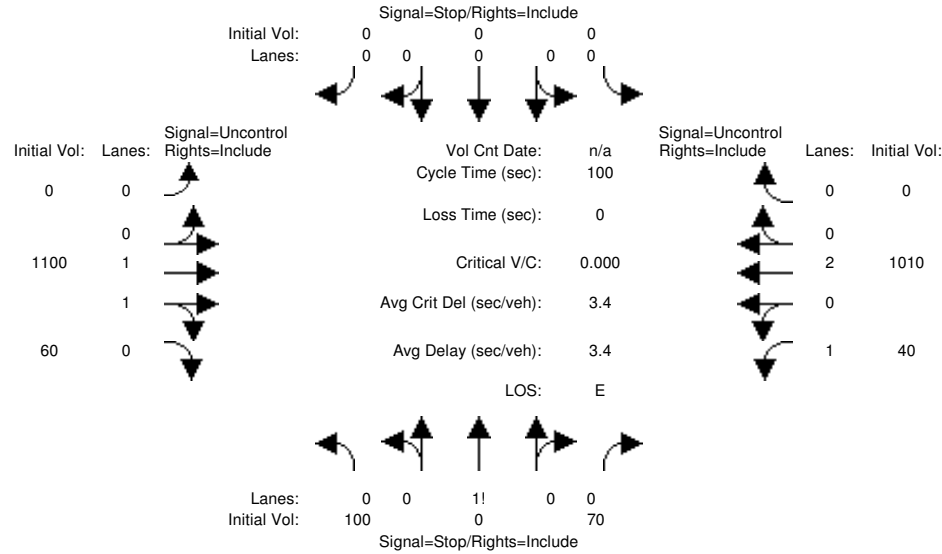
*****
Intersection #3 Meadowview Road/Freeport Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  39.9 63.7 19.1 106.9  192 76.5 59.5 271 34.5  12.2 257 0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 226.782 pounds
                  36.739 gallons
Carbon Dioxide:  707.561 pounds
Carbon Monoxide: 58.879 pounds
Hydrocarbons:   11.822 pounds
Nitrogen Oxides: 1.751 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 217.871 pounds
                  35.295 gallons
Carbon Dioxide:  679.757 pounds
Carbon Monoxide: 58.172 pounds
Hydrocarbons:   11.682 pounds
Nitrogen Oxides: 1.603 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj AM

Intersection #4: Meadowview Road/Manorside Drive



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:	-----											
Base Vol:	100	0	70	0	0	0	0	1100	60	40	1010	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	100	0	70	0	0	0	0	1100	60	40	1010	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	100	0	70	0	0	0	0	1100	60	40	1010	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Volume:	100	0	70	0	0	0	0	1100	60	40	1010	0
Critical Gap Module:	-----											
Critical Gap:	6.8	6.5	6.9	xxxxx	xxxx	xxxxxx	xxxxx	xxxx	xxxxxx	4.1	xxxx	xxxxxx
FollowUpTim:	3.5	4.0	3.3	xxxxx	xxxx	xxxxxx	xxxxx	xxxx	xxxxxx	2.2	xxxx	xxxxxx
Capacity Module:	-----											
Cnflct Vol:	1715	2220	580	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1160	xxxx	xxxxxx
Potent Cap.:	81	43	458	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	598	xxxx	xxxxxx
Move Cap.:	77	40	458	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	598	xxxx	xxxxxx
Total Cap:	190	143	xxxxxx	146	130	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Volume/Cap:	0.53	0.00	0.15	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.07	xxxx	xxxx

Level Of Service Module:	-----											
ZWay95thQ:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.2	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	11.5	xxxx	xxxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Queue:	xxxxx	250	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Shared Queue:	xxxxxx	4.4	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	45.2	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	E	*	*	*	*	*	*	*	*	*	*
ApproachDel:	45.2	*	xxxxxxx	*	*	*	xxxxxxx	*	*	xxxxxxx	*	*
ApproachLOS:	E	*	*	*	*	*	*	*	*	*	*	*

HevVeh:	2%	2%	2%	2%
Grade:	0%	0%	0%	0%
Peds/Hour:	0	0	0	0
Pedestrian Walk Speed:	4.00 feet/sec			
LaneWidth:	12 feet	12 feet	12 feet	12 feet
Time Period:	0.25 hour			

Two-Stage Gap Acceptance [Median Type: TWLTL] [Median Storage: 1 car]	-----											
Cnflct Vol:	1130	1130	xxxxxx	1090	1090	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Potent Cap.:	270	277	xxxxxx	230	289	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Move Cap.:	270	277	xxxxxx	214	270	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx

Two-Stage Gap Acceptance - Stage Two Module:	-----											
Cnflct Vol:	585	1090	xxxxxx	550	1160	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Potent Cap.:	520	289	xxxxxx	487	268	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Move Cap.:	485	270	xxxxxx	412	268	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx

 Peak Hour Delay Signal Warrant Report

 Intersection #4 Meadowview Road/Manorside Drive

 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign	Stop Sign	Uncontrolled	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	100	0	70	0	0	0	0	1100	60	40	1010	0
ApproachDel:	45.2	*	xxxxxxx	*	*	*	xxxxxxx	*	*	xxxxxxx	*	*

Approach[northbound] [lanes=1] [control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=2.1]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=170]
 SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3] [total volume=2380]
 SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	100	0	70	0	0	0	0	1100	60	40	1010	0
Major Street Volume:	2210											
Minor Approach Volume:	170											
Minor Approach Volume Threshold:	12 [less than minimum of 100]											

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj PM

Intersection #4: Meadowview Road/Manorside Drive

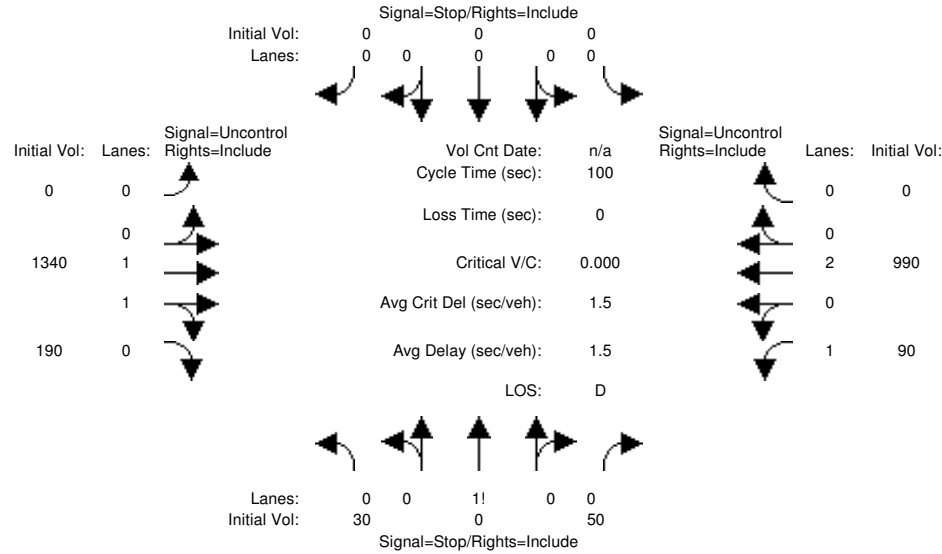


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Level Of Service Module, Shared Queue, Shared ConDel, and Shared LOS.

Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Two-Stage Gap Acceptance (Stage One and Stage Two) and Peak Hour Delay Signal Warrant Report.

Intersection #4 Meadowview Road/Manorside Drive
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound] [lanes=1] [control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.7]
Signal Warrant Rule #2: [approach volume=80]
Signal Warrant Rule #3: [approach count=3][total volume=2690]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an

"indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #4 Meadowview Road/Manorside Drive

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	2
Initial Vol:	30	0	50	0	0	0	0	1340	190	90	990	0
Major Street Volume:	2610											
Minor Approach Volume:	80											
Minor Approach Volume Threshold:	-46 [less than minimum of 100]											

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #5: Meadowview Road/24th Street

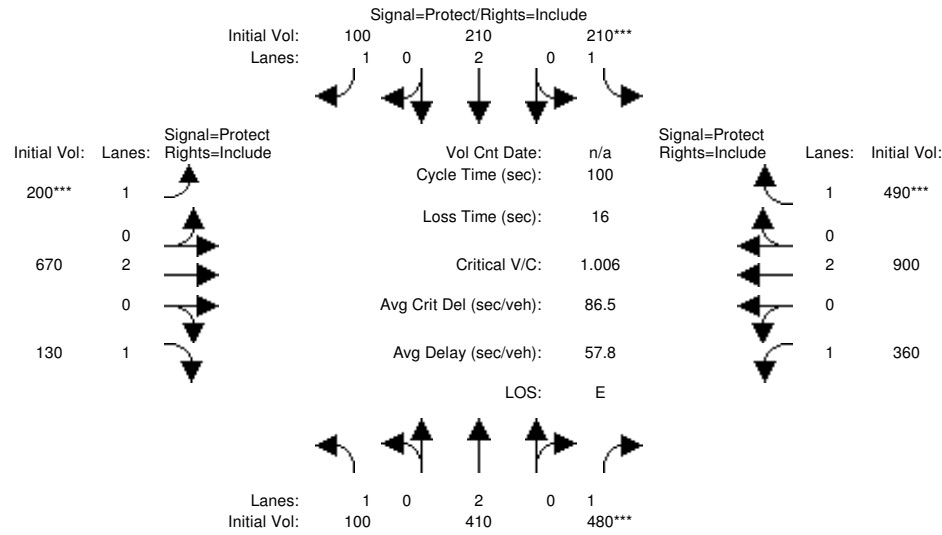


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.19 0.30  0.30  0.12 0.23  0.23  0.11 0.20  0.20  0.22 0.31  0.31
ArrivalType:  3              3              3              3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:          2.4  4.7 13.3  5.8  2.5  2.3  5.6  9.6  3.1  9.8 12.2 13.6
UpstreamVC:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj: 0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj: 1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:          0.4  0.6  7.9  5.2  0.3  0.4  5.1  5.3  0.7  5.3  3.7  8.0
HCM2kQueue:  2.8  5.4 21.2 11.0  2.8  2.6 10.6 14.9  3.8 15.1 15.9 21.6
-----
70thFactor:  1.19 1.19  1.16  1.18 1.19  1.19  1.18 1.17  1.19  1.17 1.17  1.16
HCM2k70thQ:  3.4  6.4 24.6 13.0  3.4  3.2 12.5 17.4  4.5 17.7 18.5 25.0
-----
85thFactor:  1.57 1.55  1.45  1.51 1.57  1.57  1.51 1.48  1.56  1.48 1.48  1.45
HCM2k85thQ:  4.4  8.3 30.8 16.6  4.5  4.2 16.0 22.1  5.9 22.4 23.5 31.2
-----
90thFactor:  1.75 1.71  1.54  1.63 1.75  1.75  1.64 1.59  1.73  1.59 1.58  1.54
HCM2k90thQ:  4.9  9.1 32.7 18.0  5.0  4.6 17.4 23.7  6.6 24.0 25.1 33.2
-----
95thFactor:  2.01 1.95  1.68  1.83 2.01  2.02  1.83 1.76  1.99  1.76 1.75  1.68
HCM2k95thQ:  5.7 10.4 35.8 20.1  5.7  5.3 19.5 26.3  7.5 26.6 27.8 36.3
-----
98thFactor:  2.51 2.36  1.90  2.13 2.50  2.52  2.14 2.02  2.45  2.01 1.99  1.89
HCM2k98thQ:  7.1 12.6 40.3 23.5  7.1  6.7 22.8 30.1  9.3 30.5 31.7 40.8
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #5 Meadowview Road/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  21.5 81.0 120.3 52.5 42.9 20.5 50.0 165 28.2 88.4 209 122.8
-----
Name: year 1995 composite fleet
Fuel Consumption:  225.191 pounds
                  36.481 gallons
Carbon Dioxide:   702.595 pounds
Carbon Monoxide:  58.943 pounds
Hydrocarbons:     12.009 pounds
Nitrogen Oxides:  1.673 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  217.121 pounds
                  35.174 gallons
Carbon Dioxide:   677.416 pounds
Carbon Monoxide:  58.304 pounds
Hydrocarbons:     11.881 pounds
Nitrogen Oxides:  1.540 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.21 0.23  0.23  0.28 0.30  0.30  0.10 0.18  0.18  0.14 0.22  0.22
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           4.6 2.8 14.4 19.4 4.3  5.5  4.7 13.3  2.4  9.7 11.2  8.6
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.0 0.4 21.7 28.3 0.5  1.0  4.3 20.2  0.5 15.4 6.3  4.5
HCM2kQueue:   5.6 3.1 36.1 47.7 4.9  6.5  9.0 33.5  2.9 25.1 17.5 13.1
-----
70thFactor:   1.19 1.19  1.14  1.13 1.19  1.19  1.18 1.14  1.19  1.15 1.16  1.17
HCM2k70thQ:   6.7 3.7 41.2 53.9 5.8  7.7 10.6 38.3  3.5 29.0 20.4 15.4
-----
85thFactor:   1.55 1.57  1.39  1.36 1.55  1.54  1.52 1.40  1.57  1.43 1.47  1.49
HCM2k85thQ:   8.7 4.9 50.2 65.0 7.6 10.0 13.7 46.8  4.6 36.0 25.7 19.6
-----
90thFactor:   1.70 1.74  1.47  1.44 1.71  1.69  1.66 1.47  1.75  1.51 1.57  1.61
HCM2k90thQ:   9.6 5.5 53.0 68.6 8.4 10.9 14.9 49.4  5.1 38.1 27.4 21.1
-----
95thFactor:   1.94 2.00  1.58  1.54 1.96  1.92  1.86 1.59  2.01  1.65 1.73  1.79
HCM2k95thQ:  10.9 6.3 57.1 73.6 9.6 12.4 16.7 53.4  5.9 41.5 30.2 23.5
-----
98thFactor:   2.35 2.49  1.76  1.73 2.39  2.31  2.20 1.78  2.50  1.84 1.96  2.06
HCM2k98thQ:  13.2 7.8 63.7 82.3 11.7 14.9 19.8 59.5  7.3 46.4 34.3 27.1
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #5 Meadowview Road/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  41.8 47.1 148.3 208.0 74.3 49.4 42.4 250 21.8 93.7 191 77.7
-----
Name: year 1995 composite fleet
Fuel Consumption: 547.714 pounds
                  88.730 gallons
Carbon Dioxide:  1708.866 pounds
Carbon Monoxide: 149.927 pounds
Hydrocarbons:   32.730 pounds
Nitrogen Oxides: 3.348 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 538.285 pounds
                  87.202 gallons
Carbon Dioxide:  1679.450 pounds
Carbon Monoxide: 149.177 pounds
Hydrocarbons:   32.581 pounds
Nitrogen Oxides: 3.188 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.28 0.28  0.28  0.07 0.07  0.07  0.05 0.34  0.34  0.15 0.44  0.44
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:          12.4 12.4  11.1  2.4 2.4  1.1  1.4 14.2  14.2  6.5 19.9  5.8
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           5.5 5.5  5.4  1.8 1.8  0.5  1.1 4.9  4.9  3.8 6.6  0.7
HCM2kQueue:   17.9 17.9  16.4  4.3 4.3  1.6  2.4 19.1  19.1  10.4 26.5  6.5
-----
70thFactor:   1.16 1.16  1.17  1.19 1.19  1.20  1.19 1.16  1.16  1.18 1.15  1.18
HCM2k70thQ:   20.9 20.9  19.2  5.1 5.1  1.9  2.9 22.2  22.2  12.2 30.5  7.7
-----
85thFactor:   1.47 1.47  1.47  1.56 1.56  1.58  1.58 1.46  1.46  1.51 1.42  1.54
HCM2k85thQ:   26.3 26.3  24.2  6.6 6.6  2.5  3.8 27.8  27.8  15.7 37.7  10.0
-----
90thFactor:   1.56 1.56  1.58  1.72 1.72  1.77  1.75 1.55  1.55  1.64 1.51  1.69
HCM2k90thQ:   28.0 28.0  25.9  7.3 7.3  2.8  4.2 29.6  29.6  17.0 39.9  11.0
-----
95thFactor:   1.72 1.72  1.74  1.97 1.97  2.05  2.02 1.71  1.71  1.84 1.64  1.92
HCM2k95thQ:   30.9 30.9  28.6  8.4 8.4  3.3  4.9 32.6  32.6  19.0 43.4  12.5
-----
98thFactor:   1.95 1.95  1.98  2.42 2.42  2.58  2.53 1.93  1.93  2.15 1.83  2.31
HCM2k98thQ:   35.0 35.0  32.6  10.3 10.3  4.1  6.1 36.8  36.8  22.3 48.5  15.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #6 Meadowview Road/Detroit Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  99.5 12.1  99.6  36.7  7.3  9.5  12.2 21.5  28.3  58.7  340  51.8
-----
Name: year 1995 composite fleet
Fuel Consumption:  178.611 pounds
                  28.935 gallons
Carbon Dioxide:   557.266 pounds
Carbon Monoxide:  45.822 pounds
Hydrocarbons:    9.024 pounds
Nitrogen Oxides:  1.432 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  170.759 pounds
                  27.663 gallons
Carbon Dioxide:   532.769 pounds
Carbon Monoxide:  45.200 pounds
Hydrocarbons:    8.900 pounds
Nitrogen Oxides:  1.303 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj PM

Intersection #6: Meadowview Road/Detroit Boulevard

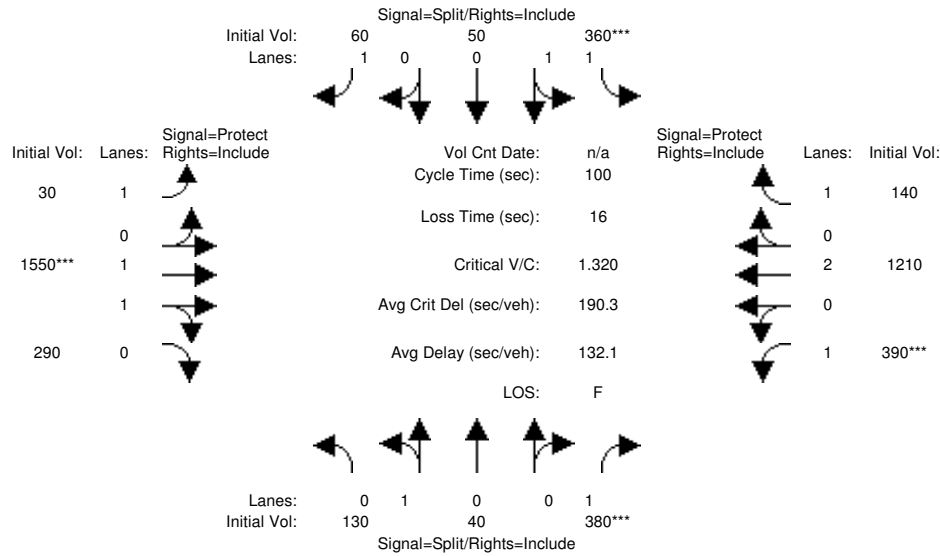


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.18 0.18  0.18  0.09 0.09  0.09  0.07 0.40  0.40  0.17 0.50  0.50
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           4.3 4.3 10.6  5.7 5.7  1.6  0.8 26.9  26.9 10.8 13.5  2.1
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.0 1.0 14.7  9.0 9.0  0.7  0.3 33.0  33.0 15.0 2.1  0.2
HCM2kQueue:   5.3 5.3 25.3 14.7 14.7  2.3  1.1 59.9  59.9 25.9 15.6  2.4
-----
70thFactor:   1.19 1.19  1.15  1.17 1.17  1.19  1.20 1.12  1.12  1.15 1.17  1.19
HCM2k70thQ:   6.3 6.3 29.2 17.2 17.2  2.7  1.3 67.2  67.2 29.8 18.2  2.8
-----
85thFactor:   1.55 1.55  1.43  1.48 1.48  1.58  1.59 1.34  1.34  1.43 1.48  1.58
HCM2k85thQ:   8.2 8.2 36.1 21.9 21.9  3.6  1.7 80.3  80.3 36.9 23.0  3.7
-----
90thFactor:   1.71 1.71  1.51  1.59 1.59  1.76  1.78 1.42  1.42  1.51 1.58  1.76
HCM2k90thQ:   9.0 9.0 38.3 23.4 23.4  4.0  1.9 85.0  85.0 39.1 24.7  4.1
-----
95thFactor:   1.95 1.95  1.65  1.76 1.76  2.03  2.06 1.52  1.52  1.64 1.75  2.03
HCM2k95thQ:  10.3 10.3 41.6 26.0 26.0  4.7  2.2 91.1  91.1 42.5 27.3  4.8
-----
98thFactor:   2.37 2.37  1.84  2.02 2.02  2.54  2.62 1.71  1.71  1.84 2.00  2.53
HCM2k98thQ:  12.5 12.5 46.6 29.8 29.8  5.8  2.8 102.4  102.4 47.5 31.2  6.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #6 Meadowview Road/Detroit Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  29.4  9.0 102.3  92.8 12.9 14.2  7.1 495  92.5 104.2  231 19.3
-----
Name: year 1995 composite fleet
Fuel Consumption:  482.953 pounds
                  78.238 gallons
Carbon Dioxide:   1506.813 pounds
Carbon Monoxide:  131.784 pounds
Hydrocarbons:    28.624 pounds
Nitrogen Oxides:  3.011 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  473.956 pounds
                  76.781 gallons
Carbon Dioxide:   1478.744 pounds
Carbon Monoxide:  131.066 pounds
Hydrocarbons:    28.482 pounds
Nitrogen Oxides:  2.857 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #7: Mack Road/Franklin Boulevard

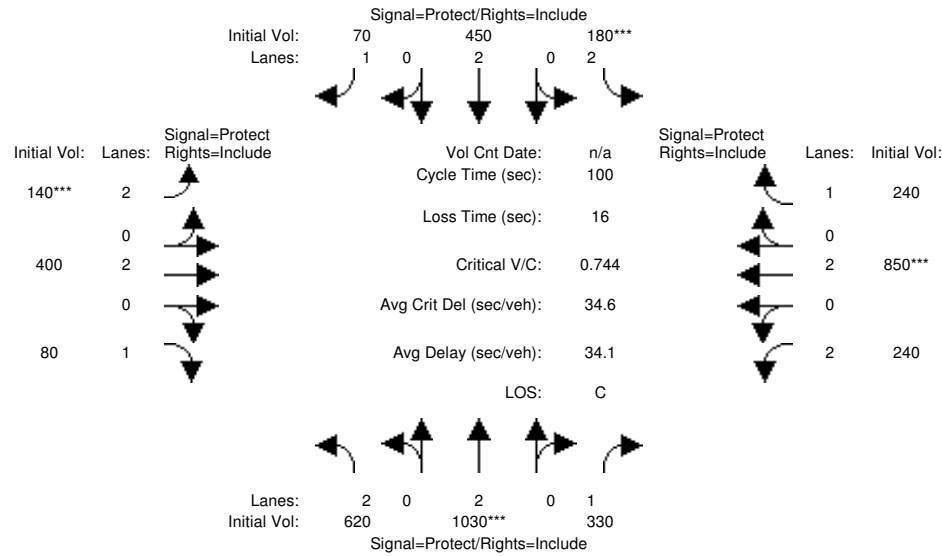


Table containing traffic engineering data including Approach, Movement, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.27 0.39  0.39  0.07 0.19  0.19  0.05 0.23  0.23  0.14 0.32  0.32
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           7.9 12.9  7.0  2.5  6.1  1.6  2.0  5.1  1.8  3.2 11.1  5.3
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.8  2.6  1.1  1.9  1.8  0.3  1.8  0.9  0.3  0.9  2.6  0.9
HCM2kQueue:   9.7 15.5  8.2  4.5  7.9  1.9  3.8  6.0  2.1  4.1 13.6  6.2
-----
70thFactor:   1.18 1.17  1.18  1.19 1.18  1.20  1.19 1.19  1.19  1.19 1.17  1.19
HCM2k70thQ:   11.5 18.1  9.6  5.3  9.3  2.3  4.5  7.1  2.5  4.8 16.0  7.3
-----
85thFactor:   1.52 1.48  1.53  1.56 1.53  1.58  1.56 1.55  1.58  1.56 1.49  1.54
HCM2k85thQ:   14.8 23.0  12.5  7.0 12.1  3.1  5.9  9.2  3.3  6.3 20.3  9.5
-----
90thFactor:   1.65 1.58  1.67  1.72 1.67  1.76  1.73 1.70  1.76  1.73 1.60  1.69
HCM2k90thQ:   16.0 24.6  13.6  7.7 13.2  3.4  6.6 10.1  3.6  7.0 21.8  10.5
-----
95thFactor:   1.85 1.75  1.88  1.97 1.89  2.04  1.99 1.93  2.03  1.98 1.78  1.93
HCM2k95thQ:   18.0 27.2  15.3  8.8 14.9  4.0  7.6 11.5  4.2  8.0 24.3  11.9
-----
98thFactor:   2.17 2.00  2.23  2.41 2.25  2.56  2.45 2.33  2.55  2.43 2.05  2.32
HCM2k98thQ:   21.1 31.1  18.2  10.8 17.7  5.0  9.3 13.9  5.3  9.9 28.0  14.4
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #7 Mack Road/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  137.9 221  63.4  44.1  104 14.8  34.5 86.4  16.1  55.2  189 47.9
-----
Name: year 1995 composite fleet
Fuel Consumption: 168.327 pounds
                  27.269 gallons
Carbon Dioxide:  525.180 pounds
Carbon Monoxide: 42.358 pounds
Hydrocarbons:    8.079 pounds
Nitrogen Oxides: 1.423 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 159.694 pounds
                  25.870 gallons
Carbon Dioxide:  498.245 pounds
Carbon Monoxide: 41.677 pounds
Hydrocarbons:    7.942 pounds
Nitrogen Oxides: 1.283 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.12 0.21  0.21  0.12 0.22  0.22  0.08 0.41  0.41  0.10 0.42  0.42
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            6.3  9.2  7.8  5.1 12.0  2.8  1.5 10.6  19.2  5.2 10.3  5.7
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            6.4  3.9  3.7  3.3  9.4  0.5  0.6  1.6  12.6  5.7  1.5  0.8
HCM2KQueue:   12.7 13.1  11.5  8.3 21.4  3.4  2.1 12.3  31.8  10.8 11.8  6.5
-----
70thFactor:   1.17 1.17  1.18  1.18 1.16  1.19  1.19 1.17  1.15  1.18 1.17  1.19
HCM2k70thQ:   14.9 15.3  13.5  9.9 24.8  4.0  2.5 14.4  36.4  12.7 13.9  7.6
-----
85thFactor:   1.50 1.49  1.50  1.53 1.45  1.57  1.58 1.50  1.40  1.51 1.50  1.54
HCM2k85thQ:   19.0 19.5  17.3  12.7 30.9  5.3  3.3 18.4  44.7  16.3 17.8  9.9
-----
90thFactor:   1.61 1.61  1.63  1.66 1.54  1.74  1.76 1.62  1.48  1.63 1.62  1.69
HCM2k90thQ:   20.4 21.0  18.7  13.9 32.9  5.8  3.7 19.8  47.1  17.7 19.2  10.9
-----
95thFactor:   1.80 1.79  1.82  1.88 1.68  2.00  2.03 1.80  1.60  1.83 1.81  1.92
HCM2k95thQ:   22.8 23.4  20.9  15.7 36.0  6.7  4.3 22.1  51.0  19.8 21.5  12.4
-----
98thFactor:   2.08 2.07  2.11  2.23 1.89  2.47  2.55 2.09  1.79  2.14 2.10  2.31
HCM2k98thQ:   26.3 27.0  24.3  18.6 40.5  8.3  5.4 25.6  56.8  23.1 24.9  14.9
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #7 Mack Road/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  111.1 157  69.8  88.3  209  25.4  26.1 182 181.5  90.7  177  51.3
-----
Name: year 1995 composite fleet
Fuel Consumption:  319.978 pounds
                  51.836 gallons
Carbon Dioxide:   998.331 pounds
Carbon Monoxide:  83.897 pounds
Hydrocarbons:    17.151 pounds
Nitrogen Oxides:  2.352 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  308.763 pounds
                  50.020 gallons
Carbon Dioxide:   963.342 pounds
Carbon Monoxide:  83.010 pounds
Hydrocarbons:    16.974 pounds
Nitrogen Oxides:  2.167 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #8: Cosumnes River Boulevard/Franklin Boulevard

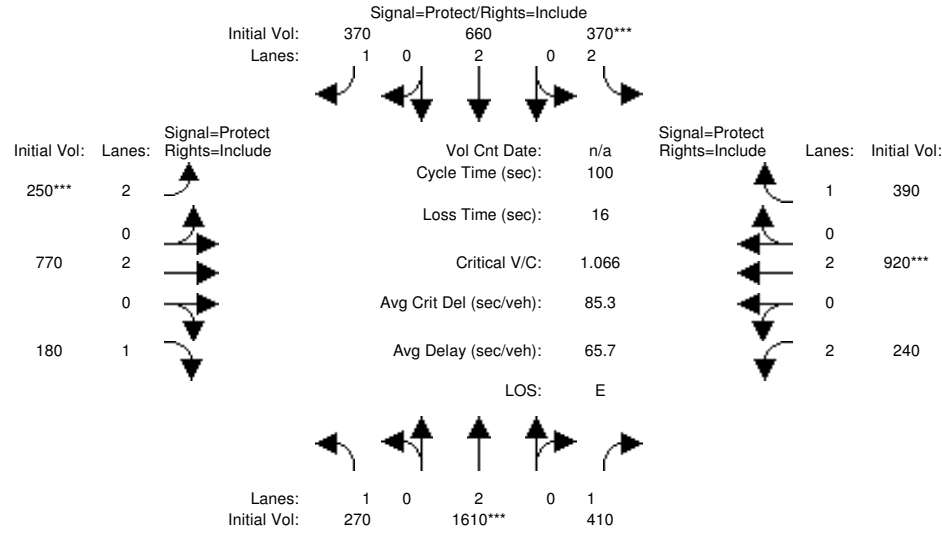


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include: Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.21 0.43  0.43  0.10 0.32  0.32  0.07 0.24  0.24  0.08 0.24  0.24
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            7.0 23.5  8.8   5.3 8.1   9.1   3.6 11.0  4.3   3.4 13.5  10.8
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            2.3 14.1  1.5   5.7 1.3   2.4   4.5 5.3   0.9   3.3 9.9   7.2
HCM2kQueue:    9.3 37.6  10.3  11.0 9.4   11.5   8.1 16.3   5.2   6.7 23.3  18.1
-----
70thFactor:    1.18 1.14  1.18  1.18 1.18  1.17  1.18 1.17  1.19  1.18 1.16  1.16
HCM2k70thQ:    11.0 42.9  12.1  12.9 11.1  13.5   9.6 19.0   6.2   8.0 27.0  21.0
-----
85thFactor:    1.52 1.39  1.51  1.51 1.52  1.50  1.53 1.47  1.55  1.54 1.44  1.46
HCM2k85thQ:    14.1 52.2  15.6  16.6 14.3  17.3  12.4 24.0   8.1  10.4 33.6  26.5
-----
90thFactor:    1.65 1.46  1.64  1.63 1.65  1.62  1.67 1.58  1.71  1.69 1.52  1.56
HCM2k90thQ:    15.4 55.0  16.9  17.9 15.5  18.7  13.5 25.7   8.9  11.4 35.6  28.2
-----
95thFactor:    1.86 1.57  1.84  1.83 1.86  1.82  1.88 1.74  1.95  1.91 1.66  1.72
HCM2k95thQ:    17.3 59.3  18.9  20.0 17.5  20.9  15.3 28.4  10.1  12.9 38.8  31.1
-----
98thFactor:    2.19 1.76  2.15  2.13 2.18  2.11  2.24 1.99  2.37  2.30 1.87  1.95
HCM2k98thQ:    20.4 66.1  22.2  23.4 20.6  24.3  18.2 32.3  12.3  15.5 43.6  35.2
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #8 Cosumnes River Boulevard/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  63.0 423  79.3  93.2 138 82.2  62.8 188  38.8  59.6 235  97.8
-----
Name: year 1995 composite fleet
Fuel Consumption:  375.803 pounds
                  60.880 gallons
Carbon Dioxide:   1172.506 pounds
Carbon Monoxide:   99.145 pounds
Hydrocarbons:     20.448 pounds
Nitrogen Oxides:   2.718 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  363.515 pounds
                  58.889 gallons
Carbon Dioxide:   1134.167 pounds
Carbon Monoxide:   98.170 pounds
Hydrocarbons:     20.253 pounds
Nitrogen Oxides:   2.513 pounds
    
```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.10 0.29  0.29  0.15 0.34  0.34  0.12 0.24  0.24  0.16 0.27  0.27
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           5.0  9.9  7.6  5.3 18.4  8.5  6.2 12.9  6.9  8.0 14.3  7.8
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           5.1  2.5  1.9  2.2 10.6  1.8  5.4  8.7  2.1  6.7  8.4  2.2
HCM2kQueue:   10.1 12.4  9.6  7.6 29.1  10.3  11.6 21.5  9.0  14.7 22.7  10.0
-----
70thFactor:   1.18 1.17  1.18  1.18 1.15  1.18  1.17 1.16  1.18  1.17 1.16  1.18
HCM2k70thQ:   11.9 14.5  11.3  8.9 33.4  12.1  13.6 24.9  10.6  17.2 26.3  11.7
-----
85thFactor:   1.51 1.50  1.52  1.53 1.41  1.51  1.50 1.45  1.52  1.48 1.44  1.52
HCM2k85thQ:   15.4 18.5  14.5  11.6 41.1  15.6  17.4 31.1  13.7  21.8 32.7  15.1
-----
90thFactor:   1.64 1.62  1.65  1.67 1.49  1.64  1.62 1.54  1.66  1.59 1.53  1.64
HCM2k90thQ:   16.6 20.0  15.7  12.7 43.4  16.9  18.8 33.1  14.9  23.4 34.7  16.4
-----
95thFactor:   1.84 1.80  1.85  1.89 1.62  1.84  1.82 1.68  1.86  1.77 1.67  1.84
HCM2k95thQ:   18.7 22.3  17.7  14.3 47.1  18.9  21.0 36.2  16.8  25.9 37.9  18.4
-----
98thFactor:   2.16 2.09  2.18  2.26 1.81  2.15  2.11 1.89  2.20  2.02 1.87  2.16
HCM2k98thQ:   21.9 25.8  20.8  17.1 52.5  22.2  24.4 40.7  19.8  29.7 42.6  21.6
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #8 Cosumnes River Boulevard/Franklin Boulevard
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  45.2 169  68.6  93.0  321  76.4  107.7  222  61.8  140.9  246  70.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  378.749 pounds
                  61.357 gallons
Carbon Dioxide:   1181.697 pounds
Carbon Monoxide:  99.690 pounds
Hydrocarbons:    20.481 pounds
Nitrogen Oxides:  2.767 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  365.997 pounds
                  59.292 gallons
Carbon Dioxide:   1141.911 pounds
Carbon Monoxide:  98.678 pounds
Hydrocarbons:    20.279 pounds
Nitrogen Oxides:  2.554 pounds
    
```

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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.08 0.13  0.13  0.09 0.14  0.14  0.08 0.48  0.48  0.14 0.54  0.54
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           1.9  1.5  1.5  1.0  3.1  3.1  1.9  5.5  5.5  0.2  8.9  8.9
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.9  0.3  0.3  0.3  1.0  1.0  0.9  0.6  0.6  0.0  1.0  1.0
HCM2kQueue:   2.8  1.8  1.8  1.4  4.0  4.0  2.8  6.1  6.1  0.3 10.0  10.0
-----
70thFactor:   1.19 1.20  1.20  1.20 1.19  1.19  1.19 1.19  1.19  1.20 1.18  1.18
HCM2k70thQ:   3.3  2.2  2.2  1.6  4.8  4.8  3.3  7.2  7.2  0.3 11.7  11.7
-----
85thFactor:   1.57 1.58  1.58  1.59 1.56  1.56  1.57 1.55  1.55  1.60 1.52  1.52
HCM2k85thQ:   4.4  2.9  2.9  2.1  6.3  6.3  4.4  9.3  9.3  0.5 15.1  15.1
-----
90thFactor:   1.75 1.76  1.76  1.77 1.73  1.73  1.75 1.70  1.70  1.79 1.64  1.64
HCM2k90thQ:   4.9  3.2  3.2  2.4  7.0  7.0  4.9 10.3  10.3  0.5 16.4  16.4
-----
95thFactor:   2.01 2.04  2.04  2.06 1.98  1.98  2.01 1.93  1.93  2.09 1.84  1.84
HCM2k95thQ:   5.6  3.8  3.8  2.8  8.0  8.0  5.6 11.7  11.7  0.6 18.4  18.4
-----
98thFactor:   2.51 2.57  2.57  2.60 2.43  2.43  2.51 2.33  2.33  2.68 2.16  2.16
HCM2k98thQ:   7.0  4.7  4.7  3.5  9.8  9.8  7.0 14.1  14.1  0.8 21.6  21.6
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #14 Cosumnes River Boulevard/Street A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  16.8 11.3  2.3  9.3  6.9 20.8  16.8 126  9.4  2.2  214  6.3
-----
Name: year 1995 composite fleet
Fuel Consumption:  66.902 pounds
                  10.838 gallons
Carbon Dioxide:   208.734 pounds
Carbon Monoxide:  15.984 pounds
Hydrocarbons:     2.797 pounds
Nitrogen Oxides:  0.617 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  62.267 pounds
                  10.087 gallons
Carbon Dioxide:   194.272 pounds
Carbon Monoxide:  15.623 pounds
Hydrocarbons:     2.723 pounds
Nitrogen Oxides:  0.548 pounds
    
```

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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.09 0.15  0.15  0.11 0.16  0.16  0.12 0.54  0.54  0.05 0.47  0.47
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            2.4  2.5  2.5  1.3  3.8  3.8  3.1  9.7  9.7  0.3  9.7  9.7
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            1.1  0.6  0.6  0.4  1.1  1.1  1.2  1.2  1.2  0.1  1.2  1.2
HCM2kQueue:    3.5  3.1  3.1  1.6  5.0  5.0  4.3 10.9  10.9  0.4 10.9  10.9
-----
70thFactor:    1.19 1.19  1.19  1.20 1.19  1.19  1.19 1.18  1.18  1.20 1.18  1.18
HCM2k70thQ:    4.1  3.7  3.7  2.0  5.9  5.9  5.1 12.8  12.8  0.5 12.9  12.9
-----
85thFactor:    1.57 1.57  1.57  1.58 1.55  1.55  1.56 1.51  1.51  1.60 1.51  1.51
HCM2k85thQ:    5.4  4.8  4.8  2.6  7.7  7.7  6.7 16.4  16.4  0.6 16.5  16.5
-----
90thFactor:    1.74 1.74  1.74  1.77 1.71  1.71  1.72 1.63  1.63  1.79 1.63  1.63
HCM2k90thQ:    6.0  5.4  5.4  2.9  8.5  8.5  7.4 17.8  17.8  0.7 17.9  17.9
-----
95thFactor:    2.00 2.01  2.01  2.05 1.96  1.96  1.97 1.83  1.83  2.09 1.83  1.83
HCM2k95thQ:    6.9  6.2  6.2  3.4  9.7  9.7  8.5 19.9  19.9  0.8 20.0  20.0
-----
98thFactor:    2.47 2.49  2.49  2.58 2.38  2.38  2.42 2.13  2.13  2.67 2.13  2.13
HCM2k98thQ:    8.5  7.7  7.7  4.2 11.8  11.8  10.4 23.2  23.2  1.0 23.3  23.3
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

 Intersection #14 Cosumnes River Boulevard/Street A

```

Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    21.5 20.3  2.3  11.5 18.4  16.1  28.3 209  29.4  2.4  228  10.8
-----
    
```

```

Name: year 1995 composite fleet
Fuel Consumption:  89.440 pounds
                  14.489 gallons
Carbon Dioxide:   279.053 pounds
Carbon Monoxide:  21.579 pounds
Hydrocarbons:     3.835 pounds
Nitrogen Oxides:  0.818 pounds
    
```

```

Name: year 2000 composite fleet
Fuel Consumption:  83.523 pounds
                  13.531 gallons
Carbon Dioxide:   260.591 pounds
Carbon Monoxide:  21.117 pounds
Hydrocarbons:     3.741 pounds
Nitrogen Oxides:  0.728 pounds
    
```

DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #15: Cosumnes River Boulevard/Street B

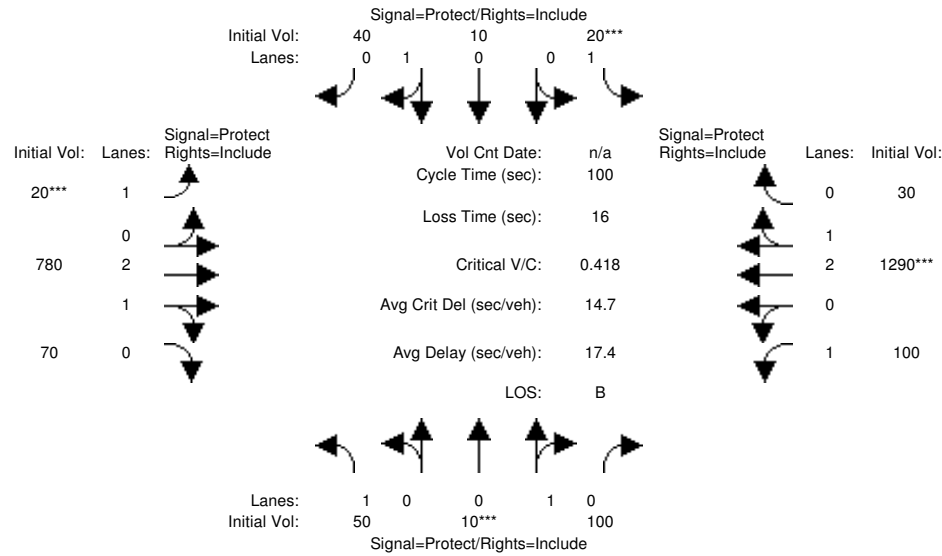


Table containing traffic engineering data including: Approach: North Bound, South Bound, East Bound, West Bound; Volume Module; Saturation Flow Module; Capacity Analysis Module; HCM Ops Adjusted Lane Utilization Module; HCM Ops Input Saturation Adj Module; HCM Ops f(lt) Adj Case Module; HCM Ops Saturation Adj Module; Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.08 0.15  0.15  0.05 0.12  0.12  0.05 0.48  0.48  0.16 0.59  0.59
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           1.3 2.8  2.8  0.5 1.3  1.3  0.5 5.4  5.4  2.5 7.5  7.5
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.5 0.8  0.8  0.3 0.3  0.3  0.3 0.5  0.5  0.5 0.8  0.8
HCM2kQueue:   1.8 3.5  3.5  0.8 1.6  1.6  0.8 6.0  6.0  3.0 8.3  8.3
-----
70thFactor:   1.20 1.19  1.19  1.20 1.20  1.20  1.20 1.19  1.19  1.19 1.18  1.18
HCM2k70thQ:   2.1 4.2  4.2  1.0 1.9  1.9  1.0 7.1  7.1  3.6 9.8  9.8
-----
85thFactor:   1.58 1.57  1.57  1.59 1.58  1.58  1.59 1.55  1.55  1.57 1.53  1.53
HCM2k85thQ:   2.8 5.5  5.5  1.3 2.5  2.5  1.3 9.3  9.3  4.7 12.7  12.7
-----
90thFactor:   1.77 1.74  1.74  1.78 1.77  1.77  1.78 1.70  1.70  1.74 1.66  1.66
HCM2k90thQ:   3.2 6.1  6.1  1.5 2.8  2.8  1.5 10.2  10.2  5.3 13.8  13.8
-----
95thFactor:   2.04 1.99  1.99  2.07 2.05  2.05  2.07 1.93  1.93  2.01 1.88  1.88
HCM2k95thQ:   3.7 7.1  7.1  1.7 3.3  3.3  1.7 11.6  11.6  6.0 15.6  15.6
-----
98thFactor:   2.57 2.46  2.46  2.64 2.58  2.58  2.64 2.33  2.33  2.49 2.23  2.23
HCM2k98thQ:   4.6 8.7  8.7  2.2 4.1  4.1  2.2 14.0  14.0  7.5 18.5  18.5
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #15 Cosumnes River Boulevard/Street B
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  11.8 2.3  22.7  4.8 2.3  9.1  4.8 123  11.0  22.3 180  4.2
-----
Name: year 1995 composite fleet
Fuel Consumption: 60.662 pounds
                  9.827 gallons
Carbon Dioxide:  189.265 pounds
Carbon Monoxide: 14.341 pounds
Hydrocarbons:   2.465 pounds
Nitrogen Oxides: 0.566 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 56.252 pounds
                  9.113 gallons
Carbon Dioxide:  175.507 pounds
Carbon Monoxide: 13.999 pounds
Hydrocarbons:   2.394 pounds
Nitrogen Oxides: 0.501 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj PM

Intersection #15: Cosumnes River Boulevard/Street B

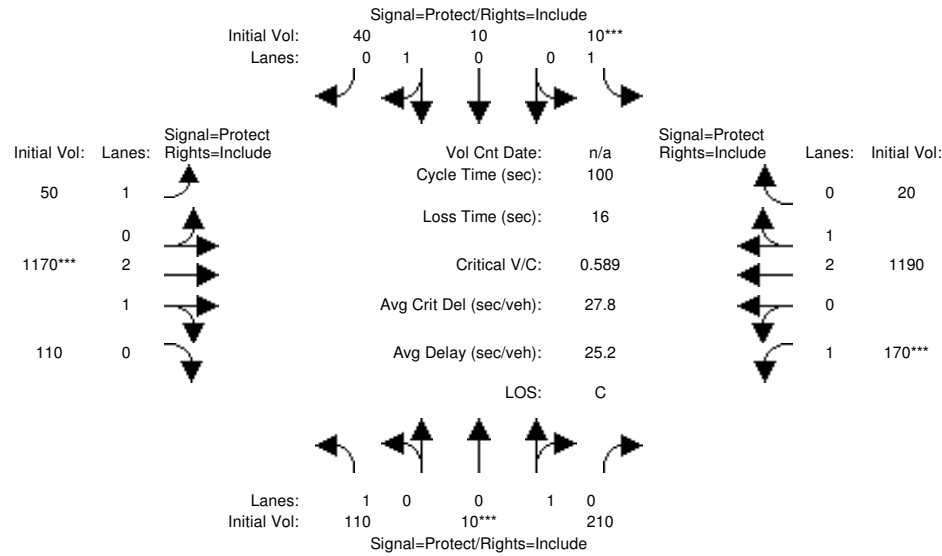


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include: Min. Green, Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.13 0.22  0.22  0.05 0.14  0.14  0.10 0.41  0.41  0.16 0.47  0.47
ArrivalType:      3          3          3          3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           2.8  5.5  5.5  0.3 1.2  1.2  1.3 10.3  10.3  4.4  8.6  8.6
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.9  1.5  1.5  0.1 0.3  0.3  0.4  1.6  1.6  1.5  1.0  1.0
HCM2kQueue:   3.7  7.0  7.0  0.4 1.5  1.5  1.7 11.8  11.8  5.9  9.6  9.6
-----
70thFactor:   1.19 1.18  1.18  1.20 1.20  1.20  1.20 1.17  1.17  1.19 1.18  1.18
HCM2k70thQ:   4.4  8.3  8.3  0.5 1.8  1.8  2.0 13.9  13.9  7.0 11.3  11.3
-----
85thFactor:   1.56 1.54  1.54  1.60 1.59  1.59  1.58 1.50  1.50  1.55 1.52  1.52
HCM2k85thQ:   5.8 10.8  10.8  0.6 2.4  2.4  2.7 17.8  17.8  9.1 14.6  14.6
-----
90thFactor:   1.73 1.68  1.68  1.79 1.77  1.77  1.77 1.62  1.62  1.70 1.65  1.65
HCM2k90thQ:   6.5 11.8  11.8  0.7 2.6  2.6  3.0 19.2  19.2  10.0 15.8  15.8
-----
95thFactor:   1.99 1.91  1.91  2.09 2.05  2.05  2.05 1.81  1.81  1.93 1.85  1.85
HCM2k95thQ:   7.4 13.4  13.4  0.8 3.1  3.1  3.4 21.4  21.4  11.4 17.8  17.8
-----
98thFactor:   2.45 2.28  2.28  2.67 2.59  2.59  2.58 2.10  2.10  2.34 2.18  2.18
HCM2k98thQ:   9.1 16.0  16.0  1.0 3.9  3.9  4.3 24.9  24.9  13.7 20.9  20.9
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #15 Cosumnes River Boulevard/Street B
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  25.6  2.3  47.3  2.4  2.2  8.8  11.6 231  21.7  39.7  207  3.5
-----
Name: year 1995 composite fleet
Fuel Consumption:  92.703 pounds
                  15.018 gallons
Carbon Dioxide:   289.233 pounds
Carbon Monoxide:  22.716 pounds
Hydrocarbons:     4.145 pounds
Nitrogen Oxides:  0.827 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  87.066 pounds
                  14.105 gallons
Carbon Dioxide:   271.644 pounds
Carbon Monoxide:  22.274 pounds
Hydrocarbons:     4.056 pounds
Nitrogen Oxides:  0.739 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.05 0.10  0.23  0.14 0.19  0.19  0.12 0.47  0.47  0.14 0.48  0.48
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            1.1  0.8  2.3  4.8  1.0  5.5  4.0  7.5  7.5  2.8 13.4 13.4
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            0.7  0.2  0.4  2.1  0.2  1.9  1.9  0.8  0.8  0.8  2.1  2.1
HCM2kQueue:    1.8  1.0  2.7  6.9  1.1  7.4  5.9  8.4  8.4  3.6 15.6 15.6
-----
70thFactor:    1.20 1.20  1.19  1.18 1.20  1.18  1.19 1.18  1.18  1.19 1.17  1.17
HCM2k70thQ:    2.2  1.2  3.2  8.2  1.4  8.8  6.9  9.9  9.9  4.3 18.2 18.2
-----
85thFactor:    1.58 1.59  1.57  1.54 1.59  1.53  1.55 1.53  1.53  1.57 1.48  1.48
HCM2k85thQ:    2.9  1.6  4.2  10.6 1.8 11.4  9.1 12.8 12.8  5.7 23.0 23.0
-----
90thFactor:    1.77 1.78  1.75  1.68 1.78  1.68  1.70 1.66  1.66  1.73 1.58  1.58
HCM2k90thQ:    3.2  1.8  4.6  11.6  2.0 12.4  9.9 13.9 13.9  6.3 24.7 24.7
-----
95thFactor:    2.04 2.07  2.02  1.91 2.06  1.90  1.93 1.88  1.88  1.99 1.75  1.75
HCM2k95thQ:    3.7  2.1  5.3  13.2  2.3 14.1 11.3 15.7 15.7  7.2 27.3 27.3
-----
98thFactor:    2.57 2.62  2.52  2.29 2.62  2.27  2.34 2.23  2.23  2.46 2.00  2.00
HCM2k98thQ:    4.7  2.7  6.7  15.8  3.0 16.8 13.7 18.6 18.6  8.9 31.2 31.2
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #16 Cosumnes River Boulevard/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:     9.7 13.8 20.5 43.1  8.4 46.7 36.0 125  3.4 25.4 224  5.8
-----
Name: year 1995 composite fleet
Fuel Consumption: 89.677 pounds
                  14.528 gallons
Carbon Dioxide:  279.792 pounds
Carbon Monoxide: 22.178 pounds
Hydrocarbons:    4.114 pounds
Nitrogen Oxides: 0.783 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 84.524 pounds
                  13.693 gallons
Carbon Dioxide:  263.716 pounds
Carbon Monoxide: 21.774 pounds
Hydrocarbons:    4.032 pounds
Nitrogen Oxides: 0.702 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.05 0.14  0.31  0.10 0.19  0.19  0.20 0.43  0.43  0.17 0.40  0.40
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           1.6  1.0  5.7  1.8  2.3  6.7  7.4 13.2  13.2  5.8 14.3  14.3
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.5  0.2  1.0  0.6  0.4  2.8  2.9  2.4  2.4  2.2  3.3  3.3
HCM2kQueue:   3.1  1.2  6.7  2.4  2.7  9.5  10.3 15.7  15.7  8.0 17.5  17.5
-----
70thFactor:  1.19 1.20  1.18  1.19 1.19  1.18  1.18 1.17  1.17  1.18 1.16  1.16
HCM2k70thQ:  3.7  1.5  7.9  2.9  3.2  11.2  12.1 18.3  18.3  9.4 20.4  20.4
-----
85thFactor:  1.57 1.59  1.54  1.58 1.57  1.52  1.51 1.48  1.48  1.53 1.47  1.47
HCM2k85thQ:  4.9  1.9  10.3  3.8  4.2  14.5  15.6 23.2  23.2  12.2 25.7  25.7
-----
90thFactor:  1.74 1.78  1.69  1.75 1.75  1.65  1.64 1.58  1.58  1.67 1.57  1.57
HCM2k90thQ:  5.4  2.2  11.2  4.3  4.7  15.7  16.9 24.8  24.8  13.3 27.4  27.4
-----
95thFactor:  2.00 2.06  1.91  2.02 2.02  1.85  1.84 1.75  1.75  1.88 1.73  1.73
HCM2k95thQ:  6.3  2.5  12.8  4.9  5.4  17.6  18.9 27.5  27.5  15.1 30.3  30.3
-----
98thFactor:  2.49 2.61  2.30  2.53 2.51  2.18  2.15 2.00  2.00  2.24 1.96  1.96
HCM2k98thQ:  7.8  3.2  15.3  6.2  6.7  20.8  22.2 31.4  31.4  17.9 34.3  34.3
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #16 Cosumnes River Boulevard/24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  14.8 17.6  51.0  16.4 19.3  57.2  66.5 214  12.5  52.1  237  6.6
-----
Name: year 1995 composite fleet
Fuel Consumption:  125.671 pounds
                  20.359 gallons
Carbon Dioxide:   392.094 pounds
Carbon Monoxide:  31.584 pounds
Hydrocarbons:     6.010 pounds
Nitrogen Oxides:  1.067 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  119.164 pounds
                  19.304 gallons
Carbon Dioxide:   371.790 pounds
Carbon Monoxide:  31.070 pounds
Hydrocarbons:     5.907 pounds
Nitrogen Oxides:  0.961 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.00 0.00  0.00  0.28 0.00  0.28  0.09 0.60  0.00  0.00 0.51  0.51
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  7.6 0.0  7.6  3.0 7.1  0.0  0.0 12.7  12.7
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  1.00 1.00  0.00  0.00 1.00  1.00
Q2:           0.0 0.0  0.0  1.8 0.0  1.8  1.6 0.7  0.0  0.0 1.9  1.9
HCM2kQueue:   0.0 0.0  0.0  9.4 0.0  9.4  4.6 7.8  0.0  0.0 14.6  14.6
-----|-----|-----|-----|
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.18  1.19 1.18  1.20  1.20 1.17  1.17
HCM2k70thQ:   0.0 0.0  0.0  11.1 0.0  11.1  5.4 9.2  0.0  0.0 17.1  17.1
-----|-----|-----|-----|
85thFactor:   1.60 1.60  1.60  1.52 1.60  1.52  1.56 1.53  1.60  1.60 1.48  1.48
HCM2k85thQ:   0.0 0.0  0.0  14.3 0.0  14.3  7.1 11.9  0.0  0.0 21.7  21.7
-----|-----|-----|-----|
90thFactor:   1.80 1.80  1.80  1.65 1.80  1.65  1.72 1.67  1.80  1.80 1.59  1.59
HCM2k90thQ:   0.0 0.0  0.0  15.5 0.0  15.5  7.8 13.0  0.0  0.0 23.3  23.3
-----|-----|-----|-----|
95thFactor:   2.10 2.10  2.10  1.86 2.10  1.86  1.97 1.89  2.10  2.10 1.77  1.77
HCM2k95thQ:   0.0 0.0  0.0  17.5 0.0  17.5  9.0 14.7  0.0  0.0 25.8  25.8
-----|-----|-----|-----|
98thFactor:   2.70 2.70  2.70  2.18 2.70  2.18  2.40 2.25  2.70  2.70 2.03  2.03
HCM2k98thQ:   0.0 0.0  0.0  20.6 0.0  20.6  11.0 17.5  0.0  0.0 29.6  29.6
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  22.1 0.0  46.4  26.6 121  0.0  0.0 197  20.5
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 64.567 pounds
                  10.460 gallons
Carbon Dioxide:  201.448 pounds
Carbon Monoxide: 15.462 pounds
Hydrocarbons:   2.713 pounds
Nitrogen Oxides: 0.597 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 60.135 pounds
                  9.742 gallons
Carbon Dioxide:  187.620 pounds
Carbon Monoxide: 15.117 pounds
Hydrocarbons:   2.643 pounds
Nitrogen Oxides: 0.530 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  | 0.28 0.00  0.28  | 0.15 0.60  0.00  | 0.00 0.45  0.45
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  | 1.00 1.00  1.00  | 1.00 1.00  1.00  | 1.00 1.00  1.00
Q1:           0.0 0.0  0.0  | 8.6 0.0  8.6  | 5.0 9.8  0.0  | 0.0 13.8 13.8
UpstreamVC:   0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  | 1.00 0.00  1.00  | 1.00 1.00  0.00  | 0.00 1.00  1.00
Q2:           0.0 0.0  0.0  | 2.3 0.0  2.3  | 2.2 1.1  0.0  | 0.0 2.5  2.5
HCM2kQueue:   0.0 0.0  0.0  | 10.9 0.0 10.9  | 7.2 10.9  0.0  | 0.0 16.3 16.3
-----
70thFactor:   1.20 1.20  1.20  | 1.18 1.20  1.18  | 1.18 1.18  1.20  | 1.20 1.17  1.17
HCM2k70thQ:   0.0 0.0  0.0  | 12.8 0.0 12.8  | 8.5 12.9  0.0  | 0.0 19.0 19.0
-----
85thFactor:   1.60 1.60  1.60  | 1.51 1.60  1.51  | 1.54 1.51  1.60  | 1.60 1.47  1.47
HCM2k85thQ:   0.0 0.0  0.0  | 16.4 0.0 16.4  | 11.1 16.5  0.0  | 0.0 24.0 24.0
-----
90thFactor:   1.80 1.80  1.80  | 1.63 1.80  1.63  | 1.68 1.63  1.80  | 1.80 1.58  1.58
HCM2k90thQ:   0.0 0.0  0.0  | 17.8 0.0 17.8  | 12.1 17.8  0.0  | 0.0 25.7 25.7
-----
95thFactor:   2.10 2.10  2.10  | 1.83 2.10  1.83  | 1.90 1.83  2.10  | 2.10 1.74  1.74
HCM2k95thQ:   0.0 0.0  0.0  | 19.9 0.0 19.9  | 13.7 20.0  0.0  | 0.0 28.4 28.4
-----
98thFactor:   2.70 2.70  2.70  | 2.13 2.70  2.13  | 2.27 2.13  2.70  | 2.70 1.99  1.99
HCM2k98thQ:   0.0 0.0  0.0  | 23.2 0.0 23.2  | 16.4 23.3  0.0  | 0.0 32.4 32.4
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #17 Cosumnes River Boulevard/Stone-Boswell Access West
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  | 29.4 0.0 47.5  | 45.4 168  0.0  | 0.0 220 16.3
-----
Name: year 1995 composite fleet
Fuel Consumption: 79.447 pounds
                  12.870 gallons
Carbon Dioxide:  247.876 pounds
Carbon Monoxide: 19.278 pounds
Hydrocarbons:    3.460 pounds
Nitrogen Oxides: 0.720 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 74.347 pounds
                  12.044 gallons
Carbon Dioxide:  231.962 pounds
Carbon Monoxide: 18.879 pounds
Hydrocarbons:    3.379 pounds
Nitrogen Oxides: 0.642 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

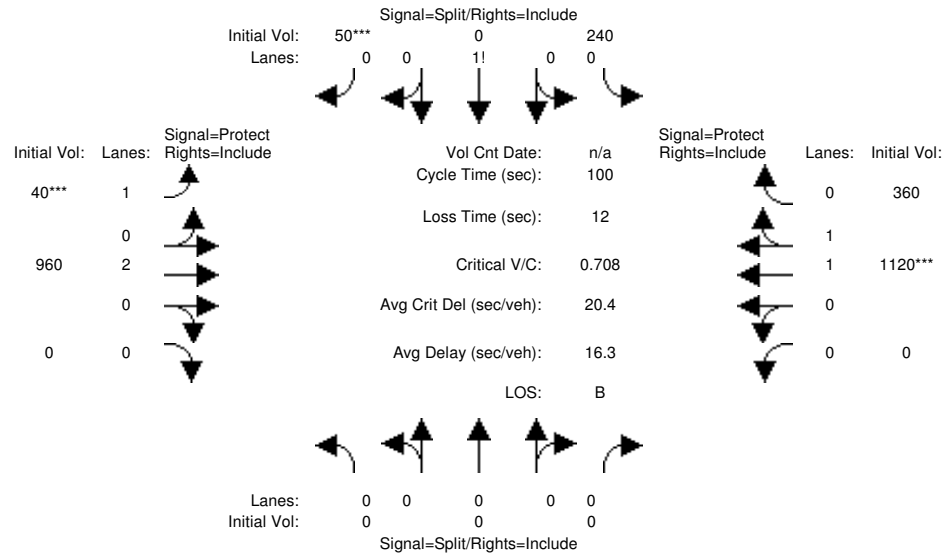


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.23 0.00  0.23  0.05 0.65  0.00  0.00 0.60  0.60
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  7.4 0.0  7.4  1.1 6.7  0.0  0.0 15.3  15.3
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  1.00 1.00  0.00  0.00 1.00  1.00
Q2:           0.0 0.0  0.0  2.2 0.0  2.2  0.7 0.7  0.0  0.0 2.4  2.4
HCM2kQueue:   0.0 0.0  0.0  9.7 0.0  9.7  1.8 7.4  0.0  0.0 17.7  17.7
-----
70thFactor:  1.20 1.20  1.20  1.18 1.20  1.18  1.20 1.18  1.20  1.20 1.16  1.16
HCM2k70thQ:  0.0 0.0  0.0  11.4 0.0  11.4  2.2 8.8  0.0  0.0 20.6  20.6
-----
85thFactor:  1.60 1.60  1.60  1.52 1.60  1.52  1.58 1.53  1.60  1.60 1.47  1.47
HCM2k85thQ:  0.0 0.0  0.0  14.7 0.0  14.7  2.9 11.4  0.0  0.0 26.0  26.0
-----
90thFactor:  1.80 1.80  1.80  1.65 1.80  1.65  1.77 1.68  1.80  1.80 1.56  1.56
HCM2k90thQ:  0.0 0.0  0.0  15.9 0.0  15.9  3.2 12.5  0.0  0.0 27.7  27.7
-----
95thFactor:  2.10 2.10  2.10  1.85 2.10  1.85  2.04 1.90  2.10  2.10 1.72  1.72
HCM2k95thQ:  0.0 0.0  0.0  17.9 0.0  17.9  3.7 14.1  0.0  0.0 30.6  30.6
-----
98thFactor:  2.70 2.70  2.70  2.17 2.70  2.17  2.57 2.26  2.70  2.70 1.96  1.96
HCM2k98thQ:  0.0 0.0  0.0  21.1 0.0  21.1  4.7 16.9  0.0  0.0 34.7  34.7
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  55.4 0.0  11.5  9.7 115  0.0  0.0 198  63.6
-----
Name: year 1995 composite fleet
Fuel Consumption: 64.785 pounds
                  10.495 gallons
Carbon Dioxide:  202.129 pounds
Carbon Monoxide: 15.215 pounds
Hydrocarbons:    2.575 pounds
Nitrogen Oxides: 0.619 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 59.909 pounds
                  9.705 gallons
Carbon Dioxide:  186.917 pounds
Carbon Monoxide: 14.836 pounds
Hydrocarbons:    2.497 pounds
Nitrogen Oxides: 0.546 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj PM

Intersection #18: Cosumnes River Boulevard/Stone-Boswell Access East

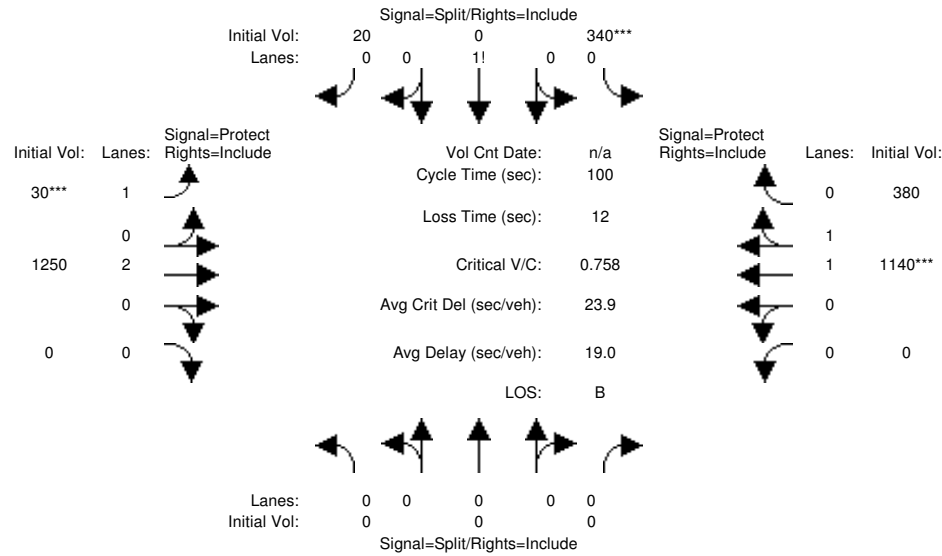


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.26 0.00  0.26  0.05 0.62  0.00  0.00 0.57  0.57
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  9.3 0.0  9.3  0.8 10.8  0.0  0.0 17.3  17.3
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  1.00 1.00  0.00  0.00 1.00  1.00
Q2:           0.0 0.0  0.0  2.9 0.0  2.9  0.5 1.3  0.0  0.0 3.2  3.2
HCM2kQueue:   0.0 0.0  0.0  12.2 0.0  12.2  1.3 12.1  0.0  0.0 20.5  20.5
-----
70thFactor:  1.20 1.20  1.20  1.17 1.20  1.17  1.20 1.17  1.20  1.20 1.16  1.16
HCM2k70thQ:  0.0 0.0  0.0  14.3 0.0  14.3  1.5 14.2  0.0  0.0 23.8  23.8
-----
85thFactor:  1.60 1.60  1.60  1.50 1.60  1.50  1.59 1.50  1.60  1.60 1.45  1.45
HCM2k85thQ:  0.0 0.0  0.0  18.3 0.0  18.3  2.0 18.1  0.0  0.0 29.8  29.8
-----
90thFactor:  1.80 1.80  1.80  1.62 1.80  1.62  1.78 1.62  1.80  1.80 1.54  1.54
HCM2k90thQ:  0.0 0.0  0.0  19.8 0.0  19.8  2.3 19.5  0.0  0.0 31.7  31.7
-----
95thFactor:  2.10 2.10  2.10  1.80 2.10  1.80  2.06 1.81  2.10  2.10 1.69  1.69
HCM2k95thQ:  0.0 0.0  0.0  22.0 0.0  22.0  2.6 21.8  0.0  0.0 34.7  34.7
-----
98thFactor:  2.70 2.70  2.70  2.09 2.70  2.09  2.61 2.10  2.70  2.70 1.91  1.91
HCM2k98thQ:  0.0 0.0  0.0  25.5 0.0  25.5  3.4 25.3  0.0  0.0 39.1  39.1
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #18 Cosumnes River Boulevard/Stone-Boswell Access East
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  0.0 0.0  0.0  79.0 0.0  4.6  7.2 184  0.0  0.0 222  73.8
-----
Name: year 1995 composite fleet
Fuel Consumption:  80.473 pounds
                  13.037 gallons
Carbon Dioxide:   251.076 pounds
Carbon Monoxide:  19.192 pounds
Hydrocarbons:     3.334 pounds
Nitrogen Oxides:  0.758 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  74.811 pounds
                  12.119 gallons
Carbon Dioxide:   233.411 pounds
Carbon Monoxide:  18.750 pounds
Hydrocarbons:     3.244 pounds
Nitrogen Oxides:  0.671 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #20: Delta Shores Circle/Street C

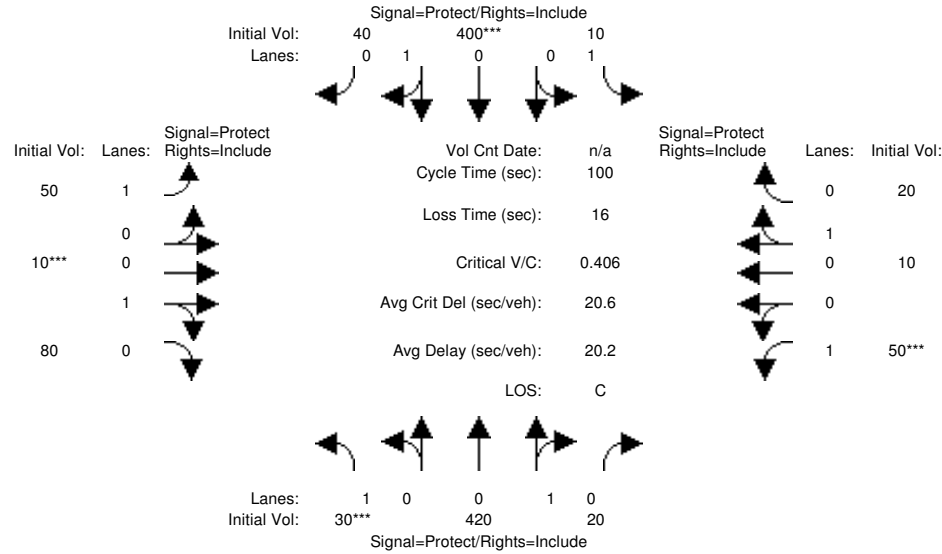


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.05 0.52  0.52  0.11 0.58  0.58  0.09 0.14  0.14  0.07 0.12  0.12
ArrivalType:      3          3          3          3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.8  7.6  7.6  0.2  6.7  6.7  1.3  2.3  2.3  1.3  0.7  0.7
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.5  0.8  0.8  0.1  0.7  0.7  0.5  0.7  0.7  0.6  0.2  0.2
HCM2kQueue:    1.3  8.4  8.4  0.3  7.4  7.4  1.8  3.0  3.0  2.0  0.9  0.9
-----
70thFactor:    1.20 1.18  1.18  1.20 1.18  1.18  1.20 1.19  1.19  1.20 1.20  1.20
HCM2k70thQ:    1.5 10.0 10.0  0.4  8.7  8.7  2.1  3.5  3.5  2.4  1.1  1.1
-----
85thFactor:    1.59 1.53  1.53  1.60 1.53  1.53  1.58 1.57  1.57  1.58 1.59  1.59
HCM2k85thQ:    2.0 12.9 12.9  0.5 11.3 11.3  2.8  4.6  4.6  3.1  1.5  1.5
-----
90thFactor:    1.78 1.66  1.66  1.79 1.68  1.68  1.77 1.75  1.75  1.76 1.78  1.78
HCM2k90thQ:    2.3 14.0 14.0  0.5 12.3 12.3  3.1  5.2  5.2  3.5  1.6  1.6
-----
95thFactor:    2.06 1.88  1.88  2.09 1.90  1.90  2.04 2.01  2.01  2.04 2.07  2.07
HCM2k95thQ:    2.6 15.8 15.8  0.6 14.0 14.0  3.6  5.9  5.9  4.0  1.9  1.9
-----
98thFactor:    2.61 2.22  2.22  2.68 2.27  2.27  2.57 2.50  2.50  2.56 2.63  2.63
HCM2k98thQ:    3.4 18.8 18.8  0.8 16.7 16.7  4.6  7.4  7.4  5.1  2.4  2.4
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #20 Delta Shores Circle/Street C
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    7.2 65.5  3.1  2.2 54.6  5.5  11.8  2.3 18.3  12.0  2.2  4.5
-----
Name: year 1995 composite fleet
Fuel Consumption: 29.763 pounds
                  4.822 gallons
Carbon Dioxide:  92.860 pounds
Carbon Monoxide: 7.142 pounds
Hydrocarbons:   1.262 pounds
Nitrogen Oxides: 0.271 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 27.751 pounds
                  4.496 gallons
Carbon Dioxide:  86.583 pounds
Carbon Monoxide: 6.986 pounds
Hydrocarbons:   1.230 pounds
Nitrogen Oxides: 0.241 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.13 0.58  0.58  0.10 0.55  0.55  0.09 0.09  0.09  0.07 0.07  0.07
ArrivalType:      3          3          3          3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            4.0  8.5  8.5  0.5 12.4 12.4  2.7  2.1  2.1  0.8  1.1  1.1
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            1.6  0.9  0.9  0.1  1.7  1.7  1.5  1.0  1.0  0.3  0.5  0.5
HCM2kQueue:    5.5  9.4  9.4  0.6 14.1 14.1  4.2  3.1  3.1  1.1  1.5  1.5
-----|-----|-----|-----|
70thFactor:    1.19 1.18  1.18  1.20 1.17  1.17  1.19 1.19  1.19  1.20 1.20  1.20
HCM2k70thQ:    6.6 11.1 11.1  0.8 16.5 16.5  5.0  3.7  3.7  1.4  1.8  1.8
-----|-----|-----|-----|
85thFactor:    1.55 1.52  1.52  1.59 1.49  1.49  1.56 1.57  1.57  1.59 1.59  1.59
HCM2k85thQ:    8.6 14.3 14.3  1.0 21.0 21.0  6.5  4.8  4.8  1.8  2.4  2.4
-----|-----|-----|-----|
90thFactor:    1.70 1.65  1.65  1.79 1.60  1.60  1.72 1.74  1.74  1.78 1.77  1.77
HCM2k90thQ:    9.4 15.5 15.5  1.1 22.6 22.6  7.2  5.4  5.4  2.0  2.7  2.7
-----|-----|-----|-----|
95thFactor:    1.94 1.86  1.86  2.08 1.77  1.77  1.98 2.01  2.01  2.06 2.05  2.05
HCM2k95thQ:    10.7 17.4 17.4  1.3 25.1 25.1  8.3  6.2  6.2  2.3  3.1  3.1
-----|-----|-----|-----|
98thFactor:    2.35 2.19  2.19  2.65 2.04  2.04  2.43 2.49  2.49  2.62 2.59  2.59
HCM2k98thQ:    13.0 20.5 20.5  1.7 28.8 28.8  10.1  7.7  7.7  3.0  4.0  4.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #20 Delta Shores Circle/Street C
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    35.6 70.3  5.9  4.5 87.1 24.4 24.2  7.1 11.9  7.1  4.8  4.8
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption:  45.538 pounds
                  7.377 gallons
Carbon Dioxide:   142.078 pounds
Carbon Monoxide:  11.103 pounds
Hydrocarbons:     2.013 pounds
Nitrogen Oxides:  0.405 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption:  42.702 pounds
                  6.918 gallons
Carbon Dioxide:   133.230 pounds
Carbon Monoxide:  10.882 pounds
Hydrocarbons:     1.968 pounds
Nitrogen Oxides:  0.362 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.09 0.17  0.17  0.05 0.13  0.13  0.21 0.49  0.49  0.14 0.42  0.42
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.3 3.8  3.8  0.5 1.0  2.9  4.7 5.7  5.7  1.2 8.1  8.1
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.1 1.0  1.0  0.3 0.2  1.1  1.0 0.6  0.6  0.3 1.1  1.1
HCM2kQueue:   0.3 4.8  4.8  0.8 1.2  4.0  5.7 6.3  6.3  1.5 9.1  9.1
-----
70thFactor:   1.20 1.19  1.19  1.20 1.20  1.19  1.19 1.19  1.19  1.20 1.18  1.18
HCM2k70thQ:   0.4 5.8  5.8  1.0 1.4  4.7  6.8 7.5  7.5  1.8 10.8  10.8
-----
85thFactor:   1.60 1.56  1.56  1.59 1.59  1.56  1.55 1.54  1.54  1.59 1.52  1.52
HCM2k85thQ:   0.5 7.5  7.5  1.3 1.9  6.2  8.9 9.7  9.7  2.4 13.9  13.9
-----
90thFactor:   1.79 1.71  1.71  1.78 1.78  1.73  1.70 1.69  1.69  1.77 1.65  1.65
HCM2k90thQ:   0.6 8.3  8.3  1.5 2.1  6.9  9.8 10.7  10.7  2.6 15.1  15.1
-----
95thFactor:   2.09 1.96  1.96  2.07 2.06  1.98  1.94 1.92  1.92  2.05 1.86  1.86
HCM2k95thQ:   0.7 9.5  9.5  1.7 2.5  7.9  11.1 12.1  12.1  3.1 17.0  17.0
-----
98thFactor:   2.68 2.39  2.39  2.64 2.61  2.44  2.34 2.32  2.32  2.59 2.19  2.19
HCM2k98thQ:   0.9 11.6  11.6  2.2 3.1  9.7  13.5 14.6  14.6  3.9 20.1  20.1
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #21 Delta Shores Circle/Street A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  2.3 16.0  18.3  4.8  8.9  25.8  42.3 50.0  1.6  11.1 52.2  20.5
-----
Name: year 1995 composite fleet
Fuel Consumption:  41.569 pounds
                  6.734 gallons
Carbon Dioxide:   129.696 pounds
Carbon Monoxide:  10.303 pounds
Hydrocarbons:     1.920 pounds
Nitrogen Oxides:  0.359 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  39.219 pounds
                  6.353 gallons
Carbon Dioxide:   122.362 pounds
Carbon Monoxide:  10.119 pounds
Hydrocarbons:     1.883 pounds
Nitrogen Oxides:  0.323 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.05 0.19  0.19  0.11 0.25  0.25  0.18 0.43  0.43  0.11 0.36  0.36
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.3 3.4  3.4  2.1 2.0  6.2  5.1 7.1  7.1  2.3 9.7  9.7
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.1 0.8  0.8  0.6 0.2  1.6  1.6 0.8  0.8  0.8 1.6  1.6
HCM2kQueue:   0.4 4.2  4.2  2.7 2.2  7.8  6.7 7.9  7.9  3.1 11.3  11.3
-----|-----|-----|-----|
70thFactor:   1.20 1.19  1.19  1.19 1.19  1.18  1.18 1.18  1.18  1.19 1.18  1.18
HCM2k70thQ:   0.5 5.0  5.0  3.2 2.6  9.2  7.9 9.4  9.4  3.7 13.3  13.3
-----|-----|-----|-----|
85thFactor:   1.60 1.56  1.56  1.57 1.58  1.53  1.54 1.53  1.53  1.57 1.51  1.51
HCM2k85thQ:   0.6 6.6  6.6  4.3 3.5  11.9  10.3 12.1  12.1  4.9 17.0  17.0
-----|-----|-----|-----|
90thFactor:   1.79 1.72  1.72  1.75 1.76  1.67  1.69 1.67  1.67  1.74 1.63  1.63
HCM2k90thQ:   0.7 7.3  7.3  4.7 3.9  13.0  11.3 13.2  13.2  5.4 18.4  18.4
-----|-----|-----|-----|
95thFactor:   2.09 1.97  1.97  2.02 2.03  1.89  1.91 1.89  1.89  2.00 1.82  1.82
HCM2k95thQ:   0.8 8.3  8.3  5.4 4.5  14.7  12.8 14.9  14.9  6.3 20.5  20.5
-----|-----|-----|-----|
98thFactor:   2.67 2.42  2.42  2.51 2.54  2.25  2.30 2.24  2.24  2.49 2.12  2.12
HCM2k98thQ:   1.0 10.2  10.2  6.8 5.6  17.5  15.4 17.8  17.8  7.8 23.9  23.9
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #21 Delta Shores Circle/Street A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   2.4 13.3  17.7  18.6 17.7  55.7  46.3 62.1  1.8  21.0 80.7  6.2
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 57.121 pounds
                  9.254 gallons
Carbon Dioxide:  178.217 pounds
Carbon Monoxide: 14.301 pounds
Hydrocarbons:    2.708 pounds
Nitrogen Oxides: 0.486 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 54.093 pounds
                  8.763 gallons
Carbon Dioxide:  168.770 pounds
Carbon Monoxide: 14.063 pounds
Hydrocarbons:    2.660 pounds
Nitrogen Oxides: 0.437 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #22: Delta Shores Circle North/24th Street

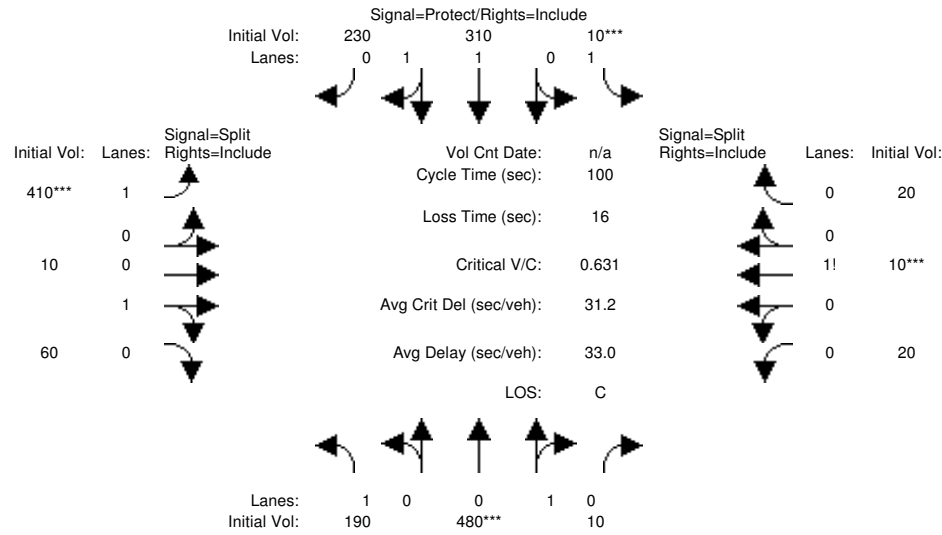


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.17 0.38  0.38  0.05 0.26  0.26  0.34 0.34  0.34  0.07 0.07  0.07
ArrivalType:      3          3          3          3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           4.9 11.4 11.4  0.3  7.0  7.0  9.8  1.3  1.3  1.3  1.3  1.3
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.5  2.1  2.1  0.1  1.5  1.5  2.0  0.1  0.1  0.7  0.7  0.7
HCM2kQueue:    6.4 13.5 13.5  0.4  8.5  8.5 11.9  1.5  1.5  2.0  2.0  2.0
-----|-----|-----|-----|
70thFactor:    1.19 1.17  1.17  1.20 1.18  1.18  1.17 1.20  1.20  1.20 1.20  1.20
HCM2k70thQ:    7.6 15.8 15.8  0.5 10.1 10.1 13.9  1.8  1.8  2.4  2.4  2.4
-----|-----|-----|-----|
85thFactor:    1.54 1.49  1.49  1.60 1.53  1.53  1.50 1.59  1.59  1.58 1.58  1.58
HCM2k85thQ:    9.9 20.1 20.1  0.6 13.0 13.0 17.8  2.4  2.4  3.1  3.1  3.1
-----|-----|-----|-----|
90thFactor:    1.69 1.60  1.60  1.79 1.66  1.66  1.62 1.77  1.77  1.76 1.76  1.76
HCM2k90thQ:    10.8 21.6 21.6  0.7 14.1 14.1 19.2  2.6  2.6  3.5  3.5  3.5
-----|-----|-----|-----|
95thFactor:    1.92 1.78  1.78  2.09 1.87  1.87  1.81 2.05  2.05  2.04 2.04  2.04
HCM2k95thQ:    12.3 24.0 24.0  0.8 16.0 16.0 21.5  3.1  3.1  4.0  4.0  4.0
-----|-----|-----|-----|
98thFactor:    2.31 2.06  2.06  2.67 2.22  2.22  2.10 2.59  2.59  2.56 2.56  2.56
HCM2k98thQ:    14.8 27.7 27.7  1.0 18.9 18.9 24.9  3.9  3.9  5.1  5.1  5.1
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

 Intersection #22 Delta Shores Circle North/24th Street

```

Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  44.1 101  2.1  2.4 68.4 50.8 88.5  1.7 10.4  4.8  2.4  4.8
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 62.561 pounds
                  10.135 gallons
Carbon Dioxide:  195.190 pounds
Carbon Monoxide: 15.699 pounds
Hydrocarbons:    2.981 pounds
Nitrogen Oxides: 0.532 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 59.289 pounds
                  9.605 gallons
Carbon Dioxide:  184.981 pounds
Carbon Monoxide: 15.441 pounds
Hydrocarbons:    2.929 pounds
Nitrogen Oxides: 0.479 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj PM

Intersection #22: Delta Shores Circle North/24th Street

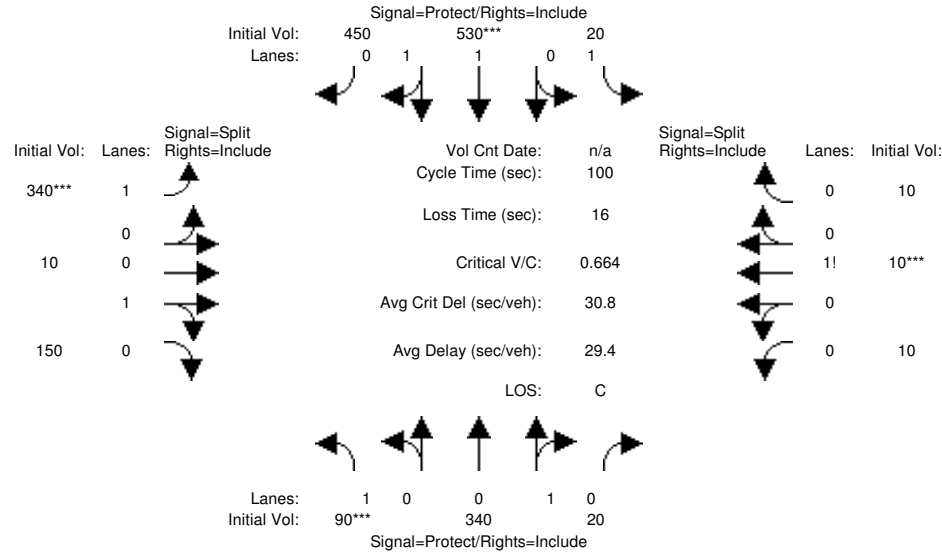


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.07 0.39  0.39  0.10 0.42  0.42  0.27 0.27  0.27  0.07 0.07  0.07
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           2.4  7.5  7.5  0.5 11.8 11.8  8.5  3.6  3.6  0.8  0.8  0.8
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           1.7  1.0  1.0  0.1  2.2  2.2  2.1  0.6  0.6  0.3  0.3  0.3
HCM2kQueue:   4.2  8.5  8.5  0.6 13.9 13.9 10.6  4.2  4.2  1.1  1.1  1.1
-----|-----|-----|-----|
70thFactor:   1.19 1.18  1.18  1.20 1.17  1.17  1.18 1.19  1.19  1.20 1.20  1.20
HCM2k70thQ:   5.0 10.0 10.0  0.8 16.3 16.3 12.5  4.9  4.9  1.3  1.3  1.3
-----|-----|-----|-----|
85thFactor:   1.56 1.53  1.53  1.59 1.49  1.49  1.51 1.56  1.56  1.59 1.59  1.59
HCM2k85thQ:   6.5 12.9 12.9  1.0 20.7 20.7 16.0  6.5  6.5  1.8  1.8  1.8
-----|-----|-----|-----|
90thFactor:   1.72 1.66  1.66  1.79 1.60  1.60  1.64 1.72  1.72  1.78 1.78  1.78
HCM2k90thQ:   7.2 14.1 14.1  1.1 22.3 22.3 17.3  7.2  7.2  2.0  2.0  2.0
-----|-----|-----|-----|
95thFactor:   1.98 1.87  1.87  2.08 1.78  1.78  1.83 1.98  1.98  2.06 2.06  2.06
HCM2k95thQ:   8.2 15.9 15.9  1.3 24.8 24.8 19.4  8.2  8.2  2.3  2.3  2.3
-----|-----|-----|-----|
98thFactor:   2.43 2.22  2.22  2.65 2.04  2.04  2.14 2.43  2.43  2.62 2.62  2.62
HCM2k98thQ:  10.1 18.8 18.8  1.7 28.5 28.5 22.7 10.1 10.1  2.9  2.9  2.9
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

 Intersection #22 Delta Shores Circle North/24th Street

```

Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  22.0 63.9  3.8  4.5 109 92.3  76.4  2.0 30.3  2.4  2.4  2.4
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption:  65.312 pounds
                  10.581 gallons
Carbon Dioxide:   203.775 pounds
Carbon Monoxide:  16.230 pounds
Hydrocarbons:     3.032 pounds
Nitrogen Oxides:  0.567 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption:  61.664 pounds
                  9.990 gallons
Carbon Dioxide:   192.392 pounds
Carbon Monoxide:  15.943 pounds
Hydrocarbons:     2.974 pounds
Nitrogen Oxides:  0.509 pounds
    
```

DISCLAIMER
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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.12 0.28  0.28  0.08 0.24  0.24  0.05 0.28  0.28  0.20 0.43  0.43
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00 1.00
Q1:            0.5 4.1  4.1  1.3 4.2  4.2  0.8 0.8  0.8  1.1 4.7  4.7
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00 1.00
Q2:           0.1 0.6  0.6  0.5 0.7  0.7  0.5 0.1  0.1  0.2 0.6  0.6
HCM2kQueue:    0.6 4.6  4.6  1.9 4.9  4.9  1.3 0.9  0.9  1.3 5.2  5.2
-----
70thFactor:    1.20 1.19  1.19  1.20 1.19  1.19  1.20 1.20  1.20  1.20 1.19  1.19
HCM2k70thQ:    0.7 5.5  5.5  2.2 5.8  5.8  1.5 1.1  1.1  1.6 6.2  6.2
-----
85thFactor:    1.59 1.56  1.56  1.58 1.56  1.56  1.59 1.59  1.59  1.59 1.55  1.55
HCM2k85thQ:    0.9 7.2  7.2  2.9 7.6  7.6  2.0 1.4  1.4  2.1 8.1  8.1
-----
90thFactor:    1.79 1.72  1.72  1.76 1.71  1.71  1.78 1.78  1.78  1.77 1.71  1.71
HCM2k90thQ:    1.1 8.0  8.0  3.3 8.3  8.3  2.3 1.6  1.6  2.3 8.9  8.9
-----
95thFactor:    2.08 1.96  1.96  2.04 1.96  1.96  2.06 2.07  2.07  2.06 1.95  1.95
HCM2k95thQ:    1.2 9.1  9.1  3.8 9.5  9.5  2.6 1.9  1.9  2.7 10.2  10.2
-----
98thFactor:    2.66 2.40  2.40  2.57 2.39  2.39  2.61 2.63  2.63  2.60 2.37  2.37
HCM2k98thQ:    1.6 11.1  11.1  4.8 11.6  11.6  3.4 2.4  2.4  3.4 12.4  12.4
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #23 Street C/ 24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    4.4 60.0  10.0  11.9 69.7  2.1  7.2 3.7  3.7  10.3 5.1  37.1
-----
Name: year 1995 composite fleet
Fuel Consumption:  37.183 pounds
                  6.024 gallons
Carbon Dioxide:   116.010 pounds
Carbon Monoxide:   9.242 pounds
Hydrocarbons:     1.730 pounds
Nitrogen Oxides:  0.320 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  35.118 pounds
                  5.689 gallons
Carbon Dioxide:   109.569 pounds
Carbon Monoxide:   9.080 pounds
Hydrocarbons:     1.698 pounds
Nitrogen Oxides:  0.287 pounds
    
```

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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.16 0.26  0.26  0.25 0.36  0.36  0.05 0.19  0.19  0.13 0.27  0.27
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.7 4.6  4.6  4.4 4.2  4.2  0.5 2.6  2.6  1.8 4.3  4.3
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.1 0.7  0.7  0.7 0.5  0.5  0.3 0.5  0.5  0.4 0.7  0.7
HCM2KQueue:   0.8 5.3  5.3  5.1 4.7  4.7  0.8 3.1  3.1  2.2 5.1  5.1
-----
70thFactor:   1.20 1.19  1.19  1.19 1.19  1.19  1.20 1.19  1.19  1.19 1.19  1.19
HCM2k70thQ:   1.0 6.3  6.3  6.1 5.6  5.6  1.0 3.7  3.7  2.6 6.0  6.0
-----
85thFactor:   1.59 1.55  1.55  1.55 1.56  1.56  1.59 1.57  1.57  1.58 1.55  1.55
HCM2k85thQ:   1.3 8.3  8.3  8.0 7.3  7.3  1.3 4.9  4.9  3.4 7.9  7.9
-----
90thFactor:   1.78 1.71  1.71  1.71 1.72  1.72  1.78 1.74  1.74  1.76 1.71  1.71
HCM2k90thQ:   1.5 9.1  9.1  8.8 8.0  8.0  1.5 5.5  5.5  3.8 8.7  8.7
-----
95thFactor:   2.07 1.95  1.95  1.95 1.96  1.96  2.07 2.00  2.00  2.03 1.95  1.95
HCM2k95thQ:   1.7 10.4 10.4 10.0 9.2  9.2  1.7 6.3  6.3  4.4 9.9  9.9
-----
98thFactor:   2.64 2.36  2.36  2.37 2.40  2.40  2.64 2.49  2.49  2.55 2.38  2.38
HCM2k98thQ:   2.2 12.6 12.6 12.2 11.2 11.2  2.2 7.8  7.8  5.5 12.0 12.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #23 Street C/ 24th Street
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   6.4 58.0  20.7 39.7 68.7  3.6  4.8 15.2  8.7 15.8  8.2 30.8
-----
Name: year 1995 composite fleet
Fuel Consumption: 46.253 pounds
                  7.493 gallons
Carbon Dioxide:  144.310 pounds
Carbon Monoxide: 11.513 pounds
Hydrocarbons:    2.160 pounds
Nitrogen Oxides: 0.397 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 43.707 pounds
                  7.081 gallons
Carbon Dioxide:  136.365 pounds
Carbon Monoxide: 11.313 pounds
Hydrocarbons:    2.120 pounds
Nitrogen Oxides: 0.357 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Cum w/proj AM

Intersection #24: Street A/Street C

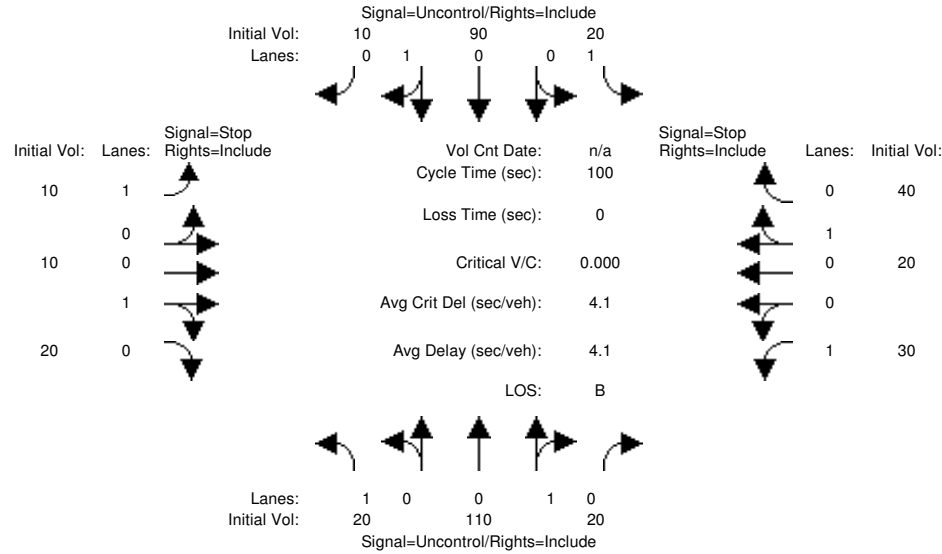


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #24 Street A/Street C

Table showing Base Volume Alternative: Peak Hour Warrant NOT Met. Columns for Approach, Movement, Control, Lanes, Initial Vol, and ApproachDel.

Approach[eastbound] [lanes=2] [control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 5 for two or more lane approach.

Approach[westbound] [lanes=2] [control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.3]
FAIL - Vehicle-hours less than 5 for two or more lane approach.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	20 110 20	20 90 10	10 10 20	30 20 40

Major Street Volume: 270
Minor Approach Volume: 90
Minor Approach Volume Threshold: 937

SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj PM

Intersection #24: Street A/Street C

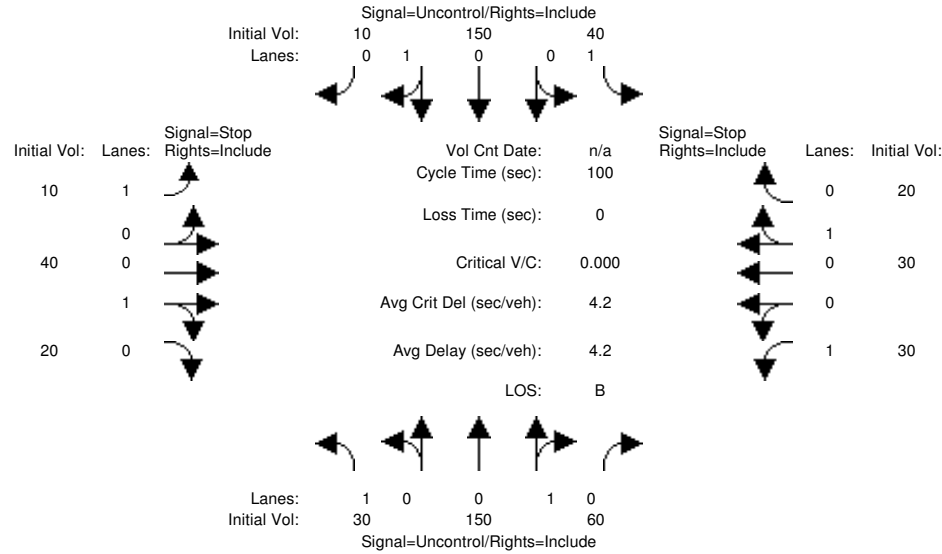


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #24 Street A/Street C
Base Volume Alternative: Peak Hour Warrant NOT Met

Table showing warrant analysis for Eastbound and Westbound approaches. Columns include Approach, Control, Lanes, Initial Vol, and Approach Del. Includes failure reasons for Signal Warrant Rules #1, #2, and #3.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #24 Street A/Street C

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	30 150 60	40 150 10	10 40 20	30 30 20

Major Street Volume: 440
Minor Approach Volume: 80
Minor Approach Volume Threshold: 727

SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj AM

Intersection #25: Street C/Street B

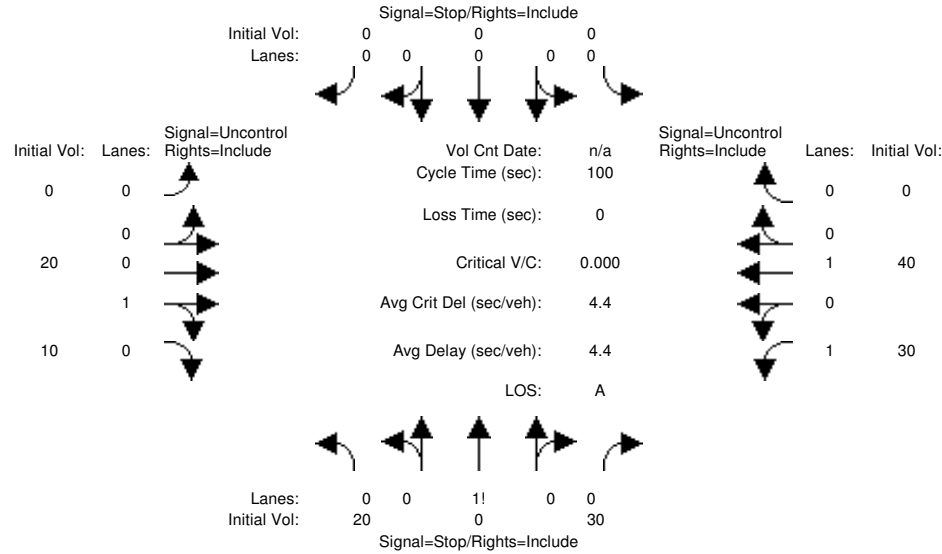


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #25 Street C/Street B
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control (Stop Sign, Uncontrolled), Lanes, Initial Vol, and ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.1] FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=50] FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=150] FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	1	0	0	0	0	0	0	0	1	0	1	0	1	0
Initial Vol:	20	0	30		0	0	0		0	20	10		30	40	0	

Major Street Volume: 100
 Minor Approach Volume: 50
 Minor Approach Volume Threshold: 1078

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj PM

Intersection #25: Street C/Street B

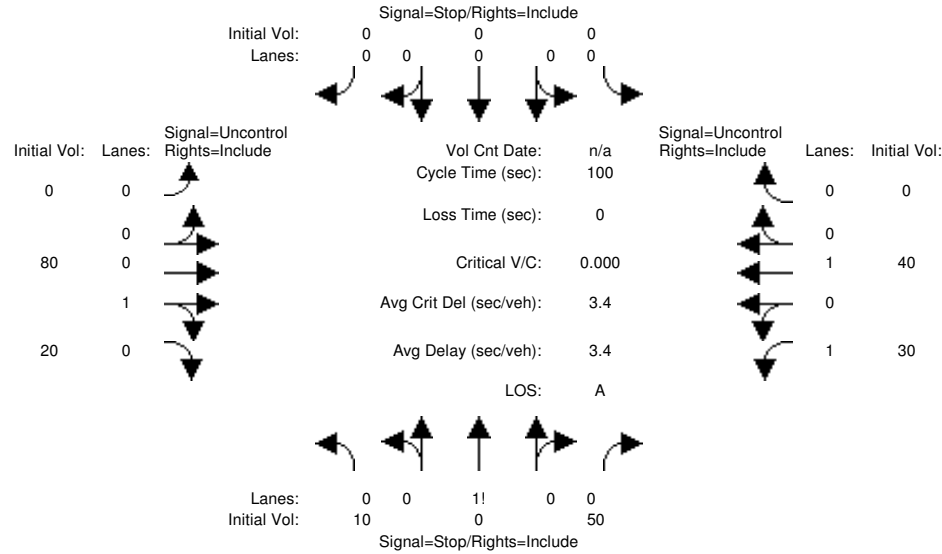


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #25 Street C/Street B
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.2]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=60]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=230]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Peak Hour Volume Signal Warrant Report [Urban]
Intersection #25 Street C/Street B

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	1	0	0	0	0	0	0	0	0	1	0	1	0	0
Initial Vol:	10	0	50		0	0	0		0	80	20		30	40	0	

Major Street Volume: 170
 Minor Approach Volume: 60
 Minor Approach Volume Threshold: 895

SIGNAL WARRANT DISCLAIMER

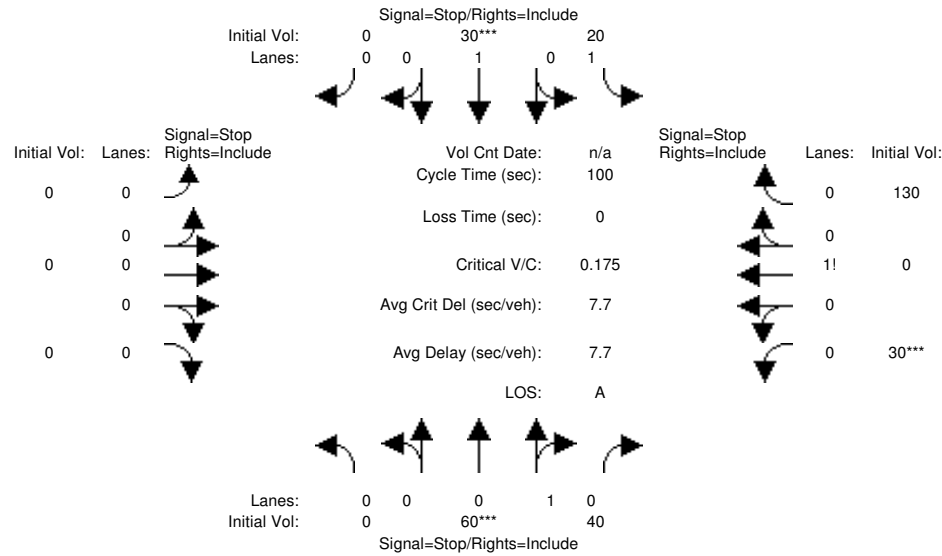
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shore

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Cum w/proj AM

Intersection #26: Street D/Street E



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:												
Base Vol:	0	60	40	20	30	0	0	0	0	30	0	130
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	60	40	20	30	0	0	0	0	30	0	130
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	60	40	20	30	0	0	0	0	30	0	130
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	60	40	20	30	0	0	0	0	30	0	130
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	60	40	20	30	0	0	0	0	30	0	130
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	0.60	0.40	1.00	1.00	0.00	0.00	0.00	0.00	0.19	0.00	0.81
Final Sat.:	0	501	334	646	710	0	0	0	0	172	0	745

Capacity Analysis Module:												
Vol/Sat:	xxxx	0.12	0.12	0.03	0.04	xxxx	xxxx	xxxx	xxxx	0.17	xxxx	0.17
Crit Moves:	****											
Delay/Veh:	0.0	7.7	7.7	8.3	7.8	0.0	0.0	0.0	0.0	7.6	0.0	7.6
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	7.7	7.7	8.3	7.8	0.0	0.0	0.0	0.0	7.6	0.0	7.6
LOS by Move:	*	A	A	A	A	*	*	*	*	A	*	A
ApproachDel:	7.7			8.0			xxxxxx			7.6		
Delay Adj:	1.00			1.00			xxxxxx			1.00		
ApprAdjDel:	7.7			8.0			xxxxxx			7.6		
LOS by Appr:	A			A			*			A		
AllWayAvgQ:	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.2	0.2

Time Period: 0.25 hour
 HevVeh: 2% 2% 2% 2%
 Alpha Value: 0.01

GroupType:	3A			5			X			2		
P[C1]:	0.7728			0.7348			x.xxxxx			0.8228		
P[C2]:	0.0579			0.0960			x.xxxxx			0.0000		
P[C3]:	0.1575			0.1497			x.xxxxx			0.1691		
P[C4]:	0.0118			0.0196			x.xxxxx			0.0081		
P[C5]:	0.0000			0.0000			x.xxxxx			0.0000		
Padj[C1]:	0.00408			0.00454			xx.xxxxx			0.00362		
Padj[C2]:	0.00123			0.00093			xx.xxxxx			0.00185		
Padj[C3]:	-0.00461			-0.00430			xx.xxxxx			-0.00499		
Padj[C4]:	-0.00071			-0.00117			xx.xxxxx			-0.00048		
Padj[C5]:	0.00000			0.00000			xx.xxxxx			0.00000		

Lanes:	L1		L2		L1		L2		L1		L2	
LaneType:	LTR	NOLANE	LEFT	THRU	NOLANE	NOLANE	LTR	NOLANE	LTR	NOLANE	LTR	NOLANE
HeadwayAdj:	-0.206	xx.xxx	0.534	0.034	xx.xxx	xx.xxx	-0.416	xx.xxx				
Volume:	100	xxxxxx	20	30	xxxxxx	xxxxxx	160	xxxxxx				
Capacity:	836	xxxxxx	646	710	xxxxxx	xxxxxx	917	xxxxxx				
DegOfUtil:	0.12	x.xx	0.03	0.04	x.xx	x.xx	0.17	x.xx				
DepHeadway:	4.17	xx.xx	5.42	4.92	xx.xx	xx.xx	3.81	xx.xx				
ServiceTime:	2.2	xx.x	3.1	2.6	xx.x	xx.x	1.8	xx.x				
Delay:	7.7	xxx.x	8.3	7.8	xxx.x	xxx.x	7.6	xxx.x				
Queue:	0.1	xxx.x	0.0	0.0	xxx.x	xxx.x	0.2	xxx.x				

Approach:	North Bound			South Bound			East Bound			West Bound		
ApproachDel:	7.7			8.0			xxx.x			7.6		
Delay Adj:	1.00			1.00			x.xx			1.00		
ApprAdjDel:	7.7			8.0			xxx.x			7.6		
LOS by Appr:	A			A			*			A		
OverallDel:	7.7											
OverallLOS:	A											

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #26 Street D/Street E

 Base Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound

```

Movement:   L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Stop Sign   Stop Sign   Stop Sign   Stop Sign
Lanes:      0 0 0 1 0   1 0 1 0 0   0 0 0 0 0   0 0 1 0 0
Initial Vol: 0 60 40   20 30 0   0 0 0 0 0   30 0 130
-----|-----|-----|-----|
Major Street Volume:      160
Minor Approach Volume:    100
Minor Approach Volume Threshold: 708
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

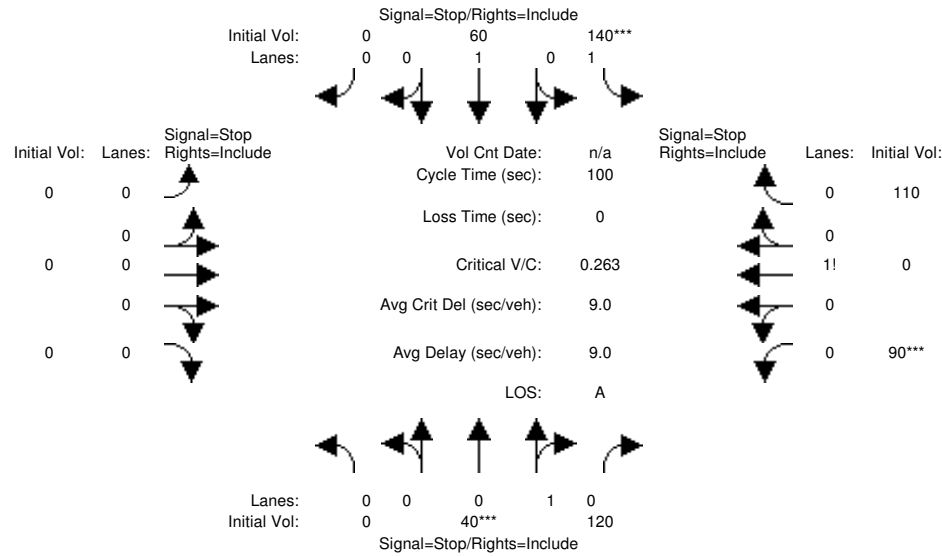
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Delta Shore

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Cum w/proj PM

Intersection #26: Street D/Street E



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:												
Base Vol:	0	40	120	140	60	0	0	0	0	90	0	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	40	120	140	60	0	0	0	0	90	0	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	40	120	140	60	0	0	0	0	90	0	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	40	120	140	60	0	0	0	0	90	0	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	40	120	140	60	0	0	0	0	90	0	110
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	0.25	0.75	1.00	1.00	0.00	0.00	0.00	0.00	0.45	0.00	0.55
Final Sat.:	0	202	605	623	682	0	0	0	0	342	0	418
Capacity Analysis Module:												
Vol/Sat:	xxxx	0.20	0.20	0.22	0.09	xxxx	xxxx	xxxx	xxxx	0.26	xxxx	0.26
Crit Moves:	****											
Delay/Veh:	0.0	8.3	8.3	9.9	8.3	0.0	0.0	0.0	0.0	9.0	0.0	9.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	8.3	8.3	9.9	8.3	0.0	0.0	0.0	0.0	9.0	0.0	9.0
LOS by Move:	A	A	A	A	A	*	*	*	*	A	*	A
ApproachDel:	8.3			9.4			xxxxxx			9.0		
Delay Adj:	1.00			1.00			xxxxxx			1.00		
ApprAdjDel:	8.3			9.4			xxxxxx			9.0		
LOS by Appr:	A			A			*			A		
AllWayAvgQ:	0.2	0.2	0.2	0.3	0.1	0.0	0.0	0.0	0.0	0.3	0.3	0.3
Time Period:	0.25 hour											
HevVeh:	2%											
Alpha Value:	0.01											
GroupType:	3A			5			X			2		
P[C1]:	0.5359			0.6066			x.xxxxx			0.5786		
P[C2]:	0.2137			0.1430			x.xxxxx			0.0000		
P[C3]:	0.1790			0.2027			x.xxxxx			0.3670		
P[C4]:	0.0714			0.0478			x.xxxxx			0.0544		
P[C5]:	0.0000			0.0000			x.xxxxx			0.0000		
Padj[C1]:	0.00786			0.00692			xx.xxxxxx			0.00897		
Padj[C2]:	0.00108			0.00155			xx.xxxxxx			0.00476		
Padj[C3]:	-0.00466			-0.00560			xx.xxxxxx			-0.01047		
Padj[C4]:	-0.00428			-0.00287			xx.xxxxxx			-0.00326		
Padj[C5]:	0.00000			0.00000			xx.xxxxxx			0.00000		
Lanes:	L1	L2		L1	L2		L1	L2		L1	L2	
LaneType:	LTR	NOLANE		LEFT	THRU		NOLANE	NOLANE		LTR	NOLANE	
HeadwayAdj:	-0.416	xx.xxx		0.534	0.034		xx.xxx	xx.xxx		-0.206	xx.xxx	
Volume:	160	xxxxxx		140	60		xxxxxx	xxxxxx		200	xxxxxx	
Capacity:	806	xxxxxx		623	682		xxxxxx	xxxxxx		759	xxxxxx	
DegOfUtil:	0.19	x.xx		0.22	0.09		x.xx	x.xx		0.25	x.xx	
DegHeadway:	4.30	xx.xx		5.62	5.12		xx.xx	xx.xx		4.51	xx.xx	
ServiceTime:	2.3	xx.x		3.3	2.9		xx.x	xx.x		2.5	xx.x	
Delay:	8.3	xxx.x		9.9	8.3		xxx.x	xxx.x		9.0	xxx.x	
Queue:	0.2	xxx.x		0.3	0.1		xxx.x	xxx.x		0.3	xxx.x	
Approach:	North Bound			South Bound			East Bound			West Bound		
ApproachDel:	8.3			9.4			xxx.x			9.0		
Delay Adj:	1.00			1.00			x.xx			1.00		
ApprAdjDel:	8.3			9.4			xxx.x			9.0		
LOS by Appr:	A			A			*			A		
OverallDel:	9.0											
OverallLOS:	A											

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #26 Street D/Street E

 Base Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound

```

Movement:   L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Stop Sign   Stop Sign   Stop Sign   Stop Sign
Lanes:      0 0 0 1 0   1 0 1 0 0   0 0 0 0 0   0 0 1 0 0
Initial Vol: 0 40 120 140 60 0   0 0 0 0 0   0 0 0 0 0   90 0 110
-----|-----|-----|-----|
Major Street Volume:          360
Minor Approach Volume:       200
Minor Approach Volume Threshold: 637
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj AM

Intersection #27: Street E/Street G

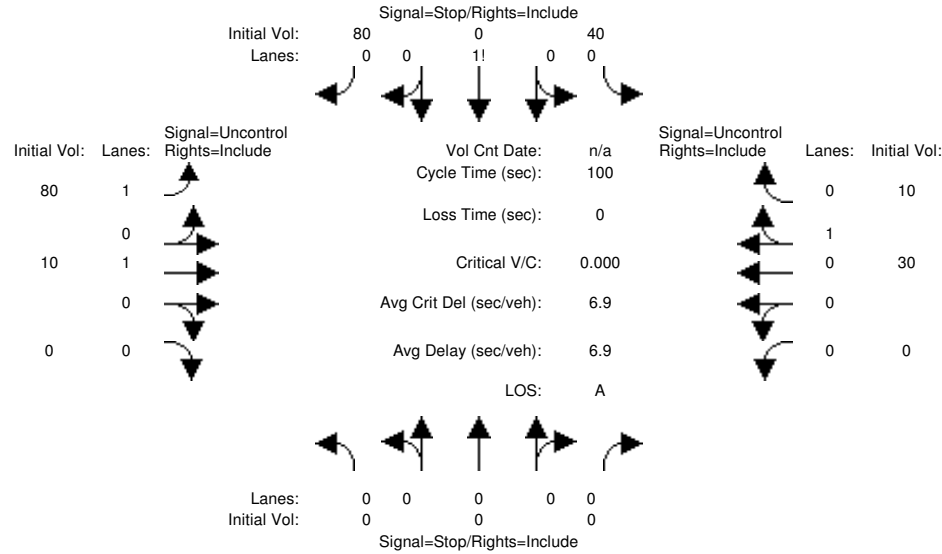


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #27 Street E/Street G
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Lanes, Initial Vol, and ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.3]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=120]
SUCCEEDED - Approach volume greater than or equal to 100 for one lane approach.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]
Intersection #27 Street E/Street G

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	0	0	0	0	1	0	1	0	1	0	0	0	0	1
Initial Vol:	0	0	0	0	40	0	80	80	80	10	0	0	0	30	10	0

Major Street Volume: 130
 Minor Approach Volume: 120
 Minor Approach Volume Threshold: 988

SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj PM

Intersection #27: Street E/Street G

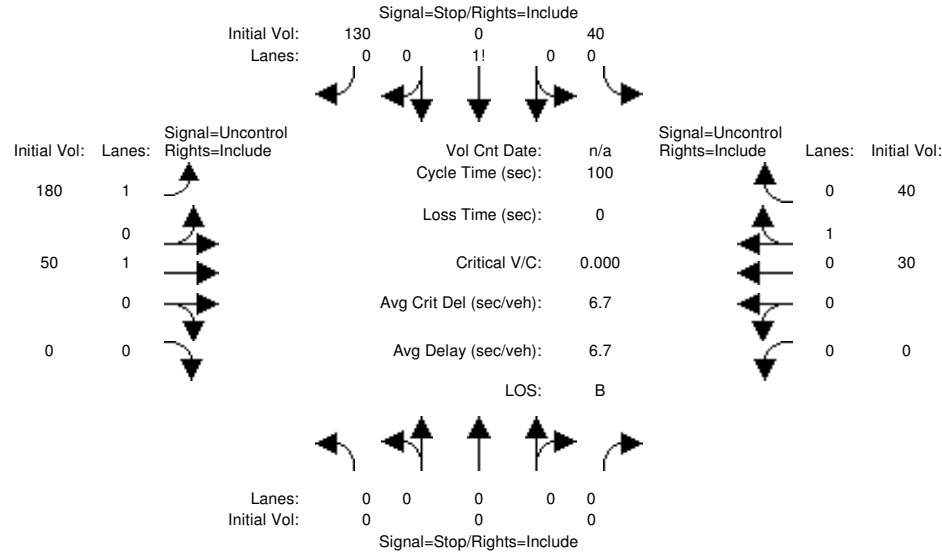


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #27 Street E/Street G
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Lanes, Initial Vol, and ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=170]
SUCCEEDED - Approach volume greater than or equal to 100 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

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Peak Hour Volume Signal Warrant Report [Urban]
Intersection #27 Street E/Street G

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	0	0	0	0	1	0	1	0	1	0	0	0	0	1
Initial Vol:	0	0	0	0	40	0	130		180	50	0	0	0	0	30	40
Major Street Volume:	300															
Minor Approach Volume:	170															
Minor Approach Volume Threshold:	700															

SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj AM

Intersection #28: Delta Shores Circle South/Street D South

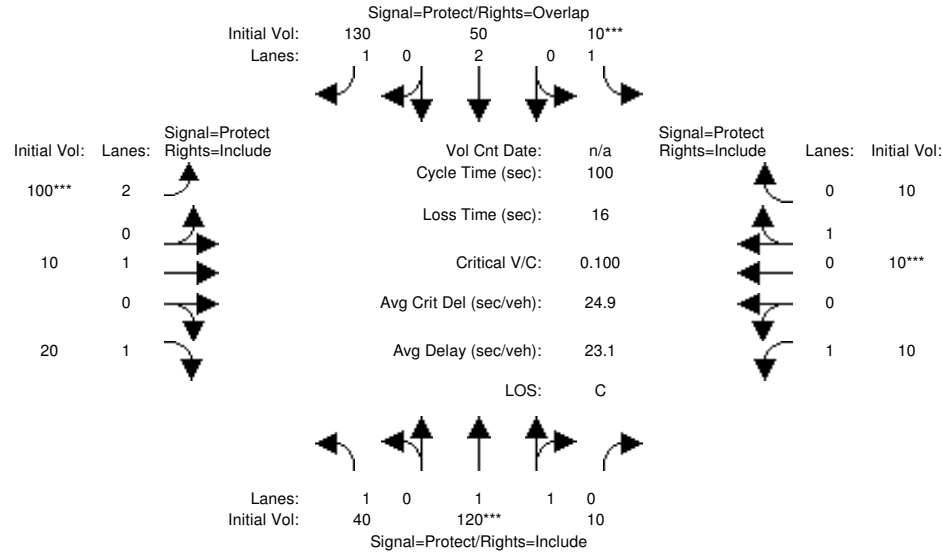


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.18 0.37  0.37  0.06 0.25  0.54  0.29 0.24  0.24  0.17 0.12  0.12
ArrivalType:  3              3              3              3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.9  1.2  1.2  0.3  0.6  1.8  1.0  0.2  0.4  0.2  0.5  0.5
UpstreamVC:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj: 0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj: 1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:           0.1  0.1  0.1  0.1  0.1  0.2  0.1  0.0  0.1  0.0  0.1  0.1
HCM2kQueue:  1.1  1.3  1.3  0.4  0.6  2.0  1.2  0.2  0.5  0.3  0.6  0.6
-----
70thFactor:  1.20 1.20  1.20  1.20 1.20  1.20  1.20 1.20  1.20  1.20 1.20  1.20
HCM2k70thQ:  1.3  1.6  1.6  0.4  0.7  2.4  1.4  0.3  0.6  0.3  0.7  0.7
-----
85thFactor:  1.59 1.59  1.59  1.60 1.59  1.58  1.59 1.60  1.60  1.60 1.59  1.59
HCM2k85thQ:  1.7  2.1  2.1  0.6  1.0  3.1  1.8  0.4  0.8  0.4  1.0  1.0
-----
90thFactor:  1.78 1.77  1.77  1.79 1.79  1.76  1.78 1.80  1.79  1.79 1.79  1.79
HCM2k90thQ:  1.9  2.4  2.4  0.7  1.1  3.5  2.1  0.4  0.9  0.5  1.1  1.1
-----
95thFactor:  2.07 2.06  2.06  2.09 2.08  2.04  2.06 2.09  2.08  2.09 2.08  2.08
HCM2k95thQ:  2.2  2.8  2.8  0.8  1.3  4.0  2.4  0.5  1.0  0.6  1.3  1.3
-----
98thFactor:  2.62 2.60  2.60  2.67 2.65  2.56  2.62 2.68  2.66  2.68 2.65  2.65
HCM2k98thQ:  2.8  3.5  3.5  1.0  1.6  5.0  3.0  0.6  1.3  0.7  1.6  1.6
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #28 Delta Shores Circle South/Street D South
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  8.4 19.5  1.6  2.4  9.5 16.1  18.2 1.9  3.9  2.1  2.2  2.2
-----
Name: year 1995 composite fleet
Fuel Consumption:  14.623 pounds
                  2.369 gallons
Carbon Dioxide:   45.624 pounds
Carbon Monoxide:  3.555 pounds
Hydrocarbons:     0.643 pounds
Nitrogen Oxides:  0.129 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  13.702 pounds
                  2.220 gallons
Carbon Dioxide:   42.751 pounds
Carbon Monoxide:  3.483 pounds
Hydrocarbons:     0.628 pounds
Nitrogen Oxides:  0.115 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Delta Shore

Level Of Service Computation Report
2000 HCM Operations (Base Volume Alternative)
Cum w/proj PM

Intersection #28: Delta Shores Circle South/Street D South

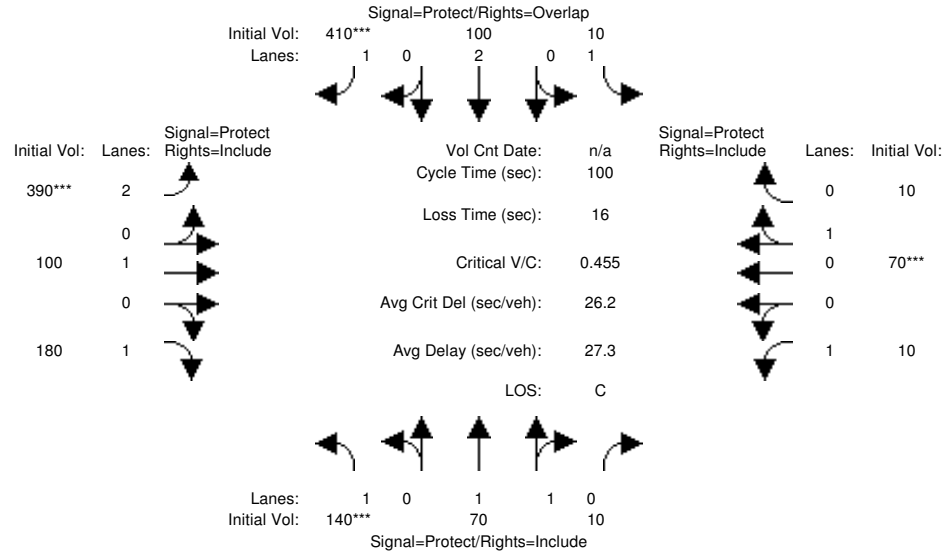


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.17 0.29  0.29  0.21 0.32  0.57  0.25 0.24  0.24  0.11 0.10  0.10
ArrivalType:      3              3              3              3
ProgFactor:    1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:            3.5 0.9  0.9  0.2 1.0  6.6  4.7 2.2  4.3  0.2 2.1  2.1
UpstreamVC:    0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q2:            0.8 0.1  0.1  0.0 0.1  0.8  0.8 0.3  0.9  0.1 0.8  0.8
HCM2kQueue:    4.3 0.9  0.9  0.3 1.1  7.4  5.5 2.5  5.2  0.3 2.9  2.9
-----
70thFactor:    1.19 1.20  1.20  1.20 1.20  1.18  1.19 1.19  1.19  1.20 1.19  1.19
HCM2k70thQ:    5.1 1.1  1.1  0.3 1.3  8.8  6.6 3.0  6.1  0.4 3.4  3.4
-----
85thFactor:    1.56 1.59  1.59  1.60 1.59  1.53  1.55 1.58  1.55  1.60 1.57  1.57
HCM2k85thQ:    6.7 1.5  1.5  0.4 1.8  11.4  8.6 4.0  8.0  0.5 4.5  4.5
-----
90thFactor:    1.72 1.78  1.78  1.80 1.78  1.68  1.70 1.75  1.71  1.79 1.75  1.75
HCM2k90thQ:    7.4 1.7  1.7  0.4 2.0  12.5  9.4 4.4  8.8  0.5 5.0  5.0
-----
95thFactor:    1.97 2.07  2.07  2.09 2.06  1.90  1.94 2.02  1.95  2.09 2.01  2.01
HCM2k95thQ:    8.5 1.9  1.9  0.5 2.3  14.1  10.7 5.1  10.0  0.6 5.8  5.8
-----
98thFactor:    2.42 2.63  2.63  2.68 2.62  2.26  2.35 2.52  2.37  2.68 2.50  2.50
HCM2k98thQ:    10.4 2.5  2.5  0.7 2.9  16.8  13.0 6.3  12.2  0.8 7.2  7.2
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

 Intersection #28 Delta Shores Circle South/Street D South

```

Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:      30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:    31.4 12.8  1.8  2.0 17.5  59.5  82.5 20.1  38.6  2.2 16.5  2.4
    
```

```

Name: year 1995 composite fleet
Fuel Consumption:  46.999 pounds
                  7.614 gallons
Carbon Dioxide:   146.636 pounds
Carbon Monoxide:  11.604 pounds
Hydrocarbons:    2.150 pounds
Nitrogen Oxides:  0.409 pounds
    
```

```

Name: year 2000 composite fleet
Fuel Consumption:  44.280 pounds
                  7.173 gallons
Carbon Dioxide:   138.154 pounds
Carbon Monoxide:  11.391 pounds
Hydrocarbons:    2.106 pounds
Nitrogen Oxides:  0.366 pounds
    
```

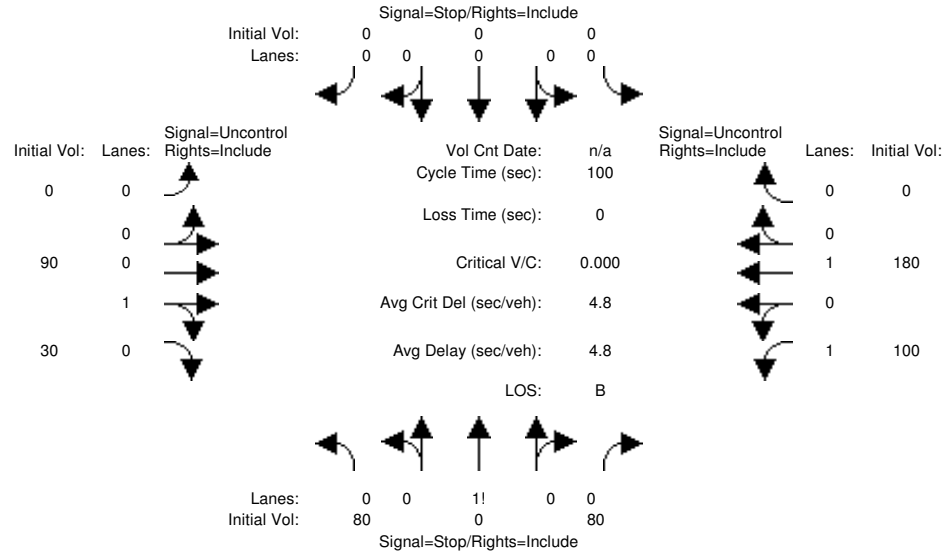
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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj AM

Intersection #29: Street C/Stone-Boswell Access West



Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Volume Module:				
Base Vol:	80 0 80	0 0 0	0 90 30	100 180 0
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	80 0 80	0 0 0	0 90 30	100 180 0
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Volume:	80 0 80	0 0 0	0 90 30	100 180 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Final Volume:	80 0 80	0 0 0	0 90 30	100 180 0
Critical Gap Module:				
Critical Gp:	6.4 6.5	6.2 xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx
FollowUpTim:	3.5 4.0	3.3 xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx
Capacity Module:				
Cnflct Vol:	485 485	105 xxxx	xxxx xxxx xxxxxx	120 xxxx xxxxxx
Potent Cap.:	541 482	949 xxxx	xxxx xxxx xxxxxx	1468 xxxx xxxxxx
Move Cap.:	513 449	949 xxxx	xxxx xxxx xxxxxx	1468 xxxx xxxxxx
Volume/Cap:	0.16 0.00	0.08 xxxx	xxxx xxxx xxxxxx	0.07 xxxx xxxxxx
Level Of Service Module:				
ZWay95thQ:	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	0.2 xxxx xxxxxx
Control Del:	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	7.6 xxxx xxxxxx
LOS by Move:	A	A	A	A
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx 666 xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx
Shared Queue:	xxxx 0.9 xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx
Shrd ConDel:	xxxx 12.1 xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx	xxxx xxxx xxxxxx
Shared LOS:	B	B	B	B
ApproachDel:	12.1	xxxxxxx	xxxxxxx	xxxxxxx
ApproachLOS:	B	B	B	B
HevVeh:	2%	2%	2%	2%
Grade:	0%	0%	0%	0%
Peds/Hour:	0	0	0	0
Pedestrian Walk Speed:	4.00 feet/sec	4.00 feet/sec	4.00 feet/sec	4.00 feet/sec
LaneWidth:	12 feet	12 feet	12 feet	12 feet
Time Period:	0.25 hour	0.25 hour	0.25 hour	0.25 hour

Peak Hour Delay Signal Warrant Report
Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1! 0 0	0 0 0 0 0	0 0 0 1 0	1 0 1 0 0
Initial Vol:	80 0 80	0 0 0 0	0 90 30	100 180 0
ApproachDel:	12.1	xxxxxxx	xxxxxxx	xxxxxxx

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=160]
SUCCEEDED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=560]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]

Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	80	0	80	0	0	0	0	90	30	100	180	0

Major Street Volume: 400
 Minor Approach Volume: 160
 Minor Approach Volume Threshold: 601

SIGNAL WARRANT DISCLAIMER

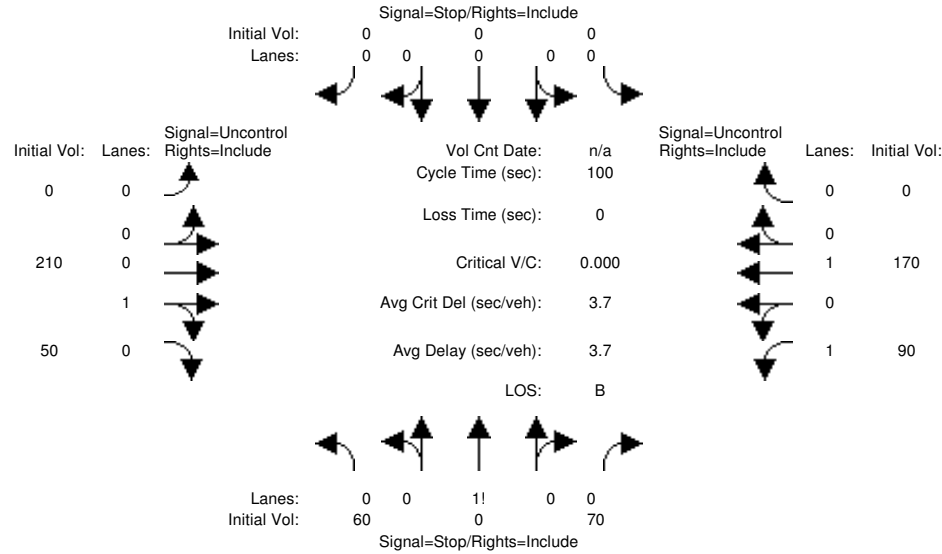
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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Cum w/proj PM

Intersection #29: Street C/Stone-Boswell Access West



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Volume Module:	-----											
Base Vol:	60	0	70	0	0	0	0	210	50	90	170	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	60	0	70	0	0	0	0	210	50	90	170	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	60	0	70	0	0	0	0	210	50	90	170	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Volume:	60	0	70	0	0	0	0	210	50	90	170	0
Critical Gap Module:	-----											
Critical Gp:	6.4	6.5	6.2	xxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	4.1	xxxx	xxxxxx
FollowUpTim:	3.5	4.0	3.3	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	2.2	xxxx	xxxxxx
Capacity Module:	-----											
Cnflct Vol:	585	585	235	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	260	xxxx	xxxxxx
Potent Cap.:	473	423	804	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1304	xxxx	xxxxxx
Move Cap.:	448	394	804	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1304	xxxx	xxxxxx
Volume/Cap:	0.13	0.00	0.09	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.07	xxxx	xxxx
Level Of Service Module:	-----											
ZWay95thQ:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.2	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	8.0	xxxx	xxxxxx
LOS by Move:	A											
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	589	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Shared Queue:	xxxxxx	0.8	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	12.8	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	B											
ApproachDel:	12.8	xxxxxxx					xxxxxxx			xxxxxxx		
ApproachLOS:	B	*					*			*		
HevVeh:	2%	2%					2%			2%		
Grade:	0%	0%					0%			0%		
Peds/Hour:	0	0					0			0		
Pedestrian Walk Speed:	4.00 feet/sec											
LaneWidth:	12 feet			12 feet			12 feet			12 feet		
Time Period:	0.25 hour											

Peak Hour Delay Signal Warrant Report
Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	0	1	0	1
Initial Vol:	60	0	70	0	0	0	0	210	50	90	170	0
ApproachDel:	12.8	xxxxxxx					xxxxxxx			xxxxxxx		

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=130]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=650]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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Peak Hour Volume Signal Warrant Report [Urban]
Intersection #29 Street C/Stone-Boswell Access West

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	1	0	0	0	0	0	0	0	0	1	0	1	0	0
Initial Vol:	60	0	70		0	0	0		0	210	50		90	170	0	
Major Street Volume:	520															
Minor Approach Volume:	130															
Minor Approach Volume Threshold:	510															

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (alternative)
Cum w/proj AM

Intersection #30: Street C/Stone-Boswell Access East

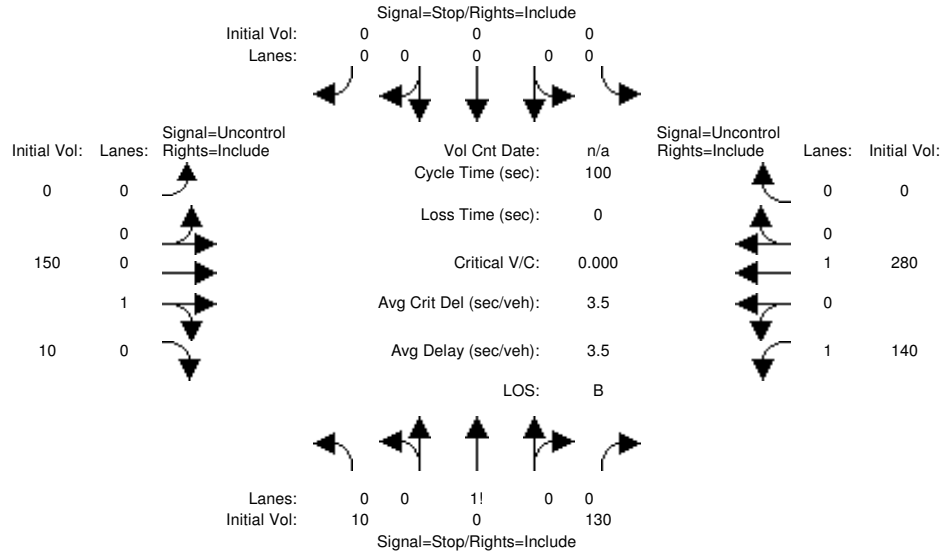


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report

Intersection #30 Street C/Stone-Boswell Access East
Base Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.4]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=140]
SUCCEEDED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=720]
SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
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The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Lanes:	0	0	1	0	0	0	0	0	1	1	0	1
Initial Vol:	10	0	130	0	0	0	0	150	10	140	280	0

Major Street Volume: 580
 Minor Approach Volume: 140
 Minor Approach Volume Threshold: 473

SIGNAL WARRANT DISCLAIMER

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Delta Shore

Level Of Service Computation Report
2000 HCM Unsignalized (Base Volume Alternative)
Cum w/proj PM

Intersection #30: Street C/Stone-Boswell Access East

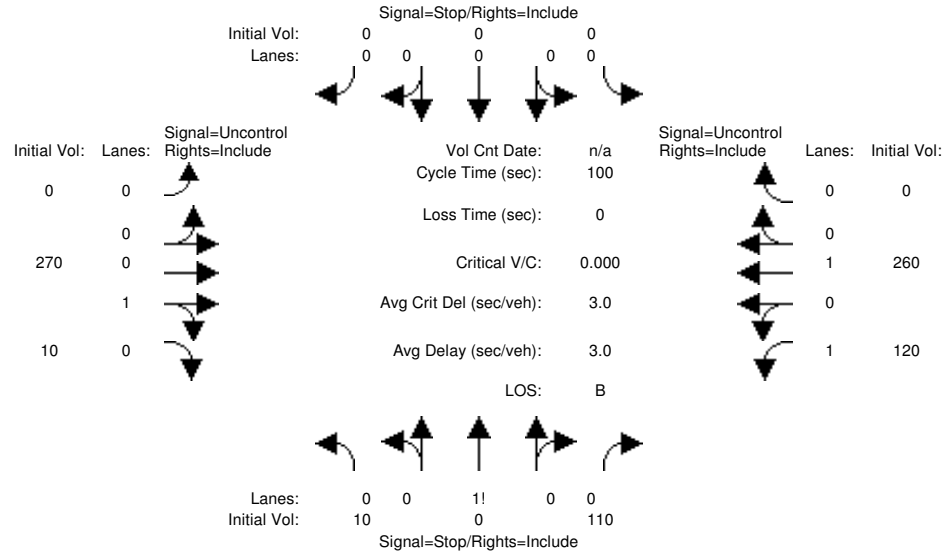


Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #30 Street C/Stone-Boswell Access East

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control (Stop Sign, Uncontrolled), Lanes, Initial Vol, and ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.4]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=120]
SUCCEEDED - Approach volume greater than or equal to 100 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

Peak Hour Volume Signal Warrant Report [Urban]
Intersection #30 Street C/Stone-Boswell Access East

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	1	0	0	0	0	0	0	0	0	1	0	1	0	0
Initial Vol:	10	0	110		0	0	0		0	270	10		120	260	0	
Major Street Volume:	660															
Minor Approach Volume:	120															
Minor Approach Volume Threshold:	428															

SIGNAL WARRANT DISCLAIMER

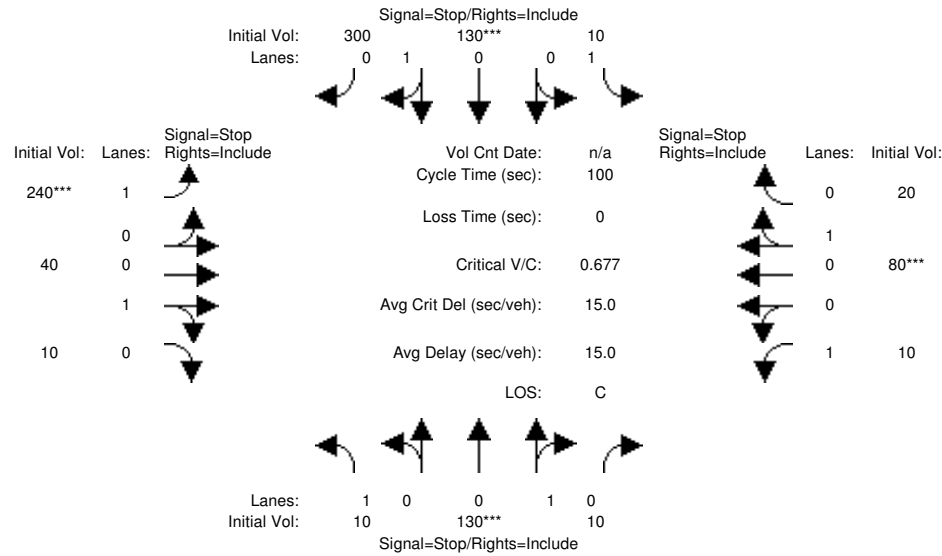
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shore

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Cum w/proj AM

Intersection #31: Street C/Detroit Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:												
Base Vol:	10	130	10	10	130	300	240	40	10	10	80	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	130	10	10	130	300	240	40	10	10	80	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	130	10	10	130	300	240	40	10	10	80	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	10	130	10	10	130	300	240	40	10	10	80	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	10	130	10	10	130	300	240	40	10	10	80	20
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.93	0.07	1.00	0.30	0.70	1.00	0.80	0.20	1.00	0.80	0.20
Final Sat.:	493	497	38	534	192	443	503	437	109	461	403	101
Capacity Analysis Module:												
Vol/Sat:	0.02	0.26	0.26	0.02	0.68	0.68	0.48	0.09	0.09	0.02	0.20	0.20
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Delay/Veh:	9.7	11.1	11.1	9.3	18.3	18.3	15.1	9.5	9.5	10.0	10.7	10.7
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	9.7	11.1	11.1	9.3	18.3	18.3	15.1	9.5	9.5	10.0	10.7	10.7
LOS by Move:	A	B	B	A	C	C	C	A	A	B	B	B
ApproachDel:	11.0			18.1			14.2			10.6		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	11.0			18.1			14.2			10.6		
LOS by Appr:	B			C			B			B		
AllWayAvgQ:	0.0	0.3	0.3	0.0	1.8	1.8	0.8	0.1	0.1	0.0	0.2	0.2
Time Period: 0.25 hour												
HevVeh:	2%			2%			2%			2%		
Alpha Value: 0.01												
GroupType:	5			5			5			5		
P[C1]:	0.13			0.30			0.20			0.12		
P[C2]:	0.27			0.10			0.05			0.12		
P[C3]:	0.17			0.37			0.46			0.29		
P[C4]:	0.37			0.20			0.25			0.38		
P[C5]:	0.07			0.03			0.03			0.09		
Padj[C1]:	0.020			0.016			0.019			0.022		
Padj[C2]:	0.008			0.008			0.010			0.012		
Padj[C3]:	-0.000			-0.009			-0.011			-0.003		
Padj[C4]:	-0.021			-0.012			-0.015			-0.022		
Padj[C5]:	-0.007			-0.003			-0.003			-0.009		
Lanes:	L1	L2		L1	L2		L1	L2		L1	L2	
LaneType:	LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU		LEFT	RTTHRU	
HeadwayAdj:	0.534	-0.016		0.534	-0.454		0.534	-0.106		0.534	-0.106	
Volume:	10	140		10	430		240	50		10	100	
Capacity:	493	535		534	636		503	547		461	504	
DegOfUtil:	0.02	0.25		0.02	0.66		0.46	0.09		0.02	0.18	
DepHeadway:	6.89	6.34		6.52	5.53		6.84	6.20		7.18	6.54	
ServiceTime:	4.6	4.0		4.2	3.2		4.5	3.9		4.9	4.2	
Delay:	9.7	11.1		9.3	18.3		15.1	9.5		10.0	10.7	
Queue:	0.0	0.3		0.0	1.8		0.8	0.1		0.0	0.2	
Approach:	North Bound			South Bound			East Bound			West Bound		
ApproachDel:	11.0			18.1			14.2			10.6		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	11.0			18.1			14.2			10.6		
LOS by Appr:	B			C			B			B		
OverallDel:	15.0											
OverallLOS:	C											
Peak Hour Volume Signal Warrant Report [Urban]												

Intersection #31 Street C/Detroit Boulevard												

Base Volume Alternative: Peak Hour Warrant NOT Met												

Approach:	North Bound			South Bound			East Bound			West Bound		

```

Movement:   L - T - R   L - T - R   L - T - R   L - T - R
-----|-----|-----|-----|
Control:    Stop Sign   Stop Sign   Stop Sign   Stop Sign
Lanes:      1 0 0 1 0   1 0 0 1 0   1 0 0 1 0   1 0 0 1 0
Initial Vol: 10 130 10   10 130 300   240 40 10   10 80 20
-----|-----|-----|-----|
Major Street Volume:      590
Minor Approach Volume:    290
Minor Approach Volume Threshold: 601
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

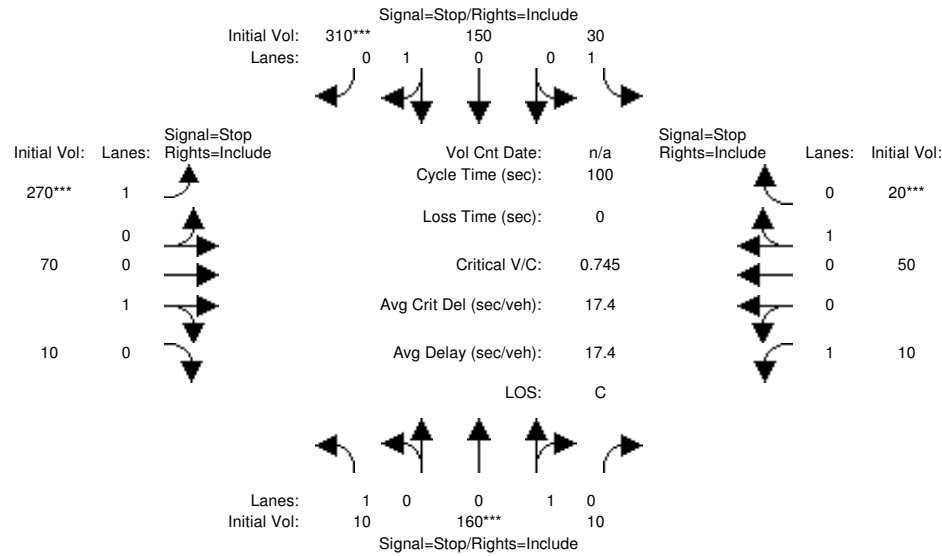
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Delta Shore

Level Of Service Computation Report
2000 HCM 4-Way Stop (Base Volume Alternative)
Cum w/proj PM

Intersection #31: Street C/Detroit Boulevard



Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Volume Module:	Base Vol: 10 160 10 30 150 310 270 70 10 10 50 20 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 10 160 10 30 150 310 270 70 10 10 50 20 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 10 160 10 30 150 310 270 70 10 10 50 20 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 10 160 10 30 150 310 270 70 10 10 50 20 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Final Volume: 10 160 10 30 150 310 270 70 10 10 50 20											
Saturation Flow Module:	Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.94 0.06 1.00 0.33 0.67 1.00 0.87 0.13 1.00 0.71 0.29 Final Sat.: 480 490 31 522 201 416 492 465 66 434 339 136											

Capacity Analysis Module:												
Vol/Sat:	0.02	0.33	0.33	0.06	0.75	0.75	0.55	0.15	0.15	0.02	0.15	0.15
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Delay/Veh:	9.9	12.2	12.2	9.8	22.4	22.4	17.3	10.2	10.2	10.4	10.6	10.6
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	9.9	12.2	12.2	9.8	22.4	22.4	17.3	10.2	10.2	10.4	10.6	10.6
LOS by Move:	A	B	B	A	C	C	C	B	B	B	B	B
ApproachDel:	12.1			21.6			15.7			10.6		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.1			21.6			15.7			10.6		
LOS by Appr:	B			C			C			B		
AllWayAvgQ:	0.0	0.4	0.4	0.1	2.4	2.4	1.0	0.2	0.2	0.0	0.1	0.1

Time Period: 0.25 hour
 HevVeh: 2% 2% 2% 2%
 Alpha Value: 0.01

GroupType:	5		5		5		5	
P[C1]:	0.09		0.23		0.15		0.07	
P[C2]:	0.26		0.11		0.03		0.10	
P[C3]:	0.14		0.38		0.50		0.24	
P[C4]:	0.44		0.24		0.29		0.45	
P[C5]:	0.07		0.03		0.04		0.14	
Padj[C1]:	0.021		0.017		0.020		0.025	
Padj[C2]:	0.010		0.008		0.012		0.015	
Padj[C3]:	0.001		-0.008		-0.011		0.000	
Padj[C4]:	-0.026		-0.014		-0.017		-0.025	
Padj[C5]:	-0.007		-0.003		-0.004		-0.014	

Lanes:	L1 L2		L1 L2		L1 L2		L1 L2	
LaneType:	LEFT	RTTHRU	LEFT	RTTHRU	LEFT	RTTHRU	LEFT	RTTHRU
HeadwayAdj:	0.534	-0.007	0.534	-0.438	0.534	-0.053	0.534	-0.166
Volume:	10	170	30	460	270	80	10	70
Capacity:	480	521	522	617	492	531	434	475
DegOfUtil:	0.02	0.31	0.06	0.73	0.53	0.14	0.02	0.13
DepHeadway:	7.10	6.56	6.69	5.72	7.03	6.44	7.56	6.86
ServiceTime:	4.8	4.3	4.4	3.4	4.7	4.1	5.3	4.6
Delay:	9.9	12.2	9.8	22.4	17.3	10.2	10.4	10.6
Queue:	0.0	0.4	0.1	2.4	1.0	0.2	0.0	0.1

Approach:	North Bound		South Bound		East Bound		West Bound	
ApproachDel:	12.1		21.6		15.7		10.6	
Delay Adj:	1.00		1.00		1.00		1.00	
ApprAdjDel:	12.1		21.6		15.7		10.6	
LOS by Appr:	B		C		C		B	
OverallDel:	17.4							
OverallLOS:	C							

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #31 Street C/Detroit Boulevard

 Base Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound


```

Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Stop Sign      Stop Sign      Stop Sign      Stop Sign
Lanes:        1 0 0 1 0      1 0 0 1 0      1 0 0 1 0      1 0 0 1 0
Initial Vol:   10 160 10      30 150 310      270 70 10      10 50 1 20
-----|-----|-----|-----|
Major Street Volume:      670
Minor Approach Volume:    350
Minor Approach Volume Threshold: 546
-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

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```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.00 0.00  0.17  0.00 0.75  0.00  0.24 0.70  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  0.0 0.0  3.2  0.0 7.1  0.0  5.3 2.4  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  0.00 0.00  1.00 0.00  1.00  0.00 1.00  1.00  0.00
Q2:           0.0 0.0  0.0  0.0 0.0  2.4  0.0 1.8  0.0  3.1 0.4  0.0
HCM2kQueue:   0.0 0.0  0.0  0.0 0.0  5.7  0.0 8.9  0.0  8.4 2.8  0.0
-----
70thFactor:   1.20 1.20  1.20  1.20 1.20  1.19  1.20 1.18  1.20  1.18 1.19  1.20
HCM2k70thQ:   0.0 0.0  0.0  0.0 0.0  6.7  0.0 10.5  0.0  10.0 3.4  0.0
-----
85thFactor:   1.60 1.60  1.60  1.60 1.60  1.55  1.60 1.52  1.60  1.53 1.57  1.60
HCM2k85thQ:   0.0 0.0  0.0  0.0 0.0  8.8  0.0 13.5  0.0  12.9 4.5  0.0
-----
90thFactor:   1.80 1.80  1.80  1.80 1.80  1.70  1.80 1.66  1.80  1.66 1.75  1.80
HCM2k90thQ:   0.0 0.0  0.0  0.0 0.0  9.7  0.0 14.7  0.0  14.0 5.0  0.0
-----
95thFactor:   2.10 2.10  2.10  2.10 2.10  1.94  2.10 1.87  2.10  1.88 2.01  2.10
HCM2k95thQ:   0.0 0.0  0.0  0.0 0.0  11.0  0.0 16.6  0.0  15.8 5.7  0.0
-----
98thFactor:   2.70 2.70  2.70  2.70 2.70  2.35  2.70 2.20  2.70  2.22 2.50  2.70
HCM2k98thQ:   0.0 0.0  0.0  0.0 0.0  13.3  0.0 19.6  0.0  18.8 7.1  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

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*****
Intersection #32
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0  0.0 0.0  85.8  0.0 203  0.0  80.0 69.0  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 58.955 pounds
                  9.551 gallons
Carbon Dioxide:  183.940 pounds
Carbon Monoxide: 13.151 pounds
Hydrocarbons:    2.007 pounds
Nitrogen Oxides: 0.603 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 53.541 pounds
                  8.674 gallons
Carbon Dioxide:  167.049 pounds
Carbon Monoxide: 12.734 pounds
Hydrocarbons:    1.921 pounds
Nitrogen Oxides: 0.526 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  | 0.00 0.00  0.17 | 0.00 0.75  0.00 | 0.24 0.70  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  | 1.00 1.00  1.00  | 1.00 1.00  1.00  | 1.00 1.00  1.00
Q1:           0.0 0.0  0.0  | 0.0 0.0  4.8  | 0.0 9.5  0.0  | 8.3 5.3  0.0
UpstreamVC:   0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00  | 0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  | 0.00 0.00  1.00  | 0.00 1.00  0.00  | 1.00 1.00  0.00
Q2:           0.0 0.0  0.0  | 0.0 0.0  7.5  | 0.0 2.6  0.0  | 13.9 1.1  0.0
HCM2kQueue:   0.0 0.0  0.0  | 0.0 0.0  12.3 | 0.0 12.1  0.0  | 22.3 6.4  0.0
-----
70thFactor:   1.20 1.20  1.20  | 1.20 1.20  1.17 | 1.20 1.17  1.20  | 1.16 1.19  1.20
HCM2k70thQ:   0.0 0.0  0.0  | 0.0 0.0  14.4  | 0.0 14.2  0.0  | 25.8 7.6  0.0
-----
85thFactor:   1.60 1.60  1.60  | 1.60 1.60  1.50 | 1.60 1.50  1.60  | 1.44 1.54  1.60
HCM2k85thQ:   0.0 0.0  0.0  | 0.0 0.0  18.4  | 0.0 18.2  0.0  | 32.1 9.9  0.0
-----
90thFactor:   1.80 1.80  1.80  | 1.80 1.80  1.62 | 1.80 1.62  1.80  | 1.53 1.69  1.80
HCM2k90thQ:   0.0 0.0  0.0  | 0.0 0.0  19.9  | 0.0 19.6  0.0  | 34.1 10.8  0.0
-----
95thFactor:   2.10 2.10  2.10  | 2.10 2.10  1.80 | 2.10 1.81  2.10  | 1.67 1.92  2.10
HCM2k95thQ:   0.0 0.0  0.0  | 0.0 0.0  22.1  | 0.0 21.9  0.0  | 37.3 12.3  0.0
-----
98thFactor:   2.70 2.70  2.70  | 2.70 2.70  2.09 | 2.70 2.09  2.70  | 1.88 2.31  2.70
HCM2k98thQ:   0.0 0.0  0.0  | 0.0 0.0  25.7 | 0.0 25.4  0.0  | 41.9 14.8  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #32
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:   0.0 0.0  0.0  | 0.0 129.5  0.0  | 0.0 270  0.0  | 132.4 152  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 136.678 pounds
                  22.142 gallons
Carbon Dioxide:  426.437 pounds
Carbon Monoxide: 33.929 pounds
Hydrocarbons:    6.392 pounds
Nitrogen Oxides: 1.125 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 129.175 pounds
                  20.926 gallons
Carbon Dioxide:  403.026 pounds
Carbon Monoxide: 33.347 pounds
Hydrocarbons:    6.273 pounds
Nitrogen Oxides: 1.015 pounds
    
```

DISCLAIMER
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Green/Cycle:  0.00 0.00  0.00  0.52 0.00  0.52  0.00 0.40  0.00  0.00 0.40  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  9.6 0.0  30.8  0.0 23.0  0.0  0.0 28.2  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  1.2 0.0  39.8  0.0 16.2  0.0  0.0 36.7  0.0
HCM2kQueue:   0.0 0.0  0.0  10.8 0.0  70.6  0.0 39.2  0.0  0.0 64.8  0.0
-----|-----|-----|-----|
70thFactor:   1.20 1.20  1.20  1.18 1.20  1.12  1.20 1.14  1.20  1.20 1.12  1.20
HCM2k70thQ:   0.0 0.0  0.0  12.7 0.0  78.9  0.0 44.6  0.0  0.0 72.6  0.0
-----|-----|-----|-----|
85thFactor:   1.60 1.60  1.60  1.51 1.60  1.33  1.60 1.38  1.60  1.60 1.33  1.60
HCM2k85thQ:   0.0 0.0  0.0  16.4 0.0  93.8  0.0 54.2  0.0  0.0 86.5  0.0
-----|-----|-----|-----|
90thFactor:   1.80 1.80  1.80  1.63 1.80  1.41  1.80 1.46  1.80  1.80 1.42  1.80
HCM2k90thQ:   0.0 0.0  0.0  17.7 0.0  99.7  0.0 57.1  0.0  0.0 91.8  0.0
-----|-----|-----|-----|
95thFactor:   2.10 2.10  2.10  1.83 2.10  1.51  2.10 1.57  2.10  2.10 1.52  2.10
HCM2k95thQ:   0.0 0.0  0.0  19.8 0.0 106.8  0.0 61.5  0.0  0.0 98.3  0.0
-----|-----|-----|-----|
98thFactor:   2.70 2.70  2.70  2.13 2.70  1.70  2.70 1.75  2.70  2.70 1.71  2.70
HCM2k98thQ:   0.0 0.0  0.0  23.1 0.0 120.4  0.0 68.6  0.0  0.0 111  0.0
-----|-----|-----|-----|
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #33
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  0.0 0.0  0.0 167.8  0.0 448.1  0.0 608  0.0  0.0 910  0.0
-----|-----|-----|-----|
Name: year 1995 composite fleet
Fuel Consumption: 756.692 pounds
                  122.584 gallons
Carbon Dioxide:  2360.878 pounds
Carbon Monoxide: 206.461 pounds
Hydrocarbons:    44.753 pounds
Nitrogen Oxides: 4.804 pounds
-----|-----|-----|-----|
Name: year 2000 composite fleet
Fuel Consumption: 742.328 pounds
                  120.257 gallons
Carbon Dioxide:  2316.062 pounds
Carbon Monoxide: 205.306 pounds
Hydrocarbons:    44.526 pounds
Nitrogen Oxides: 4.548 pounds
    
```

DISCLAIMER
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design Base Volume Alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.


```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.00 0.00  0.00  0.53 0.00  0.53  0.00 0.39  0.00  0.00 0.39  0.00
ArrivalType:           3           3           3           3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00  1.00 1.00  1.00
Q1:           0.0 0.0  0.0  20.5 0.0  33.6  0.0 23.3  0.0  0.0 28.7  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00  0.00 0.00  0.00
EarlyArrAdj:  0.00 0.00  0.00  1.00 0.00  1.00  0.00 1.00  0.00  0.00 1.00  0.00
Q2:           0.0 0.0  0.0  5.5 0.0  48.9  0.0 20.0  0.0  0.0 42.1  0.0
HCM2kQueue:   0.0 0.0  0.0  26.0 0.0  82.5  0.0 43.3  0.0  0.0 70.8  0.0
-----
70thFactor:   1.20 1.20  1.20  1.15 1.20  1.11  1.20 1.13  1.20  1.20 1.12  1.20
HCM2k70thQ:   0.0 0.0  0.0  30.0 0.0  91.8  0.0 49.1  0.0  0.0 79.1  0.0
-----
85thFactor:   1.60 1.60  1.60  1.43 1.60  1.32  1.60 1.37  1.60  1.60 1.33  1.60
HCM2k85thQ:   0.0 0.0  0.0  37.1 0.0 108.8  0.0 59.4  0.0  0.0 94.1  0.0
-----
90thFactor:   1.80 1.80  1.80  1.51 1.80  1.41  1.80 1.45  1.80  1.80 1.41  1.80
HCM2k90thQ:   0.0 0.0  0.0  39.2 0.0 116.0  0.0 62.7  0.0  0.0 xxxx  0.0
-----
95thFactor:   2.10 2.10  2.10  1.64 2.10  1.51  2.10 1.55  2.10  2.10 1.51  2.10
HCM2k95thQ:   0.0 0.0  0.0  42.7 0.0 124.2  0.0 67.4  0.0  0.0 107  0.0
-----
98thFactor:   2.70 2.70  2.70  1.84 2.70  1.70  2.70 1.74  2.70  2.70 1.70  2.70
HCM2k98thQ:   0.0 0.0  0.0  47.7 0.0 140.4  0.0 75.2  0.0  0.0 121  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #33
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:   0.0 0.0  0.0 358.2  0.0 599.7  0.0 639  0.0  0.0 971  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  948.307 pounds
                  153.626 gallons
Carbon Dioxide:   2958.719 pounds
Carbon Monoxide:  259.866 pounds
Hydrocarbons:     56.653 pounds
Nitrogen Oxides:  5.932 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  931.952 pounds
                  150.976 gallons
Carbon Dioxide:   2907.690 pounds
Carbon Monoxide:  258.544 pounds
Hydrocarbons:     56.395 pounds
Nitrogen Oxides:  5.633 pounds
    
```

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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj AM

Intersection #34:

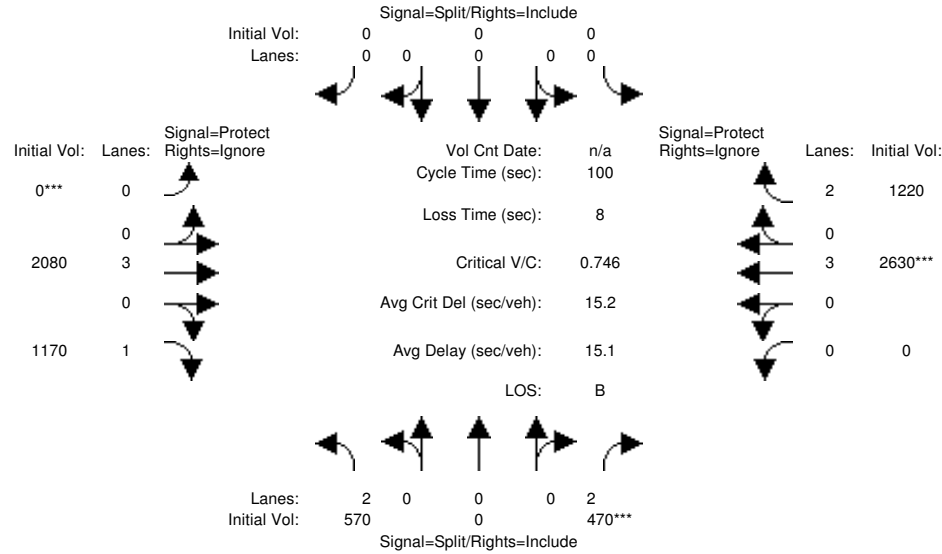


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, HCM Ops Saturation Adj Module, and Delay Adjustment Factor Module.

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.23 0.00  0.23  0.00 0.00  0.00  0.00 0.69  0.00  0.00 0.69  0.00
ArrivalType:          3          3          3          3
ProgFactor:   1.00 1.00  1.00  1.00 1.00  1.00 1.00  1.00  1.00 1.00 1.00  1.00
Q1:           7.6 0.0  6.9  0.0 0.0  0.0  0.0 11.0  0.0  0.0 17.0  0.0
UpstreamVC:   0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00  0.00
UpstreamAdj:  0.00 0.00  0.00  0.00 0.00  0.00 0.00  0.00  0.00 0.00 0.00  0.00
EarlyArrAdj:  1.00 0.00  1.00  0.00 0.00  0.00 0.00  1.00  0.00  0.00 1.00  0.00
Q2:           2.3 0.0  2.4  0.0 0.0  0.0  0.0 1.4  0.0  0.0 2.7  0.0
HCM2KQueue:   9.9 0.0  9.3  0.0 0.0  0.0  0.0 12.4  0.0  0.0 19.7  0.0
-----
70thFactor:  1.18 1.20  1.18  1.20 1.20  1.20 1.20  1.17  1.20  1.20 1.16  1.20
HCM2k70thQ:  11.7 0.0  11.0  0.0 0.0  0.0  0.0 14.5  0.0  0.0 22.9  0.0
-----
85thFactor:  1.52 1.60  1.52  1.60 1.60  1.60 1.60  1.50  1.60  1.60 1.46  1.60
HCM2k85thQ:  15.0 0.0  14.2  0.0 0.0  0.0  0.0 18.5  0.0  0.0 28.7  0.0
-----
90thFactor:  1.64 1.80  1.65  1.80 1.80  1.80 1.80  1.62  1.80  1.80 1.55  1.80
HCM2k90thQ:  16.3 0.0  15.4  0.0 0.0  0.0  0.0 20.0  0.0  0.0 30.6  0.0
-----
95thFactor:  1.85 2.10  1.86  2.10 2.10  2.10 2.10  1.80  2.10  2.10 1.70  2.10
HCM2k95thQ:  18.3 0.0  17.3  0.0 0.0  0.0  0.0 22.3  0.0  0.0 33.6  0.0
-----
98thFactor:  2.17 2.70  2.19  2.70 2.70  2.70 2.70  2.09  2.70  2.70 1.92  2.70
HCM2k98thQ:  21.5 0.0  20.4  0.0 0.0  0.0  0.0 25.8  0.0  0.0 37.9  0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #34
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH      30 MPH      30 MPH      30 MPH
NumOfStops:  132.2 0.0 109.4  0.0 0.0  0.0  0.0 270  0.0  0.0 417  0.0
-----
Name: year 1995 composite fleet
Fuel Consumption:  129.526 pounds
                  20.983 gallons
Carbon Dioxide:   404.123 pounds
Carbon Monoxide:  30.180 pounds
Hydrocarbons:     5.027 pounds
Nitrogen Oxides:  1.255 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption:  119.429 pounds
                  19.347 gallons
Carbon Dioxide:   372.617 pounds
Carbon Monoxide:  29.396 pounds
Hydrocarbons:     4.866 pounds
Nitrogen Oxides:  1.106 pounds
    
```

DISCLAIMER
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Delta Shore

Level Of Service Computation Report
2000 HCM Operations (alternative)
Cum w/proj PM

Intersection #34:

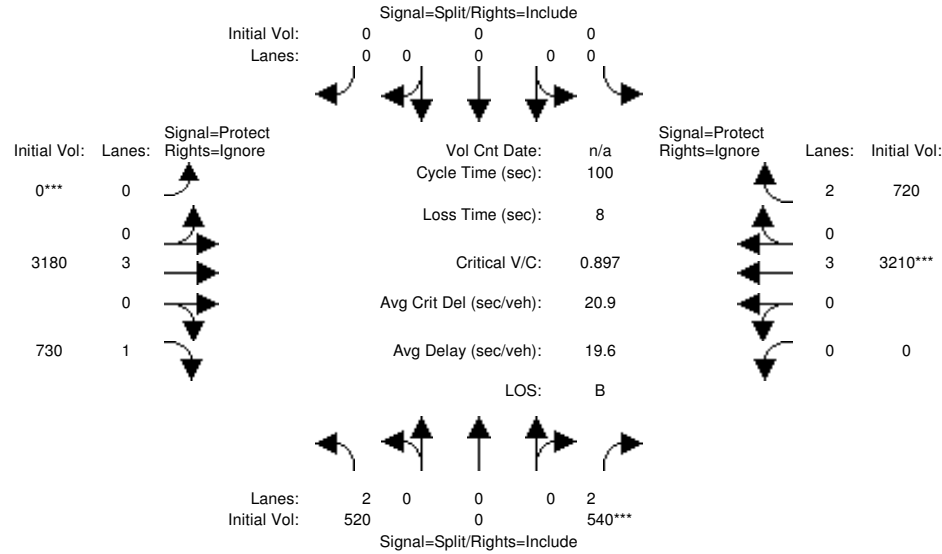


Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Volume Module, Saturation Flow Module, Capacity Analysis Module, HCM Ops Adjusted Lane Utilization Module, HCM Ops Input Saturation Adj Module, HCM Ops f(lt) Adj Case Module, and HCM Ops Saturation Adj Module.

Level Of Service Detailed Computation Report (HCM2000 Queue Method)
2000 HCM Operations Method
Base Volume Alternative
Intersection #34

```

*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Green/Cycle:  0.22 0.00 0.22 | 0.00 0.00 0.00 | 0.00 0.70 0.00 | 0.00 0.70 0.00
ArrivalType:      3          3          3          3
ProgFactor:    1.00 1.00 1.00 | 1.00 1.00 1.00 | 1.00 1.00 1.00 | 1.00 1.00 1.00
Q1:            6.9 0.0 8.3 | 0.0 0.0 0.0 | 0.0 25.6 0.0 | 0.0 26.2 0.0
UpstreamVC:    0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00
UpstreamAdj:  0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00 | 0.00 0.00 0.00
EarlyArrAdj:  1.00 0.00 1.00 | 0.00 0.00 0.00 | 0.00 1.00 0.00 | 0.00 1.00 0.00
Q2:           2.1 0.0 4.4 | 0.0 0.0 0.0 | 0.0 6.0 0.0 | 0.0 6.3 0.0
HCM2KQueue:   8.9 0.0 12.7 | 0.0 0.0 0.0 | 0.0 31.6 0.0 | 0.0 32.6 0.0
-----
70thFactor:   1.18 1.20 1.17 | 1.20 1.20 1.20 | 1.20 1.15 1.20 | 1.20 1.14 1.20
HCM2k70thQ:  10.5 0.0 14.8 | 0.0 0.0 0.0 | 0.0 36.2 0.0 | 0.0 37.3 0.0
-----
85thFactor:   1.52 1.60 1.50 | 1.60 1.60 1.60 | 1.60 1.40 1.60 | 1.60 1.40 1.60
HCM2k85thQ:  13.6 0.0 18.9 | 0.0 0.0 0.0 | 0.0 44.4 0.0 | 0.0 45.7 0.0
-----
90thFactor:   1.66 1.80 1.61 | 1.80 1.80 1.80 | 1.80 1.48 1.80 | 1.80 1.48 1.80
HCM2k90thQ:  14.8 0.0 20.4 | 0.0 0.0 0.0 | 0.0 46.8 0.0 | 0.0 48.2 0.0
-----
95thFactor:   1.87 2.10 1.80 | 2.10 2.10 2.10 | 2.10 1.60 2.10 | 2.10 1.60 2.10
HCM2k95thQ:  16.7 0.0 22.7 | 0.0 0.0 0.0 | 0.0 50.7 0.0 | 0.0 52.1 0.0
-----
98thFactor:   2.20 2.70 2.08 | 2.70 2.70 2.70 | 2.70 1.79 2.70 | 2.70 1.78 2.70
HCM2k98thQ:  19.7 0.0 26.3 | 0.0 0.0 0.0 | 0.0 56.5 0.0 | 0.0 58.0 0.0
-----
Fuel Consumption and Emissions
2000 HCM Operations Method
Base Volume Alternative
    
```

```

*****
Intersection #34
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:     L - T - R      L - T - R      L - T - R      L - T - R
-----
Run Speed:    30 MPH          30 MPH          30 MPH          30 MPH
NumOfStops:  120.1 0.0 131.3 | 0.0 0.0 0.0 | 0.0 629 0.0 | 0.0 645 0.0
-----
Name: year 1995 composite fleet
Fuel Consumption: 195.068 pounds
                  31.601 gallons
Carbon Dioxide:  608.613 pounds
Carbon Monoxide: 46.656 pounds
Hydrocarbons:    8.092 pounds
Nitrogen Oxides: 1.883 pounds
-----
Name: year 2000 composite fleet
Fuel Consumption: 181.381 pounds
                  29.384 gallons
Carbon Dioxide:  565.908 pounds
Carbon Monoxide: 45.580 pounds
Hydrocarbons:    7.875 pounds
Nitrogen Oxides: 1.666 pounds
    
```

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Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2392	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
E-mail:

Operational Analysis

Analyst: BP
Agency or Company: Fehr & Peers
Date Performed: 6/27/2007
Analysis Time Period:
Freeway/Direction: I-5 N of Meadowview, NB PM
From/To:
Jurisdiction:
Analysis Year:
Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	2925	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	795	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	1129	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1129	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	17.4	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
E-mail:

Operational Analysis

Analyst: BP
Agency or Company: Fehr & Peers
Date Performed: 6/27/2007
Analysis Time Period:
Freeway/Direction: I-5 N of Meadowview, SB AM
From/To:
Jurisdiction:
Analysis Year:
Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	2610	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	709	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	1007	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	1007	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	15.5	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
E-mail:

Operational Analysis

Analyst: BP
Agency or Company: Fehr & Peers
Date Performed: 6/27/2007
Analysis Time Period:
Freeway/Direction: I-5 N of Meadowview, SB PM
From/To:
Jurisdiction:
Analysis Year:
Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	6290	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1709	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	2427	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2427	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
E-mail:

Operational Analysis

Analyst: BP
Agency or Company: Fehr & Peers
Date Performed: 6/27/2007
Analysis Time Period:
Freeway/Direction: I-5 N of Laguna, NB AM
From/To:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores: SA07-0088A

Flow Inputs and Adjustments

Volume, V	5617	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1526	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	2167	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2167	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	57.9	mi/h
Number of lanes, N	3	
Density, D	37.4	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1073	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	16.5	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	903	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	13.9	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2182	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	57.5	mi/h
Number of lanes, N	3	
Density, D	37.9	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB OFF Pocket AM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	5617	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	502	vph	
Length of first accel/decel lane	150	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	5617	502	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1526	136	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6502	551	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.572 Using Equation 5

FD

$v = v + (v - v) P = 3956$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	6502	7050	No
$F_i F$			
$v = v - v$	5951	7050	No
$F O F R$			
v	551	2000	No
R			
$v v$	2546 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3956	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 36.9$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.478$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = 65.3$ mph
 0

Space mean speed for all vehicles, $S = 57.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2782	448	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	756	122	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3220	492	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.657 Using Equation 5

FD

$v = v + (v - v) P = 2284$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3220	7050	No
$F_i F$			
$v = v - v$	2728	7050	No
$F_O F R$			
v	492	2000	No
R			
$v v$	936 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2284	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 22.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.472$
 S

Space mean speed in ramp influence area, $S = 54.1$ mph
 R

Space mean speed in outer lanes, $S = 71.3$ mph
 0

Space mean speed for all vehicles, $S = 58.2$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Loop Pocket AM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	5115	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	408	vph	
Length of first accel/decel lane	450	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	677	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5115	408	677	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1390	111	184	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5921	448	743	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 3494$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	6369	7050	No
FO			
v v	2427 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3494	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 33.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.490$
 S

Space mean speed in ramp influence area, $S = 53.7$ mph
 R

Space mean speed in outer lanes, $S = 57.7$ mph
 0

Space mean speed for all vehicles, $S = 55.2$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Loop Pocket PM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

 Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	2334	vph

 On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	199	vph
Length of first accel/decel lane	450	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	392	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2334	199	392	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	634	54	107	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2702	218	430	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 1594 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2920	7050	No
FO			
v v	1108 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1594	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.313$
 S

Space mean speed in ramp influence area, $S = 57.8$ mph
 R

Space mean speed in outer lanes, $S = 62.8$ mph
 0

Space mean speed for all vehicles, $S = 59.6$ mph

Volume on adjacent Ramp	408	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	5523	677	408	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1501	184	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6393	743	448	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 3728 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7136	7050	Yes
FO			
v v	2665 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3728	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 38.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.648$
 S

Space mean speed in ramp influence area, $S = 50.1$ mph
 R

Space mean speed in outer lanes, $S = 56.3$ mph
 0

Space mean speed for all vehicles, $S = 52.2$ mph

Volume on adjacent Ramp	199	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2533	392	199	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	688	107	54	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2932	430	218	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 1710 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3362	7050	No
FO			
v v	1222 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1710	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.340$
 S

Space mean speed in ramp influence area, $S = 57.2$ mph
 R

Space mean speed in outer lanes, $S = 62.4$ mph
 0

Space mean speed for all vehicles, $S = 59.0$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2610	574	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	709	156	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3021	630	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)

EQ

$P = 0.655$ Using Equation 5

FD

$v = v + (v - v) P = 2197$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3021	7050	No
$F_i F$			
$v = v - v$	2391	7050	No
$F_O F R$			
v	630	2000	No
R			
$v v$	824 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2197	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 21.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.485$
 S

Space mean speed in ramp influence area, $S = 53.9$ mph
 R

Space mean speed in outer lanes, $S = 71.3$ mph
 0

Space mean speed for all vehicles, $S = 57.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB OFF Pocket PM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	6290	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1153	vph	
Length of first accel/decel lane	150	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	6290	1153	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1709	313	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	7281	1266	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.520 Using Equation 5

FD

$v = v + (v - v) P = 4392$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	7281	7050	Yes
$F_i F$			
$v = v - v$	6015	7050	No
$F_O F R$			
v	1266	2000	No
R			
$v v$	2889 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v = 4581$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4581	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.542$
 S

Space mean speed in ramp influence area, $S = 52.5$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.5$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Merge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 6/27/2007
 Analysis time period:
 Freeway/Dir of Travel: I-5 SB ON Loop Pocket AM
 Junction:
 Jurisdiction:
 Analysis Year: Existing
 Description: Delta Shores

 Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2036	vph	

 On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	97	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	208	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	2036	97	208	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	553	26	57	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2357	106	228	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

v = v (P) = 1381 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2463	7050	No
FO			
v v	976 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1381	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 15.1$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.317$
 S

Space mean speed in ramp influence area, $S = 57.7$ mph
 R

Space mean speed in outer lanes, $S = 63.3$ mph
 0

Space mean speed for all vehicles, $S = 59.8$ mph

Volume on adjacent Ramp	231	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5137	286	231	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1396	78	63	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5947	314	254	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 3484 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	6261	7050	No
FO			
v v	2463 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
--	--------	---------------	------------

v	3484	4400	No
-----	------	------	----

12			!
----	--	--	---

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 33.1$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.474$
 S

Space mean speed in ramp influence area, $S = 54.1$ mph
 R

Space mean speed in outer lanes, $S = 57.5$ mph
 0

Space mean speed for all vehicles, $S = 55.4$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Slip Pocket AM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2133	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	208	vph	
Length of first accel/decel lane	400	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	97	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2133	208	97	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	580	57	26	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2469	228	106	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 1454 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2697	7050	No
FO			
v v	1015 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1454	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.314$
 S

Space mean speed in ramp influence area, $S = 57.8$ mph
 R

Space mean speed in outer lanes, $S = 63.1$ mph
 0

Space mean speed for all vehicles, $S = 59.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Slip Pocket PM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	5423	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	231	vph
Length of first accel/decel lane	400	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	286	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	5423	231	286	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1474	63	78	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6278	254	314	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 3696 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	6532	7050	No
FO			
v v	2582 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3696	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 33.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.496$
 S

Space mean speed in ramp influence area, $S = 53.6$ mph
 R

Space mean speed in outer lanes, $S = 56.8$ mph
 0

Space mean speed for all vehicles, $S = 54.8$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2276	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	54.8	mi/h
Number of lanes, N	2	
Density, D	41.6	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	2412	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
E-mail:

Operational Analysis

Analyst: BP
 Agency or Company: Fehr & Peers
 Date Performed: 7/2/2007
 Analysis Time Period:
 Freeway/Direction: S of Cosumnes, SB AM
 From/To:
 Jurisdiction:
 Analysis Year: Existing
 Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	2078	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	565	v
Trucks and buses	6	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.971	
Driver population factor, fp	1.00	
Flow rate, vp	1163	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1163	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	17.9	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1542	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	23.7	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2211	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	56.7	mi/h
Number of lanes, N	2	
Density, D	39.0	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2250	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	55.6	mi/h
Number of lanes, N	2	
Density, D	40.5	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2279	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	54.7	mi/h
Number of lanes, N	2	
Density, D	41.7	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2094	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	59.6	mi/h
Number of lanes, N	2	
Density, D	35.1	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3771	731	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1025	199	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4222	803	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4222$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4222	4700	No
$F_i F$			
$v = v - v$	3419	4700	No
$F_O F R$			
v	803	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4222	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 38.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.500$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3433	648	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	933	176	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3843	711	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 1.000$ Using Equation 0
 FD
 $v = v + (v - v) P = 3843$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3843	4700	No
$F_i F$			
$v = v - v$	3132	4700	No
$FO F R$			
v	711	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3843	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 35.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.492$
 S

Space mean speed in ramp influence area, $S = 53.7$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 OFF Mack AM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	4072	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	342	vph
Length of first accel/decel lane	200	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4072	342	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1107	93	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4559	375	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4559$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4559	4700	No
$F_i F$			
$v = v - v$	4184	4700	No
$F_O F R$			
v	375	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4559	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.462$
 S

Space mean speed in ramp influence area, $S = 54.4$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.4$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3740	495	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1016	135	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4187	543	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 1.000$ Using Equation 0
 FD
 $v = v + (v - v) P = 4187$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4187	4700	No
$F_i F$			
$v = v - v$	3644	4700	No
$FO F R$			
v	543	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4187	4600	No

12 !

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 38.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.477$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.0$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 ON Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

 Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	1916	vph

 On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	180	vph
Length of first accel/decel lane	300	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	1916	180	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	521	49	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2145	198	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 2145 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2343	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2145	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 21.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.341$
 S

Space mean speed in ramp influence area, $S = 57.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 57.2$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2482	303	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	674	82	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2779	333	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2779$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	3112	4700	No
FO			
$v v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v /2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2779	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 27.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.388$
 S

Space mean speed in ramp influence area, $S = 56.1$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.1$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4066	741	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1105	201	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4552	813	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4552$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4552	4700	No
$F_i F$			
$v = v - v$	3739	4700	No
$F_O F R$			
v	813	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4552	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.501$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.5$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: NB 99 OFF Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Existing
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	4308	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	736	vph	
Length of first accel/decel lane	175	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4308	736	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1171	200	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4823	808	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4823$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4823	4700	Yes
$F_i F$			
$v = v - v$	4015	4700	No
$F_O F R$			
v	808	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4823	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 44.2$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.501$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.5$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2773	1068	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	754	290	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3105	1172	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 3105 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4277	4700	No
FO			
$v \ v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3105	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 36.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.584$
 S

Space mean speed in ramp influence area, $S = 51.6$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 51.6$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2464	1298	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	670	353	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2759	1425	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2759$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4184	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area			
	Actual	Max Desirable	Violation?
v	2759	4400	No
12			!
Level of Service Determination (if not F)			

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 35.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

- Intermediate speed variable, $M = 0.559$
- S
- Space mean speed in ramp influence area, $S = 52.1$ mph
- R
- Space mean speed in outer lanes, $S = N/A$ mph
- 0
- Space mean speed for all vehicles, $S = 52.1$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2682	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1362	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	21.0	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	1100	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	16.9	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2701	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2906	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1401	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	21.6	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1192	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	18.3	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone: Fax:
 E-mail:

Operational Analysis

Analyst: BP
 Agency or Company: Fehr & Peers
 Date Performed: 6/27/2007
 Analysis Time Period:
 Freeway/Direction: I-5 N of Meadowview, SB PM
 From/To:
 Jurisdiction:
 Analysis Year: Base No Proj
 Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	7540	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	2049	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	2909	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
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Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2909	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2520	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1246	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	19.2	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1146	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	17.6	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2647	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB OFF Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Base No Proj
Description: Delta Shores

 Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	6530	vph

 Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	390	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500	ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	6530	390	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1774	106	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	7559	428	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 3637$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	7559	7050	Yes
Fi F			
v = v - v	7131	7050	Yes
FO F R			
v	428	3800	No
R			
v v	3922 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 4859$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4859	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 32.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.467$
 S

Space mean speed in ramp influence area, $S = 54.3$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 57.6$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3230	190	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	878	52	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3739	209	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 1797$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3739	7050	No
$F_i F$			
$v = v - v$	3530	7050	No
$F_O F R$			
v	209	3800	No
R			
$v v$	1942 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 2136$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2136	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 9.1$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.447$
 S

Space mean speed in ramp influence area, $S = 54.7$ mph
 R

Space mean speed in outer lanes, $S = 69.0$ mph
 0

Space mean speed for all vehicles, $S = 60.0$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	6950	310	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1889	84	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8045	340	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.543 Using Equation 5

FD

$v = v + (v - v) P = 4526$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	8045	7050	Yes
$F_i F$			
$v = v - v$	7705	7050	Yes
$F_O F R$			
v	340	2000	No
R			
$v v$	3519 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5345$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5345	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 48.9$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.459$
 S

Space mean speed in ramp influence area, $S = 54.5$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 57.5$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB OFF Pocket PM
Junction:
Jurisdiction:
Analysis Year: Base No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	3530	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	480	vph
Length of first accel/decel lane	150	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3530	480	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	959	130	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4086	527	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.634 Using Equation 5

FD

$v = v + (v - v) P = 2782$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4086	7050	No
$F_i F$			
$v = v - v$	3559	7050	No
$F_O F R$			
v	527	2000	No
R			
$v v$	1304 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2782	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 26.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.475$
 S

Space mean speed in ramp influence area, $S = 54.1$ mph
 R

Space mean speed in outer lanes, $S = 70.1$ mph
 0

Space mean speed for all vehicles, $S = 58.3$ mph

Volume on adjacent Ramp	690	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6140	120	690	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1668	33	187	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7108	132	757	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 4314 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7240	7050	Yes
FO			
v v	2794 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4408$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4408	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 34.2$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.613$
 S

Space mean speed in ramp influence area, $S = 50.9$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 52.7$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3040	80	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	826	22	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3519	88	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 2136 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3607	7050	No
FO			
v v	1383 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2136	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.284$
 S

Space mean speed in ramp influence area, $S = 58.5$ mph
 R

Space mean speed in outer lanes, $S = 61.8$ mph
 0

Space mean speed for all vehicles, $S = 59.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
 E-mail:

 Merge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 6/27/2007
 Analysis time period:
 Freeway/Dir of Travel: I-5 NB ON Loop Pocket AM
 Junction:
 Jurisdiction:
 Analysis Year: Base No Proj
 Description: Delta Shores

 Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	6640	vph

 On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	410	vph
Length of first accel/decel lane	450	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	480	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6640	410	480	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1804	111	130	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7687	450	527	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.590$ Using Equation 1

FM

$v = v (P) = 4536$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	8137	7050	Yes
FO			
v v	3151 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4987$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4987	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 44.9$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.186$
 S

Space mean speed in ramp influence area, $S = 37.7$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 42.3$ mph

Volume on adjacent Ramp	120	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6260	690	120	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1701	187	33	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7247	757	132	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 4453 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8004	7050	Yes
FO			
v v	2794 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4547$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4547	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 38.2$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.013$
 S

Space mean speed in ramp influence area, $S = 41.7$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 45.6$ mph

Volume on adjacent Ramp	360	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3050	220	360	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	829	60	98	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3531	242	395	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 2084 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3773	7050	No
FO			
v v	1447 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2084	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.329$
 S

Space mean speed in ramp influence area, $S = 57.4$ mph
 R

Space mean speed in outer lanes, $S = 61.6$ mph
 0

Space mean speed for all vehicles, $S = 59.0$ mph

Volume on adjacent Ramp	80	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3120	410	80	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	848	111	22	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3612	450	88	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 2219 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4062	7050	No
FO			
v v	1393 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2219	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.285$
 S

Space mean speed in ramp influence area, $S = 58.4$ mph
 R

Space mean speed in outer lanes, $S = 61.8$ mph
 0

Space mean speed for all vehicles, $S = 59.6$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7050	480	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1916	130	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8161	527	450	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.583$ Using Equation 1

FM

$v = v (P) = 4759$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8688	7050	Yes
FO			
v v	3402 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5461$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5461	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 50.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.862$
 S

Space mean speed in ramp influence area, $S = 22.2$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 27.3$ mph

Volume on adjacent Ramp	220	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3270	360	220	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	889	98	60	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3785	395	242	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 2207 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4180	7050	No
FO			
v v	1578 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2207	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 24.3$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.360$
 S

Space mean speed in ramp influence area, $S = 56.7$ mph
 R

Space mean speed in outer lanes, $S = 61.1$ mph
 0

Space mean speed for all vehicles, $S = 58.3$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2850	290	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	774	79	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3299	318	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 1659$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3299	7050	No
$F_i F$			
$v = v - v$	2981	7050	No
$F_O F R$			
v	318	3800	No
R			
$v v$	1640 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 1885$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	1885	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 7.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.457$
 S

Space mean speed in ramp influence area, $S = 54.5$ mph
 R

Space mean speed in outer lanes, $S = 69.7$ mph
 0

Space mean speed for all vehicles, $S = 60.1$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7000	820	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1902	223	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8103	900	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 4141$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	8103	7050	Yes
Fi F			
$v = v - v$	7203	7050	Yes
FO F R			
v	900	3800	No
R			
$v = v$	3962 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5403$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5403	4600	No

12A !

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 37.2$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.509$
 S

Space mean speed in ramp influence area, $S = 53.3$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.6$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3090	570	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	840	155	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3577	626	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 0.642$ Using Equation 5
 FD
 $v = v + (v - v) P = 2520$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3577	7050	No
$F_i F$			
$v = v - v$	2951	7050	No
$FO F R$			
v	626	2000	No
R			
$v v$	1057 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2520	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 24.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.484$
 S

Space mean speed in ramp influence area, $S = 53.9$ mph
 R

Space mean speed in outer lanes, $S = 71.1$ mph
 0

Space mean speed for all vehicles, $S = 58.0$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7540	1000	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2049	272	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8728	1098	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.491 Using Equation 5

FD

$v = v + (v - v) P = 4847$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	8728	7050	Yes
Fi F			
v = v - v	7630	7050	Yes
FO F R			
v	1098	2000	No
R			
v v	3881 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 6028$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	6028	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 54.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.527$
 S

Space mean speed in ramp influence area, $S = 52.9$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.0$ mph

Volume on adjacent Ramp	290	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2560	120	290	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	696	33	79	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2963	132	318	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.595$ Using Equation 1

FM

$v = v (P) = 1763$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3095	7050	No
FO			
v v	1200 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1763	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.3$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.304$
 S

Space mean speed in ramp influence area, $S = 58.0$ mph
 R

Space mean speed in outer lanes, $S = 62.5$ mph
 0

Space mean speed for all vehicles, $S = 59.7$ mph

Volume on adjacent Ramp	540	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6180	140	540	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1679	38	147	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7154	154	593	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 4256 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7308	7050	Yes
FO			
v v	2898 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4454$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4454	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 37.5$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.669$
 S

Space mean speed in ramp influence area, $S = 49.6$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 51.8$ mph

Volume on adjacent Ramp	270	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2520	60	270	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	685	16	73	v
Trucks and buses	2	2	2	%
Recreational vehicles	13	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.965	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2838	66	296	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 1663 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2904	7050	No
FO			
v v	1175 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1663	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.322$
 S

Space mean speed in ramp influence area, $S = 57.6$ mph
 R

Space mean speed in outer lanes, $S = 62.6$ mph
 0

Space mean speed for all vehicles, $S = 59.5$ mph

Volume on adjacent Ramp	300	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6540	160	300	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1777	43	82	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7571	176	329	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 4436 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7747	7050	Yes
FO			
v v	3135 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4871$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4871	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 42.9$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.907$
 S

Space mean speed in ramp influence area, $S = 44.1$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 47.7$ mph

Volume on adjacent Ramp	120	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	2680	290	120	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	728	79	33	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3102	318	132	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.614$ Using Equation 1

FM

$v = v (P) = 1906$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	3420	7050	No
FO			
v v	1196 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1906	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 14.4$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.265$
 S

Space mean speed in ramp influence area, $S = 58.9$ mph
 R

Space mean speed in outer lanes, $S = 62.5$ mph
 0

Space mean speed for all vehicles, $S = 60.1$ mph

Volume on adjacent Ramp	140	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6320	540	140	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1717	147	38	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7316	593	154	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 4495 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7909	7050	Yes
FO			
v v	2821 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4616$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4616	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 37.6$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.942$
 S

Space mean speed in ramp influence area, $S = 43.3$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 47.0$ mph

Volume on adjacent Ramp	60	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2580	270	60	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	701	73	16	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2987	296	66	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 1758 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3283	7050	No
FO			
v v	1229 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1758	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 18.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.323$
 S

Space mean speed in ramp influence area, $S = 57.6$ mph
 R

Space mean speed in outer lanes, $S = 62.4$ mph
 0

Space mean speed for all vehicles, $S = 59.3$ mph

Volume on adjacent Ramp	160	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6700	300	160	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1821	82	43	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7756	329	176	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.589$ Using Equation 1

FM

$v = v (P) = 4566$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	8085	7050	Yes
FO			
v v	3190 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5056$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5056	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 44.8$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.144$

S

Space mean speed in ramp influence area, $S = 38.7$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S = 43.2$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2334	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	52.8	mi/h
Number of lanes, N	2	
Density, D	44.2	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2409	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1293	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	19.9	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1678	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	64.6	mi/h
Number of lanes, N	2	
Density, D	26.0-	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2415	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2459	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2490	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2290	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	54.3	mi/h
Number of lanes, N	2	
Density, D	42.2	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4170	840	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1133	228	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4669	922	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4669$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4669	4700	No
$F_i F$			
$v = v - v$	3747	4700	No
$F O F R$			
v	922	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4669	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.511$
 S

Space mean speed in ramp influence area, $S = 53.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.2$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4303	860	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1169	234	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4817	944	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 1.000$ Using Equation 0
 FD
 $v = v + (v - v) P = 4817$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4817	4700	Yes
$F_i F$			
$v = v - v$	3873	4700	No
$FO F R$			
v	944	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4817	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 44.1$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.513$
 S

Space mean speed in ramp influence area, $S = 53.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.2$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3028	1120	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	823	304	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3390	1230	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 3390$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4620	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3390	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 39.4$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.699$
 S

Space mean speed in ramp influence area, $S = 48.9$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 48.9$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2693	1430	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	732	389	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3015	1570	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 3015 \text{ pc/h}$

12 F FM

Capacity Checks

Actual Maximum LOS F?
 v 4585 4700 No
 FO
 $v \ v \ 0 \ \text{pc/h}$ (Equation 25-4 or 25-5)
 3 or av34
 Is $v \ v > 2700 \ \text{pc/h?}$ No
 3 or av34
 Is $v \ v > 1.5 \ v \ /2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3015	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 38.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.686$
 S

Space mean speed in ramp influence area, $S = 49.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 49.2$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4141	830	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1125	226	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4636	911	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4636$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4636	4700	No
$F_i F$			
$v = v - v$	3725	4700	No
$F_O F R$			
v	911	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4636	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.510$
 S

Space mean speed in ramp influence area, $S = 53.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.3$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3774	700	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1026	190	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4225	768	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4225$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4225	4700	No
$F_i F$			
$v = v - v$	3457	4700	No
$F_O F R$			
v	768	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4225	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 38.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.497$
 S

Space mean speed in ramp influence area, $S = 53.6$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.6$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 OFF Mack AM
Junction:
Jurisdiction:
Analysis Year: Base No Proj
Description: Delta Shores

 Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	4449	vph

 Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	350	vph
Length of first accel/decel lane	200	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4449	350	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1209	95	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4981	384	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4981$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4981	4700	Yes
$F_i F$			
$v = v - v$	4597	4700	No
$F_O F R$			
v	384	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4981	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 45.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.463$
 S

Space mean speed in ramp influence area, $S = 54.4$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.4$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4090	510	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1111	139	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4579	560	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4579$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4579	4700	No
$F_i F$			
$v = v - v$	4019	4700	No
$F_O F R$			
v	560	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4579	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.478$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.0$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 ON Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	2093	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	240	vph
Length of first accel/decel lane	300	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2093	240	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	569	65	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2343	263	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2343$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2606	4700	No
FO			
$v v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v /2$		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2343	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 23.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.353$
 S

Space mean speed in ramp influence area, $S = 56.9$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.9$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 ON Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Base No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2643	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	410	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2643	410	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	718	111	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2959	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 2959 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3409	4700	No
FO			
$v \ v \ 0 \text{ pc/h}$		(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2959	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.418$
 S

Space mean speed in ramp influence area, $S = 55.4$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 55.4$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	3037	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1806	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	63.9	mi/h
Number of lanes, N	3	
Density, D	28.3	pc/mi/ln
Level of service, LOS	D	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1443	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	22.2	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3041	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3210	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1740	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	64.3	mi/h
Number of lanes, N	3	
Density, D	27.0	pc/mi/ln
Level of service, LOS	D	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1470	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	22.6	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3195	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	2713	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1439	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	22.1	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1266	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	19.5	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2844	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7030	890	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1910	242	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8138	977	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 4199$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	8138	7050	Yes
Fi F			
v = v - v	7161	7050	Yes
FO F R			
v	977	3800	No
R			
v v	3939 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5438$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5438	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 37.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.516$
 S

Space mean speed in ramp influence area, $S = 53.1$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3730	690	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1014	187	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4318	757	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 2359$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4318	7050	No
$F_i F$			
$v = v - v$	3561	7050	No
$F_O F R$			
v	757	3800	No
R			
$v v$	1959 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 2467$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2467	4600	No

12A !

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 12.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.496$
 S

Space mean speed in ramp influence area, $S = 53.6$ mph
 R

Space mean speed in outer lanes, $S = 68.0$ mph
 0

Space mean speed for all vehicles, $S = 58.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7870	440	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2139	120	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	9110	483	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.510 Using Equation 5
 FD
 $v = v + (v - v) P = 4883 \text{ pc/h}$
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	9110	7050	Yes
Fi F			
v = v - v	8627	7050	Yes
FO F R			
v	483	2000	No
R			
v v	4227 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 6410$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	6410	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 58.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.471$
 S

Space mean speed in ramp influence area, $S = 54.2$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4680	820	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1272	223	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5418	900	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.583 Using Equation 5

FD

$v = v + (v - v) P = 3535$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5418	7050	No
$F_i F$			
$v = v - v$	4518	7050	No
$F_O F R$			
v	900	2000	No
R			
$v v$	1883 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3535	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 33.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $D = 0.509$
 S

Space mean speed in ramp influence area, $S = 53.3$ mph
 R

Space mean speed in outer lanes, $S = 67.9$ mph
 0

Space mean speed for all vehicles, $S = 57.6$ mph

Volume on adjacent Ramp	1620	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6140	110	1620	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1668	30	440	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7108	121	1778	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 4314 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7229	7050	Yes
FO			
v v	2794 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4408$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4408	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 34.2$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.609$
 S

Space mean speed in ramp influence area, $S = 51.0$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 52.8$ mph

Volume on adjacent Ramp	1600	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3040	40	1600	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	826	11	435	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3519	44	1757	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 2136 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3563	7050	No
FO			
v v	1383 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2136	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 15.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.282$
 S

Space mean speed in ramp influence area, $S = 58.5$ mph
 R

Space mean speed in outer lanes, $S = 61.8$ mph
 0

Space mean speed for all vehicles, $S = 59.8$ mph

Volume on adjacent Ramp	480	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7430	410	480	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2019	111	130	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8601	450	527	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 5075 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9051	7050	Yes
FO			
v v	3526 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5901$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5901	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 52.0$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 2.524$
 S

Space mean speed in ramp influence area, $S = 6.9$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 9.4$ mph

Volume on adjacent Ramp	450	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3860	200	450	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1049	54	122	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4468	220	494	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.590$ Using Equation 1

FM

$v = v (P) = 2637$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4688	7050	No
FO			
v v	1831 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2637	4400	No

12 !

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 24.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.357$
 S

Space mean speed in ramp influence area, $S = 56.8$ mph
 R

Space mean speed in outer lanes, $S = 60.2$ mph
 0

Space mean speed for all vehicles, $S = 58.1$ mph

Volume on adjacent Ramp	110	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6250	1620	110	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1698	440	30	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7235	1778	121	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 4446 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9013	7050	Yes
FO			
v v	2789 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4535$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4535	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 45.6$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 2.380$
 S

Space mean speed in ramp influence area, $S = 10.3$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 13.6$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Slip Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Base + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	3080	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1600	vph
Length of first accel/decel lane	1320	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	40	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3080	1600	40	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	837	435	11	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3565	1757	44	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 2191 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	5322	7050	No
FO			
v v	1374 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2191	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 27.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.431$
 S

Space mean speed in ramp influence area, $S = 55.1$ mph
 R

Space mean speed in outer lanes, $S = 61.9$ mph
 0

Space mean speed for all vehicles, $S = 56.7$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7840	480	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2130	130	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	9076	527	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 5292 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9603	7050	Yes
FO			
v v	3784 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6376$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6376	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 57.8$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 4.188$

S

Space mean speed in ramp influence area, $S = -31.3$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S =$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
 E-mail:

 Merge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 6/27/2007
 Analysis time period:
 Freeway/Dir of Travel: I-5 NB ON Slip Pocket PM
 Junction:
 Jurisdiction:
 Analysis Year: Base + Proj
 Description: Delta Shores

 Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	4060	vph	

 On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	450	vph	
Length of first accel/decel lane	200	ft	
Length of second accel/decel lane		ft	

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	200	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4060	450	200	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1103	122	54	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4700	494	220	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 2741 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	5194	7050	No
FO			
$v \ v$	1959 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2741	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 29.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.406$
 S

Space mean speed in ramp influence area, $S = 55.7$ mph
 R

Space mean speed in outer lanes, $S = 59.7$ mph
 0

Space mean speed for all vehicles, $S = 57.1$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3740	1180	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1016	321	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4329	1295	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 2660$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4329	7050	No
$F_i F$			
$v = v - v$	3034	7050	No
$F_O F R$			
v	1295	3800	No
R			
$v v$	1669 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2660	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 13.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.545$
 S

Space mean speed in ramp influence area, $S = 52.5$ mph
 R

Space mean speed in outer lanes, $S = 68.7$ mph
 0

Space mean speed for all vehicles, $S = 57.7$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7880	1700	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2141	462	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	9122	1866	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 5131$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	9122	7050	Yes
Fi F			
$v = v - v$	7256	7050	Yes
FO F R			
v	1866	3800	No
R			
$v = v$	3991 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 6422$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	6422	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 46.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.596$
 S

Space mean speed in ramp influence area, $S = 51.3$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 54.6$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3810	570	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1035	155	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4410	626	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.621 Using Equation 5

FD

$v = v + (v - v) P = 2976$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4410	7050	No
$F_i F$			
$v = v - v$	3784	7050	No
$F_O F R$			
v	626	2000	No
R			
$v v$	1434 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2976	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 28.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $D = 0.484$
 S

Space mean speed in ramp influence area, $S = 53.9$ mph
 R

Space mean speed in outer lanes, $S = 69.6$ mph
 0

Space mean speed for all vehicles, $S = 58.1$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB OFF Pocket PM
Junction:
Jurisdiction:
Analysis Year: Base + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	8280	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1020	vph	
Length of first accel/decel lane	150	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	8280	1020	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2250	277	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	9585	1120	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.469 Using Equation 5

FD

$v = v + (v - v) P = 5089$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	9585	7050	Yes
Fi F			
v = v - v	8465	7050	Yes
FO F R			
v	1120	2000	No
R			
v v	4496 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 6885$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	6885	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 62.1$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.529$
 S

Space mean speed in ramp influence area, $S = 52.8$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 55.7$ mph

Volume on adjacent Ramp	250	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2560	470	250	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	696	128	68	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2963	516	274	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 1763 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3479	7050	No
FO			
v v	1200 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1763	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 19.1$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.316$
 S

Space mean speed in ramp influence area, $S = 57.7$ mph
 R

Space mean speed in outer lanes, $S = 62.5$ mph
 0

Space mean speed for all vehicles, $S = 59.3$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6180	780	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1679	212	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7154	856	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 4256 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8010	7050	Yes
FO			
v v	2898 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4454$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4454	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 42.6$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.067$
 S

Space mean speed in ramp influence area, $S = 40.5$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 44.7$ mph

Volume on adjacent Ramp	440	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3240	60	440	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	880	16	120	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3751	66	483	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 2198 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3817	7050	No
FO			
$v \ v$	1553 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2198	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 21.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.338$
 S

Space mean speed in ramp influence area, $S = 57.2$ mph
 R

Space mean speed in outer lanes, $S = 61.2$ mph
 0

Space mean speed for all vehicles, $S = 58.8$ mph

Volume on adjacent Ramp	460	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7260	160	460	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1973	43	125	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8404	176	505	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.586$ Using Equation 1

FM

$v = v (P) = 4924$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	8580	7050	Yes
FO			
v v	3480 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5704$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5704	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 49.4$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.695$

S

Space mean speed in ramp influence area, $S = 26.0$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S = 31.3$ mph

Volume on adjacent Ramp	470	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3030	250	470	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	823	68	128	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3508	274	516	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 2156 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3782	7050	No
FO			
v v	1352 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2156	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.273$
 S

Space mean speed in ramp influence area, $S = 58.7$ mph
 R

Space mean speed in outer lanes, $S = 61.9$ mph
 0

Space mean speed for all vehicles, $S = 59.8$ mph

Volume on adjacent Ramp	780	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6960	410	780	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1891	111	212	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8057	450	856	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 4951 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8507	7050	Yes
FO			
v v	3106 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5357$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5357	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 42.3$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.526$
 S

Space mean speed in ramp influence area, $S = 29.9$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 35.1$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Slip Pocket AM
Junction:
Jurisdiction:
Analysis Year: Base + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	3300	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	440	vph	
Length of first accel/decel lane	400	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	60	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3300	440	60	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	897	120	16	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3820	483	66	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.589$ Using Equation 1

FM

$v = v (P) = 2249$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	4303	7050	No
FO			
v v	1571 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2249	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 24.1$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.353$
 S

Space mean speed in ramp influence area, $S = 56.9$ mph
 R

Space mean speed in outer lanes, $S = 61.1$ mph
 0

Space mean speed for all vehicles, $S = 58.4$ mph

Volume on adjacent Ramp	160	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	7420	460	160	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2016	125	43	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8589	505	176	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 5056$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9094	7050	Yes
FO			
v v	3533 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5889$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5889	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 52.6$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 2.626$
 S

Space mean speed in ramp influence area, $S = 4.6$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 6.3$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2419	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	2462	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2501	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2296	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	54.1	mi/h
Number of lanes, N	2	
Density, D	42.4	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2343	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	52.5	mi/h
Number of lanes, N	2	
Density, D	44.6	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2426	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1333	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	20.5	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1690	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	64.6	mi/h
Number of lanes, N	2	
Density, D	26.2	pc/mi/ln
Level of service, LOS	D	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4186	860	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1137	234	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4686	944	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4686$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4686	4700	No
$F_i F$			
$v = v - v$	3742	4700	No
$F_O F R$			
v	944	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4686	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 43.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.513$
 S

Space mean speed in ramp influence area, $S = 53.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.2$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: NB 99 OFF Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Base + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	4334	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	900	vph	
Length of first accel/decel lane	175	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4334	900	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1178	245	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4852	988	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4852$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4852	4700	Yes
$F_i F$			
$v = v - v$	3864	4700	No
$F_O F R$			
v	988	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4852	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 44.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.517$
 S

Space mean speed in ramp influence area, $S = 53.1$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.1$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3028	1130	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	823	307	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3390	1241	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 3390 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4631	4700	No
FO			
$v \ v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3390	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 39.5$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.704$
 S

Space mean speed in ramp influence area, $S = 48.8$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 48.8$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2693	1440	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	732	391	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3015	1581	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 3015$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	4596	4700	No
FO			
$v v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v /2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3015	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 39.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.690$
 S

Space mean speed in ramp influence area, $S = 49.1$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 49.1$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4150	840	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1128	228	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4646	922	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4646$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4646	4700	No
$F_i F$			
$v = v - v$	3724	4700	No
$F_O F R$			
v	922	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4646	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.511$
 S

Space mean speed in ramp influence area, $S = 53.2$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.2$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3780	710	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1027	193	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4232	779	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4232$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4232	4700	No
$F_i F$			
$v = v - v$	3453	4700	No
$F_O F R$			
v	779	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4232	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 38.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.498$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4467	360	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1214	98	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5001	395	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 5001$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5001	4700	Yes
$F_i F$			
$v = v - v$	4606	4700	No
$F_O F R$			
v	395	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5001	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 45.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.464$
 S

Space mean speed in ramp influence area, $S = 54.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.3$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 OFF Mack PM
Junction:
Jurisdiction:
Analysis Year: Base + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	4102	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	520	vph	
Length of first accel/decel lane	200	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4102	520	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1115	141	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4592	571	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4592$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4592	4700	No
$F_i F$			
$v = v - v$	4021	4700	No
$F_O F R$			
v	571	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4592	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.9$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.479$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.0$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
 E-mail:

 Merge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 7/2/2007
 Analysis time period:
 Freeway/Dir of Travel: SB 99 ON Cosumnes AM
 Junction:
 Jurisdiction:
 Analysis Year: Base + Proj
 Description: Delta Shores

 Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2093	vph	

 On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	320	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2093	320	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	569	87	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2343	351	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2343$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2694	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2343	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 24.4$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.358$
 S

Space mean speed in ramp influence area, $S = 56.8$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.8$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2643	440	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	718	120	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2959	483	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2959$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3442	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2959	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.2$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.422$
 S

Space mean speed in ramp influence area, $S = 55.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 55.3$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	2859	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1269	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	19.5	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1077	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	16.6	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3363	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2975	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1326	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	20.4	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1174	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	18.1	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3552	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2824	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1164	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	17.9	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1087	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	16.7	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3344	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB OFF Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	7319	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	960	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7319	960	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1989	261	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8473	1054	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.450 Using Equation 0
 FD
 $v = v + (v - v) P = 4393 \text{ pc/h}$
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	8473	7050	Yes
Fi F			
v = v - v	7419	7050	Yes
FO F R			
v	1054	3800	No
R			
v v	4080 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5773$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5773	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 40.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.523$
 S

Space mean speed in ramp influence area, $S = 53.0$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.2$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB OFF Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	3016	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	450	vph	
Length of first accel/decel lane	0	ft	
Length of second accel/decel lane	1500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3016	450	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	820	122	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3491	494	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 1843$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3491	7050	No
$F_i F$			
$v = v - v$	2997	7050	No
$F_O F R$			
v	494	3800	No
R			
$v v$	1648 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 1994$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	1994	4600	No

12A !

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 7.9$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.472$
 S

Space mean speed in ramp influence area, $S = 54.1$ mph
 R

Space mean speed in outer lanes, $S = 69.4$ mph
 0

Space mean speed for all vehicles, $S = 59.8$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7409	810	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2013	220	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8577	889	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.505 Using Equation 5

FD

$v = v + (v - v) P = 4769$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	8577	7050	Yes
$F_i F$			
$v = v - v$	7688	7050	Yes
$F O F R$			
v	889	2000	No
R			
$v v$	3808 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5877$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5877	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 53.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.508$
 S

Space mean speed in ramp influence area, $S = 53.3$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 56.4$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3289	660	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	894	179	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3807	725	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 0.631$ Using Equation 5
 FD
 $v = v + (v - v) P = 2671$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3807	7050	No
$F_i F$			
$v = v - v$	3082	7050	No
$FO F R$			
v	725	2000	No
R			
$v v$	1136 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2671	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 25.9$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.493$
 S

Space mean speed in ramp influence area, $S = 53.7$ mph
 R

Space mean speed in outer lanes, $S = 70.8$ mph
 0

Space mean speed for all vehicles, $S = 57.8$ mph

Volume on adjacent Ramp	890	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6359	160	890	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1728	43	242	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7361	176	977	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 4467 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7537	7050	Yes
FO			
v v	2894 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4661$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4661	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 36.5$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.739$
 S

Space mean speed in ramp influence area, $S = 48.0$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 50.6$ mph

Volume on adjacent Ramp	690	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2701	150	690	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	734	41	187	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3127	165	757	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 1898 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3292	7050	No
FO			
v v	1229 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1898	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 14.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.278$
 S

Space mean speed in ramp influence area, $S = 58.6$ mph
 R

Space mean speed in outer lanes, $S = 62.4$ mph
 0

Space mean speed for all vehicles, $S = 60.0$ mph

Volume on adjacent Ramp	700	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	6599	410	700	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1793	111	190	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7639	450	768	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.590$ Using Equation 1

FM

$v = v (P) = 4508$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	8089	7050	Yes
FO			
v v	3131 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4939$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4939	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 44.5$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.144$
 S

Space mean speed in ramp influence area, $S = 38.7$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 43.2$ mph

Volume on adjacent Ramp	650	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2827	220	650	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	768	60	177	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3273	242	714	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 1931 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3515	7050	No
FO			
v v	1342 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1931	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 19.5$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.324$
 S

Space mean speed in ramp influence area, $S = 57.6$ mph
 R

Space mean speed in outer lanes, $S = 62.0$ mph
 0

Space mean speed for all vehicles, $S = 59.2$ mph

Volume on adjacent Ramp	160	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6519	890	160	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1771	242	43	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7546	977	176	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 4637 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8523	7050	Yes
FO			
v v	2909 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4846$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4846	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 42.2$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.547$

S

Space mean speed in ramp influence area, $S = 29.4$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S = 34.6$ mph

Volume on adjacent Ramp	150	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2806	690	150	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	762	187	41	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3248	757	165	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 1996 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4005	7050	No
FO			
v v	1252 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1996	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 18.3$ pc/mi/ln
R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.290$
S

Space mean speed in ramp influence area, $S = 58.3$ mph
R

Space mean speed in outer lanes, $S = 62.3$ mph
0

Space mean speed for all vehicles, $S = 59.5$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	7009	700	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1905	190	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8114	768	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 4731 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8882	7050	Yes
FO			
v v	3383 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5414$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5414	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 52.1$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 2.194$
 S

Space mean speed in ramp influence area, $S = 14.5$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 18.8$ mph

Volume on adjacent Ramp	220	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2981	650	220	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	810	177	60	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3451	714	242	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 2012 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4165	7050	No
FO			
v v	1439 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2012	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 25.2$ pc/mi/ln
R R 12 A

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.367$
S

Space mean speed in ramp influence area, $S = 56.6$ mph
R

Space mean speed in outer lanes, $S = 61.6$ mph
0

Space mean speed for all vehicles, $S = 58.2$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2790	540	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	758	147	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3230	593	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 0.450$ Using Equation 0
 FD
 $v = v + (v - v) P = 1780$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3230	7050	No
$F_i F$			
$v = v - v$	2637	7050	No
$FO F R$			
v	593	3800	No
R			
$v v$	1450 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 1845$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	1845	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 6.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.481$
 S

Space mean speed in ramp influence area, $S = 53.9$ mph
 R

Space mean speed in outer lanes, $S = 69.8$ mph
 0

Space mean speed for all vehicles, $S = 59.8$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	8716	1040	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2368	283	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	10090	1142	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 5169$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	10090	7050	Yes
$F_i F$			
$v = v - v$	8948	7050	Yes
$F_O F R$			
v	1142	3800	No
R			
$v v$	4921 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 7390$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	7390	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 54.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.531$
 S

Space mean speed in ramp influence area, $S = 52.8$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 55.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3042	760	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	827	207	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3521	834	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.634 Using Equation 5

FD

$v = v + (v - v) P = 2537$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3521	7050	No
$F_i F$			
$v = v - v$	2687	7050	No
$F_O F R$			
v	834	2000	No
R			
$v v$	984 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2537	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 24.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.503$
 S

Space mean speed in ramp influence area, $S = 53.4$ mph
 R

Space mean speed in outer lanes, $S = 71.3$ mph
 0

Space mean speed for all vehicles, $S = 57.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	9206	1220	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2502	332	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	10657	1339	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.432 Using Equation 5

FD

$v = v + (v - v) P = 5364$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	10657	7050	Yes
$F_i F$			
$v = v - v$	9318	7050	Yes
$F O F R$			
v	1339	2000	No
R			
$v v$	5293 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 7957$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	7957	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 71.3$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.549$
 S

Space mean speed in ramp influence area, $S = 52.4$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 55.0$ mph

Volume on adjacent Ramp	390	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2412	190	390	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	655	52	106	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2792	209	428	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.595$ Using Equation 1

FM

$v = v (P) = 1661$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	3001	7050	No
FO			
$v v$	1131 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v /2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1661	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 16.1$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.303$
 S

Space mean speed in ramp influence area, $S = 58.0$ mph
 R

Space mean speed in outer lanes, $S = 62.7$ mph
 0

Space mean speed for all vehicles, $S = 59.7$ mph

Volume on adjacent Ramp	710	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	7676	280	710	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2086	76	193	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8886	307	779	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 5286 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9193	7050	Yes
FO			
v v	3600 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6186$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6186	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 52.1$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 2.854$
 S

Space mean speed in ramp influence area, $S = -0.6$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	300	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2580	100	300	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	701	27	82	v
Trucks and buses	2	2	2	%
Recreational vehicles	13	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.965	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2905	110	329	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 1702 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3015	7050	No
FO			
v v	1203 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1702	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.7$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.324$
 S

Space mean speed in ramp influence area, $S = 57.6$ mph
 R

Space mean speed in outer lanes, $S = 62.5$ mph
 0

Space mean speed for all vehicles, $S = 59.4$ mph

Volume on adjacent Ramp	440	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	8276	290	440	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2249	79	120	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	9580	318	483	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 5613 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9898	7050	Yes
FO			
v v	3967 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6880$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6880	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 59.6$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 5.513$

S

Space mean speed in ramp influence area, $S = -61.8$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	190	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2545	390	190	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	692	106	52	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2946	428	209	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

$v = v (P) = 1810 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3374	7050	No
FO			
v v	1136 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1810	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 14.5$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.265$
 S

Space mean speed in ramp influence area, $S = 58.9$ mph
 R

Space mean speed in outer lanes, $S = 62.7$ mph
 0

Space mean speed for all vehicles, $S = 60.1$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Slip Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	7956	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	710	vph
Length of first accel/decel lane	1320	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	280	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	7956	710	280	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2162	193	76	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	9210	779	307	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

v = v (P) = 5659 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9989	7050	Yes
FO			
v v	3551 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6510$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6510	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 53.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 5.939$
 S

Space mean speed in ramp influence area, $S = -71.6$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	100	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2580	300	100	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	701	82	27	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2987	329	110	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 1758 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3316	7050	No
FO			
v v	1229 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1758	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 19.1$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.324$
 S

Space mean speed in ramp influence area, $S = 57.5$ mph
 R

Space mean speed in outer lanes, $S = 62.4$ mph
 0

Space mean speed for all vehicles, $S = 59.2$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Slip Pocket PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	8276	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	440	vph
Length of first accel/decel lane	400	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	290	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	8276	440	290	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2249	120	79	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	9580	483	318	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 5640 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	10063	7050	Yes
FO			
v v	3940 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6880$ (Equation 25-8)

12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6880	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 60.2$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 6.442$

S

Space mean speed in ramp influence area, $S = -83.2$ mph

R

Space mean speed in outer lanes, $S = 56.1$ mph

0

Space mean speed for all vehicles, $S =$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2502	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.21

Phone:
 E-mail:
 Fax:

Operational Analysis

Analyst: BP
 Agency or Company: Fehr & Peers
 Date Performed: 7/2/2007
 Analysis Time Period:
 Freeway/Direction: N of Mack, NB PM
 From/To:
 Jurisdiction:
 Analysis Year: Cum No Proj
 Description: Delta Shores

Flow Inputs and Adjustments

Volume, V	4474	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1216	v
Trucks and buses	6	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.971	
Driver population factor, fp	1.00	
Flow rate, vp	2504	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
------------	------	----

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2504	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	2681	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2340	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	52.6	mi/h
Number of lanes, N	2	
Density, D	44.5	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2228	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	56.3	mi/h
Number of lanes, N	2	
Density, D	39.6	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2424	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1394	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	21.4	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1478	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	22.7	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3980	1020	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1082	277	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4456	1120	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4456$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4456	4700	No
$F_i F$			
$v = v - v$	3336	4700	No
$F_O F R$			
v	1120	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4456	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.0$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.529$
 S

Space mean speed in ramp influence area, $S = 52.8$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.8$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4330	1030	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1177	280	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4848	1131	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 1.000$ Using Equation 0
 FD
 $v = v + (v - v) P = 4848$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4848	4700	Yes
$F_i F$			
$v = v - v$	3717	4700	No
$FO F R$			
v	1131	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4848	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 44.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.530$
 S

Space mean speed in ramp influence area, $S = 52.8$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.8$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3160	1150	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	859	312	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3538	1262	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 3538$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4800	4700	Yes
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3538	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 40.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.777$
 S

Space mean speed in ramp influence area, $S = 47.1$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 47.1$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: NB 99 ON Loop Mack PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

 Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	2704	vph

 On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1480	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2704	1480	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	735	402	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3027	1625	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
 EQ
 P = 1.000 Using Equation 0
 FM
 $v = v (P) = 3027 \text{ pc/h}$
 12 F FM

Capacity Checks

Actual Maximum LOS F?
 v 4652 4700 No
 FO
 $v \text{ v } 0 \text{ pc/h}$ (Equation 25-4 or 25-5)
 3 or av34
 Is $v \text{ v } > 2700 \text{ pc/h?}$ No
 3 or av34
 Is $v \text{ v } > 1.5 \text{ v } /2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3027	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 39.4$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.712$
 S

Space mean speed in ramp influence area, $S = 48.6$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 48.6$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 OFF Bruceville AM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	4480	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1080	vph	
Length of first accel/decel lane	200	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4480	1080	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1217	293	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5016	1186	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 5016$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5016	4700	Yes
$F_i F$			
$v = v - v$	3830	4700	No
$F_O F R$			
v	1186	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5016	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 45.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.535$
 S

Space mean speed in ramp influence area, $S = 52.7$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 OFF Bruceville PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	3854	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	700	vph
Length of first accel/decel lane	200	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3854	700	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1047	190	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4315	768	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4315$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4315	4700	No
$F_i F$			
$v = v - v$	3547	4700	No
$F_O F R$			
v	768	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4315	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 39.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.497$
 S

Space mean speed in ramp influence area, $S = 53.6$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.6$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
 E-mail:

Diverge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 7/2/2007
 Analysis time period:
 Freeway/Dir of Travel: SB 99 OFF Mack AM
 Junction:
 Jurisdiction:
 Analysis Year: Cum No Proj
 Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	4790	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	350	vph
Length of first accel/decel lane	200	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4790	350	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1302	95	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5363	384	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 5363$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5363	4700	Yes
$F_i F$			
$v = v - v$	4979	4700	Yes
$F_O F R$			
v	384	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5363	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 48.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.463$
 S

Space mean speed in ramp influence area, $S = 54.4$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.4$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4180	510	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1136	139	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4680	560	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4680$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4680	4700	No
$F_i F$			
$v = v - v$	4120	4700	No
$F_O F R$			
v	560	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4680	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.478$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.0$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2151	360	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	585	98	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2408	395	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 2408$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	2803	4700	No
FO			
$v v$	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v /2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2408	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 25.3$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.364$
 S

Space mean speed in ramp influence area, $S = 56.6$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.6$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

 Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: SB 99 ON Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum No Proj
Description: Delta Shores

 Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	2160	vph

 On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	510	vph
Length of first accel/decel lane	300	ft
Length of second accel/decel lane		ft

 Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2160	510	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	587	139	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2418	560	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

$v = v (P) = 2418 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2978	4700	No
FO			
$v \ v \ 3 \text{ or } av34$	0 pc/h	(Equation 25-4 or 25-5)	
Is $v \ v \ > 2700 \text{ pc/h?}$		No	
$3 \text{ or } av34$			
Is $v \ v \ > 1.5 \ v \ /2$		No	
$3 \text{ or } av34 \ 12$			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2418	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 26.6$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.377$
 S

Space mean speed in ramp influence area, $S = 56.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.3$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Slip Pocket AM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	7580	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	720	vph	
Length of first accel/decel lane	200	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	7580	720	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2060	196	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8775	790	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.583 Using Equation 1

FM

$v = v (P) = 5117 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9565	7050	Yes
FO			
v v	3658 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6075$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6075	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 57.4$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 4.044$
 S

Space mean speed in ramp influence area, $S = -28.0$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	200	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3374	780	200	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	917	212	54	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3906	856	220	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.583$ Using Equation 1

FM

$v = v (P) = 2278$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	4762	7050	No
FO			
v v	1628 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2278	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 28.3$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $M = 0.397$
 S

Space mean speed in ramp influence area, $S = 55.9$ mph
 R

Space mean speed in outer lanes, $S = 60.9$ mph
 0

Space mean speed for all vehicles, $S = 57.5$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2879	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1267	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	19.5	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1132	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	17.4	pc/mi/ln
Level of service, LOS	B	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	3454	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3045	900	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	827	245	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3525	988	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.450 Using Equation 0
 FD
 $v = v + (v - v) P = 2130 \text{ pc/h}$
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	3525	7050	No
Fi F			
v = v - v	2537	7050	No
FO F R			
v	988	3800	No
R			
v v	1395 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2130	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 9.1$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.517$
 S

Space mean speed in ramp influence area, $S = 53.1$ mph
 R

Space mean speed in outer lanes, $S = 69.8$ mph
 0

Space mean speed for all vehicles, $S = 58.7$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	9560	1880	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2598	511	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	11067	2064	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 6115$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	11067	7050	Yes
Fi F			
$v = v - v$	9003	7050	Yes
FO F R			
v	2064	3800	No
R			
$v = v$	4952 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 8367$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	8367	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 62.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.614$
 S

Space mean speed in ramp influence area, $S = 50.9$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 53.7$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3199	730	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	869	198	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3703	801	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.631 Using Equation 5

FD

$v = v + (v - v) P = 2631$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3703	7050	No
$F_i F$			
$v = v - v$	2902	7050	No
$F O F R$			
v	801	2000	No
R			
$v v$	1072 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2631	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 25.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $D = 0.500$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = 71.0$ mph
 0

Space mean speed for all vehicles, $S = 57.6$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	9960	1250	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2707	340	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	11530	1372	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.409 Using Equation 5

FD

$v = v + (v - v) P = 5523$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	11530	7050	Yes
$F_i F$			
$v = v - v$	10158	7050	Yes
$F_O F R$			
v	1372	2000	No
R			
$v v$	6007 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 8830$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	8830	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 78.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.551$
 S

Space mean speed in ramp influence area, $S = 52.3$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 54.8$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Loop Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2415	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	370	vph	
Length of first accel/decel lane	620	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	370	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2415	370	370	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	656	101	101	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2796	406	406	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 1663 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3202	7050	No
FO			
v v	1133 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1663	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.5$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.308$
 S

Space mean speed in ramp influence area, $S = 57.9$ mph
 R

Space mean speed in outer lanes, $S = 62.7$ mph
 0

Space mean speed for all vehicles, $S = 59.5$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 SB ON Loop Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	7680	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	630	vph	
Length of first accel/decel lane	620	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	640	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7680	630	640	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2087	171	174	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8890	692	703	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.595 Using Equation 1

FM

$v = v (P) = 5288 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9582	7050	Yes
FO			
v v	3602 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6190$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6190	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 54.9$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 4.078$
 S

Space mean speed in ramp influence area, $S = -28.8$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2688	100	410	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	730	27	111	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3112	110	450	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 1823 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3222	7050	No
FO			
$v \ v$	1289 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v \ v > 2700 \text{ pc/h?}$		No	
3 or av34			
Is $v \ v > 1.5 \ v / 2$		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1823	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 18.6$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.327$
 S

Space mean speed in ramp influence area, $S = 57.5$ mph
 R

Space mean speed in outer lanes, $S = 62.2$ mph
 0

Space mean speed for all vehicles, $S = 59.3$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
 E-mail:

Merge Analysis

Analyst: BP
 Agency/Co.: Fehr & Peers
 Date performed: 6/27/2007
 Analysis time period:
 Freeway/Dir of Travel: I-5 SB ON Loop Pocket PM
 Junction:
 Jurisdiction:
 Analysis Year: Cum + Proj
 Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	8710	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	290	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	560	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	8710	290	560	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2367	79	152	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	10083	318	615	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.586 Using Equation 1

FM

$v = v (P) = 5908 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	10401	7050	Yes
FO			
v v	4175 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 7383$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	7383	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 63.5$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 8.921$
 S

Space mean speed in ramp influence area, $S = -140.2$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	370	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2674	370	370	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	727	101	101	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3095	406	406	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

v = v (P) = 1902 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3501	7050	No
FO			
v v	1193 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1902	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 15.0$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.268$
 S

Space mean speed in ramp influence area, $S = 58.8$ mph
 R

Space mean speed in outer lanes, $S = 62.5$ mph
 0

Space mean speed for all vehicles, $S = 60.0$ mph

Volume on adjacent Ramp	630	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	8310	640	630	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2258	174	171	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	9620	703	692	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.614$ Using Equation 1

FM

$v = v (P) = 5911$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	10323	7050	Yes
FO			
v v	3709 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 6920$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	6920	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 56.3$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 8.203$
 S

Space mean speed in ramp influence area, $S = -123.7$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

Volume on adjacent Ramp	100	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2758	410	100	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	749	111	27	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3193	450	110	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.589 Using Equation 1

FM

$v = v (P) = 1880 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3643	7050	No
FO			
v v	1313 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1880	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.9$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.333$
 S

Space mean speed in ramp influence area, $S = 57.3$ mph
 R

Space mean speed in outer lanes, $S = 62.1$ mph
 0

Space mean speed for all vehicles, $S = 59.0$ mph

Volume on adjacent Ramp	290	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	9000	560	290	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	2446	152	79	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	10418	615	318	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.589$ Using Equation 1

FM

$v = v (P) = 6133$ pc/h

$12 F FM$

Capacity Checks

	Actual	Maximum	LOS F?
v	11033	7050	Yes
FO			
v v	4285 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 7718$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	7718	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 67.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 16.513$
 S

Space mean speed in ramp influence area, $S = -314.8$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 508.8$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3133	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1493	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	23.0	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1175	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	18.1	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3689	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3203	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	1543	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	23.8	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFSS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	1234	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	3	
Density, D	19.0	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	3843	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	3	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	7460	1100	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2027	299	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	8636	1208	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 4551$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	8636	7050	Yes
$F_i F$			
$v = v - v$	7428	7050	Yes
$F_O F R$			
v	1208	3800	No
R			
$v v$	4085 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 5936$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5936	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.537$
 S

Space mean speed in ramp influence area, $S = 52.7$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 55.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3283	830	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	892	226	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3800	911	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 2211$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3800	7050	No
$F_i F$			
$v = v - v$	2889	7050	No
$F_O F R$			
v	911	3800	No
R			
$v v$	1589 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2211	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 9.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.510$
 S

Space mean speed in ramp influence area, $S = 53.3$ mph
 R

Space mean speed in outer lanes, $S = 69.0$ mph
 0

Space mean speed for all vehicles, $S = 58.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	8120	950	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	2207	258	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	9400	1043	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.477 Using Equation 5
 FD
 $v = v + (v - v) P = 5029 \text{ pc/h}$
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	9400	7050	Yes
Fi F			
v = v - v	8357	7050	Yes
FO F R			
v	1043	2000	No
R			
v v	4371 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? Yes
 3 or av34

Is $v > 1.5 v / 2$ Yes
 3 or av34 12

If yes, $v = 6700$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	6700	4600	No
12A			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 60.5$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.522$
 S

Space mean speed in ramp influence area, $S = 53.0$ mph
 R

Space mean speed in outer lanes, $S = 64.7$ mph
 0

Space mean speed for all vehicles, $S = 55.9$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3870	850	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1052	231	v
Trucks and buses	13	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4480	933	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.605 Using Equation 5

FD

$v = v + (v - v) P = 3079$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4480	7050	No
$F_i F$			
$v = v - v$	3547	7050	No
$F_O F R$			
v	933	2000	No
R			
$v v$	1401 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3079	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 29.4$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable, $D = 0.512$
 S

Space mean speed in ramp influence area, $S = 53.2$ mph
 R

Space mean speed in outer lanes, $S = 69.7$ mph
 0

Space mean speed for all vehicles, $S = 57.5$ mph

Volume on adjacent Ramp	1580	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6360	180	1580	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1728	49	429	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7362	198	1735	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 4468 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7560	7050	Yes
FO			
v v	2894 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4662$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4662	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 36.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.751$
 S

Space mean speed in ramp influence area, $S = 47.7$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S = 50.4$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Loop Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	65.0	mph	
Volume on freeway	2702	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	120	vph	
Length of first accel/decel lane	1050	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	1490	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2702	120	1490	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	734	33	405	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3128	132	1636	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.607 Using Equation 1

FM

$v = v (P) = 1898 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3260	7050	No
FO			
v v	1230 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1898	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 14.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.277$
 S

Space mean speed in ramp influence area, $S = 58.6$ mph
 R

Space mean speed in outer lanes, $S = 62.4$ mph
 0

Space mean speed for all vehicles, $S = 60.0$ mph

Volume on adjacent Ramp	720	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	7170	410	720	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1948	111	196	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	8300	450	790	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 4898 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	8750	7050	Yes
FO			
v v	3402 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 5600$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	5600	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 49.6$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 1.944$
 S

Space mean speed in ramp influence area, $S = 20.3$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 O

Space mean speed for all vehicles, $S = 25.3$ mph

Volume on adjacent Ramp	780	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3234	200	780	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	879	54	212	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3744	220	856	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.590 Using Equation 1

FM

$v = v (P) = 2209 \text{ pc/h}$

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3964	7050	No
FO			
v v	1535 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2209	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 21.5$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.334$
 S

Space mean speed in ramp influence area, $S = 57.3$ mph
 R

Space mean speed in outer lanes, $S = 61.3$ mph
 0

Space mean speed for all vehicles, $S = 58.8$ mph

Volume on adjacent Ramp	180	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	6540	1580	180	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1777	429	49	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7571	1735	198	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

v = v (P) = 4652 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	9306	7050	Yes
FO			
v v	2919 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		Yes	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v = 4871$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	4871	4400	No
12A			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 47.9$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 3.113$
 S

Space mean speed in ramp influence area, $S = -6.6$ mph
 R

Space mean speed in outer lanes, $S = 56.1$ mph
 0

Space mean speed for all vehicles, $S =$ mph

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Merge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 6/27/2007
Analysis time period:
Freeway/Dir of Travel: I-5 NB ON Slip Cosumnes PM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	65.0	mph
Volume on freeway	2786	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1490	vph
Length of first accel/decel lane	1320	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes

Volume on adjacent Ramp	120	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1380	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2786	1490	120	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	757	405	33	v
Trucks and buses	13	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3225	1636	132	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.614 Using Equation 1

FM

v = v (P) = 1982 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4861	7050	No
FO			
v v	1243 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1982	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 24.7$ pc/mi/ln
 R R 12 A

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.374$
 S

Space mean speed in ramp influence area, $S = 56.4$ mph
 R

Space mean speed in outer lanes, $S = 62.3$ mph
 0

Space mean speed for all vehicles, $S = 57.8$ mph

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	2506	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2508	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2691	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2347	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	52.3	mi/h
Number of lanes, N	2	
Density, D	44.8	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2236	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	56.0	mi/h
Number of lanes, N	2	
Density, D	39.9	pc/mi/ln
Level of service, LOS	E	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	2441	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S		mi/h
Number of lanes, N	2	
Density, D		pc/mi/ln
Level of service, LOS	F	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1432	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	22.0	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

Right-shoulder lateral clearance	6.0	ft
Interchange density	0.50	interchange/mi
Number of lanes, N	2	
Free-flow speed:	Measured	
FFS or BFFS	65.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.0	mi/h
Number of lanes adjustment, fN	4.5	mi/h
Free-flow speed, FFS	65.0	mi/h
Urban Freeway		

LOS and Performance Measures

Flow rate, vp	1488	pc/h/ln
Free-flow speed, FFS	65.0	mi/h
Average passenger-car speed, S	65.0	mi/h
Number of lanes, N	2	
Density, D	22.9	pc/mi/ln
Level of service, LOS	C	

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Ramps and Ramp Junctions Release 5.21

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: BP
Agency/Co.: Fehr & Peers
Date performed: 7/2/2007
Analysis time period:
Freeway/Dir of Travel: NB 99 OFF Cosumnes AM
Junction:
Jurisdiction:
Analysis Year: Cum + Proj
Description: Delta Shores

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	65.0	mph
Volume on freeway	3995	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1040	vph
Length of first accel/decel lane	175	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? No

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3995	1040	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1086	283	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4473	1142	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
 EQ
 P = 1.000 Using Equation 0
 FD
 $v = v + (v - v) P = 4473 \text{ pc/h}$
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	4473	4700	No
Fi F			
v = v - v	3331	4700	No
FO F R			
v	1142	2000	No
R			
v v	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4473	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 41.1$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.531$
 S

Space mean speed in ramp influence area, $S = 52.8$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.8$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4360	1070	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1185	291	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4881	1175	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4881$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4881	4700	Yes
$F_i F$			
$v = v - v$	3706	4700	No
$F_O F R$			
v	1175	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4881	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 44.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.534$
 S

Space mean speed in ramp influence area, $S = 52.7$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.7$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3160	1160	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	859	315	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3538	1273	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 3538$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4811	4700	Yes
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3538	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 40.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $M = 0.783$
 S

Space mean speed in ramp influence area, $S = 47.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 47.0$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2704	1490	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	735	405	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3027	1636	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 1.000$ Using Equation 0

FM

$v = v (P) = 3027$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	4663	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	3027	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 39.5$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $M = 0.717$
 S

Space mean speed in ramp influence area, $S = 48.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 48.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4489	1090	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1220	296	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5026	1197	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 5026$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5026	4700	Yes
$F_i F$			
$v = v - v$	3829	4700	No
$F_O F R$			
v	1197	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5026	4600	No

12 !

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 45.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.536$
 S

Space mean speed in ramp influence area, $S = 52.7$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 52.7$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	3860	710	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1049	193	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4322	779	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4322$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4322	4700	No
$F_i F$			
$v = v - v$	3543	4700	No
$F_O F R$			
v	779	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4322	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 39.6$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.498$
 S

Space mean speed in ramp influence area, $S = 53.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 53.5$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4807	360	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1306	98	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5382	395	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 5382$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	5382	4700	Yes
$F_i F$			
$v = v - v$	4987	4700	Yes
$F_O F R$			
v	395	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	5382	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 48.7$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable, $D = 0.464$
 S

Space mean speed in ramp influence area, $S = 54.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.3$ mph

Volume on adjacent ramp vph
 Position of adjacent ramp
 Type of adjacent ramp
 Distance to adjacent ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4192	510	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	1139	139	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4693	560	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 4693$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	4693	4700	No
$F_i F$			
$v = v - v$	4133	4700	No
$F_O F R$			
v	560	2000	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			

Is $v > 2700$ pc/h? No
 3 or av34

Is $v > 1.5 v / 2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-18)
 12A

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	4693	4600	No
12			!

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 42.8$ pc/mi/ln
 R 12 D

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable, $D = 0.478$
 S

Space mean speed in ramp influence area, $S = 54.0$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 54.0$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2151	440	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	585	120	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2408	483	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 2408 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2891	4700	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2408	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 25.9$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.370$
 S

Space mean speed in ramp influence area, $S = 56.5$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.5$ mph

Volume on adjacent Ramp vph
 Position of adjacent Ramp
 Type of adjacent Ramp
 Distance to adjacent Ramp ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	2160	540	vph
Peak-hour factor, PHF	0.92	0.92	
Peak 15-min volume, v15	587	147	v
Trucks and buses	6	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	
Heavy vehicle adjustment, fHV	0.971	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2418	593	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
 EQ
 P = 1.000 Using Equation 0
 FM
 $v = v (P) = 2418 \text{ pc/h}$
 12 F FM

Capacity Checks

Actual Maximum LOS F?
 v 3011 4700 No
 FO
 $v \text{ v } 0 \text{ pc/h}$ (Equation 25-4 or 25-5)
 3 or av34
 Is $v \text{ v } > 2700 \text{ pc/h?}$ No
 3 or av34
 Is $v \text{ v } > 1.5 \text{ v } /2$ No
 3 or av34 12

If yes, $v =$ (Equation 25-8)
 12A

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2418	4400	No
12			!

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 26.8$ pc/mi/ln
 $R \quad R \quad 12 \quad A$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, $M = 0.379$
 S

Space mean speed in ramp influence area, $S = 56.3$ mph
 R

Space mean speed in outer lanes, $S = N/A$ mph
 0

Space mean speed for all vehicles, $S = 56.3$ mph
